

GENERAL NOTES

MOORPARK COLLEGE REPAIR BASEBALL DUGOUTS

BID 623

7075 CAMPUS ROAD
MOORPARK, CALIFORNIA 93021

VENTURA COUNTY COMMUNITY COLLEGE

JUNE 2021

1. INTERPRETATION OF CONSTRUCTION DOCUMENTS
 - A. ALL INFORMATION DEPICTED IN THESE DRAWINGS AND RELATIVE TO EXISTING CONDITIONS IS BASED ON THE BEST AVAILABLE DATA AT THE TIME THESE CONSTRUCTION DOCUMENTS WERE BEING EXECUTED, BUT WITHOUT GUARANTEE OF ACCURACY. CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS AT JOB SITE AND SHALL REPORT ANY DISCREPANCIES TO ARCHITECT PRIOR TO COMMENCING ANY WORK.
 - B. THE CONTRACTOR IS RESPONSIBLE FOR ALL COSTS INCURRED RESULTING FROM THE REMOVAL OR REPLACEMENT OF WORK INSTALLED WITHOUT PROPER COORDINATION TO ALL OTHER TRADES, AND/OR PRIOR TO OBTAINING CLARIFICATION FROM THE ARCHITECT WHERE CONFLICTING INFORMATION EXISTS ON THE DRAWINGS.
 - C. THE CONTRACTOR SHALL FURNISH ALL BIDDERS WITH A COMPLETE SET OF CONSTRUCTION DOCUMENTS, INCLUDING BUT NOT LIMITED TO DRAWINGS, SPECIFICATIONS AND ADDENDA.
 - D. ALL BIDS AND LINE ITEM COSTS SUBMITTED BY THE CONTRACTOR IN CONJUNCTION WITH HIS SUBCONTRACTORS ARE CONSIDERED TO INCLUDE COMPLETE COORDINATION BETWEEN THE VARIOUS DISCIPLINES AS WELL AS ALL OTHER REQUIREMENTS OF THESE CONSTRUCTION DOCUMENTS, INCLUDING BUT NOT LIMITED TO CODE AND PUBLIC UTILITY REQUIREMENTS. FURTHER, WHERE THERE ARE CONFLICTING SOLUTIONS IN THE CONSTRUCTION DOCUMENTS AND BID OR LINE ITEM COST IS SUBMITTED BY THE CONTRACTOR WITHOUT ANY FORMAL WRITTEN REQUEST FOR CLARIFICATION PRIOR TO BID OPENING, ALL SUCH ITEMS WILL BE CONSIDERED TO INCLUDE THE MOST EXPENSIVE OF THE POSSIBLE SOLUTIONS DEPICTED IN THE CONSTRUCTION DOCUMENTS.
 - E. MODIFICATIONS OF DETAILS OF CONSTRUCTION SHALL NOT BE MADE WITHOUT WRITTEN APPROVAL OF THE ARCHITECT.
2. CONTRACTOR SHALL VISIT THE SITE TO INVESTIGATE AND VERIFY ALL DIMENSIONS AND EXISTING SITE CONDITIONS AT JOB SITE PRIOR TO START OF WORK.
3. ALL DIMENSIONS INDICATED ARE BELIEVED TO BE ACCURATE, BUT ARE NOT GUARANTEED TO BE SO. CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS AND REPORT ANY DISCREPANCIES TO THE ARCHITECT. COORDINATE WITH EXISTING CONDITIONS WHERE INSUFFICIENT DETAIL DIMENSIONS ARE AVAILABLE. ALL DIMENSIONS ARE TO FINISHED FACE OF CONSTRUCTION OR CENTERLINE OF COLUMNS UNLESS NOTED OTHERWISE. DIMENSIONS NOTED AT "CLR" (CLEAR) ARE NOT ADJUSTABLE WITHOUT ARCHITECT'S APPROVAL.
4. DIMENSIONS SHOWN SHALL HAVE PREFERENCE OVER SCALE.
5. ALL ITEMS INCLUDING BUILDINGS SHOWN ARE EXISTING (E) UNLESS NOTED NEW (N); EXCEPT FOR THE DETAIL SHEETS WHERE ITEMS SHOWN ARE NEW UNLESS NOTED EXISTING (E).
6. CONTRACTOR SHALL TAKE ALL PRECAUTIONARY MEASURES TO PROTECT EXISTING PIPELINES AND UTILITIES THAT ARE TO REMAIN IN SERVICE. CONTRACTOR SHALL VERIFY THAT THOSE PIPELINES AND UTILITIES TO BE REMOVED HAVE BEEN DISCONNECTED, SHUT DOWN OR ABANDONED PRIOR TO ATTEMPTING REMOVAL OR DEMOLITION IN A MANNER TO AVOID ANY DISRUPTION OF EXISTING FACILITIES.
7. CONTRACTOR SHALL PROTECT ALL SURFACES & FIXTURES TO REMAIN DURING DEMOLITION AND CONSTRUCTION.
8. ALL DAMAGE DONE TO EXISTING CONSTRUCTION AS A RESULT OF DEMOLITION OR INSTALLATION SHALL BE COMPLETELY REPAIRED BY CONTRACTOR AT OR NO COST TO OWNER. REPAIRED WORK SHALL MATCH EXISTING CONSTRUCTION.
9. CONTRACTOR SHALL REPAIR AND PATCH UP ALL DAMAGES TO EXISTING SURFACES CAUSED BY REMOVAL OF EXISTING EQUIPMENT ATTACHED TO EXISTING SURFACES. (CHALKBOARDS, BOOKSHELVES, TACKBOARDS, WALL HEATERS, PIPING, ETC.)
10. WHERE PATCHES ARE REQUIRED IN EXISTING, SURFACES ADJACENT MATERIAL SHALL BE MATCHED IN TEXTURE AND FINISH.
11. "DEMOLISH" AND "REMOVE" SHALL MEAN TO DEMOLISH, REMOVE FROM THE SITE AND DISPOSE OF IN A LEGAL MANNER UNLESS NOTED OTHERWISE. TERMINATE PIPING BELOW SUBSTRATE FOR PATCHING. ELECTRICAL WIRE DISCONNECT SHALL BE AT THE SOURCE OF POWER.
12. SALVAGED PRODUCTS SAVED FOR OWNER AS A RESULT OF DEMOLITION ACTIVITY AND/OR PRODUCTS STORED FOR USE IN CONSTRUCTION SHALL BE STORED IN A MANNER SUCH THAT NO MATERIALS ARE DAMAGED AND PUBLIC SAFETY IS MAINTAINED.
13. CONTRACTOR SHALL THOROUGHLY CLEAN AND SECURE THE AREA OF CONSTRUCTION AFTER EACH DAY OF WORK. CONTRACTOR SHALL REMOVE AND DISPOSE OF ALL CONSTRUCTION DEBRIS OFF SITE.
14. LOCATIONS OF STRUCTURES, UNDERGROUND PIPELINES AND UTILITIES WERE OBTAINED FROM AVAILABLE RECORDS. THE CONTRACTOR SHALL FIELD VERIFY THE LOCATIONS AND ELEVATIONS OF ALL PIPELINES AND UTILITIES BEFORE COMMENCING DEMOLITION, EARTHWORK OR CONSTRUCTION WORK.
15. GENERAL CONTRACTOR SHALL VERIFY ALL SITE CONDITIONS PRIOR TO START OF CONSTRUCTION. ALL QUESTIONS SHALL BE SENT TO ARCHITECT.
16. ALL SALVAGEABLE MATERIALS AND EQUIPMENT TO BE REMOVED SHALL REMAIN THE SOLE PROPERTY OF THE OWNER. THE CONTRACTOR SHALL CONSULT WITH THE OWNER CONCERNING STORAGE AND/OR DISPOSAL OF SUCH EQUIPMENT. OWNER HAS FULL SALVAGE RIGHTS. ALL REMOVED MATERIALS OTHER THAN ITEMS TO BE SALVAGED, OR REUSED SHALL BECOME CONTRACTOR'S PROPERTY AND SHALL BE REMOVED FROM THE PROJECT SITE.
17. ALL WORK, INCLUDING REMOVAL OF EXISTING WORK, SHALL BE PERFORMED IN A MANNER THAT MINIMIZES THE AMOUNT OF NOISE, DUST, TRAFFIC AND/OR OTHER FORMS OF DISTURBANCES IN COMPLIANCE WITH ALL APPLICABLE CODES AND ORDINANCES SO THAT THE PUBLIC, STUDENTS AND STAFF, AS WELL AS OTHER OCCUPIED AREAS OF THE SCHOOL ARE SUBJECTED TO AS LITTLE DISRUPTION AS REASONABLY POSSIBLE.
18. ROUTES OF INGRESS AND EGRESS FOR MATERIALS AND WORKMEN, AND LIMITS OF THE PROJECT AREA WILL BE DESIGNATED BY THE OWNER. THE CONTRACTOR SHALL CONFINE HIS ACTIVITIES WITHIN SUCH LIMITS. THE CONTRACTOR SHALL INSTALL AND MAINTAIN ADEQUATE SAFETY AND DUST BARRIERS IN THE SITE, ACROSS CORRIDORS AND ELSEWHERE AS REQUIRED.

GENERAL NOTES

19. SHUT DOWN OF EXISTING AND OPERATING PLUMBING, MECHANICAL AND ELECTRICAL SYSTEMS OR PORTIONS THEREOF SHALL BE COORDINATED IN ADVANCE WITH THE OWNER.
20. CONTRACTOR SHALL COORDINATE ALL WORK SHOWN ON THE ARCHITECTURAL DRAWINGS WITH THE SPECIFICATIONS AND THE WORK SHOWN ON THE MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS. ANY DISCREPANCIES FOUND SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT IN WRITING BEFORE PROCEEDING WITH ANY RELATED WORK.
21. NOT USED.
22. NOT USED.
23. BEFORE PROCEEDING WITH THE CORING OR CUTTING OF WALLS AND FLOORS, ETC., THE CONTRACTOR SHALL PREPARE LAYOUT OF CUTTING OR CORING AND SHALL HAVE THE APPROVAL BY THE STRUCTURAL ENGINEER IN ORDER TO PROCEED WITH THE CUTTING OR CORING.
24. SAW-CUT EXISTING A.C. PAVING AND/OR CONCRETE FLOOR SLAB AS REQUIRED FOR NEW PIPE INSTALLATION AND NEW DEPRESSED CONCRETE SLAB, AND REPAIR TO MATCH EXISTING.
25. STRENGTH OF CONCRETE:
 - A) SLABS ON EARTH, SIDEWALKS AND CURBS: 3,000 PSI AT 28 DAYS
 - B) FOUNDATIONS: 3,000 PSI AT 28 DAYS
 - C) FILL ON METAL DECK (LIGHTWEIGHT): 3,000 PSI AT 28 DAYS
26. THE CONTRACTOR SHALL NOT COMMENCE THE WORK, IN PART OR IN FULL, PRIOR TO OBTAINING THE NOTICE-TO-PROCEED (NTP) FROM VCCCD.
27. IN CASE OF CONFLICT, THE MORE EXPENSIVE CONSTRUCTION MEANS AND METHOD SHALL BE USED.
28. THE PROVISIONS OF CFC CHAPTER 14 AND CBC CHAPTER 33 SHALL BE ENFORCED ON THIS PROJECT.
29. ALL ASTM SPECIFICATIONS NOTED ON THESE DRAWINGS SHALL BE OF THE LATEST EDITION.

APPLICABLE CODES

LIST OF 2019 CALIFORNIA CODE OF REGULATIONS (C.C.R.):
APPLICABLE CODE AS OF JANUARY 1, 2020

- PART 1- 2018 CALIFORNIA BUILDING STANDARD ADMINISTRATIVE CODE, TITLE 24 C.C.R.
- PART 2- 2018 CALIFORNIA BUILDING CODE, TITLE 24 C.C.R. (2018 INTERNATIONAL BUILDING CODE OF THE INTERNATIONAL CODE COUNCIL, WITH CALIFORNIA AMENDMENTS)

DSA EXEMPT

1. THIS PROJECT IS EXEMPT FROM DSA REVIEW, APPROVAL AND CERTIFICATION IN ACCORDANCE WITH DSA IR A-22, PARAGRAPH 1.3.1.
2. ALL WORK SHALL COMPLY WITH APPLICABLE CODES AS LISTED.

SCOPE OF WORK

1. DEMOLISH 1ST BASE AND 3RD BASE DUGOUTS AS INDICATED ON DRAWINGS.
2. CONSTRUCT CMU REAR WALLS.
3. CONSTRUCT STORM DRAINAGE AND NEW CONCRETE SLABS.
4. CONSTRUCT FRAMING AND ROOF DECK AS INDICATED ON DRAWINGS.
5. PROVIDE ACCESSORIES AS INDICATED INCLUDING FENCING, MILLWORK AND PAINTING.

DESIGN TEAM

ARCHITECT

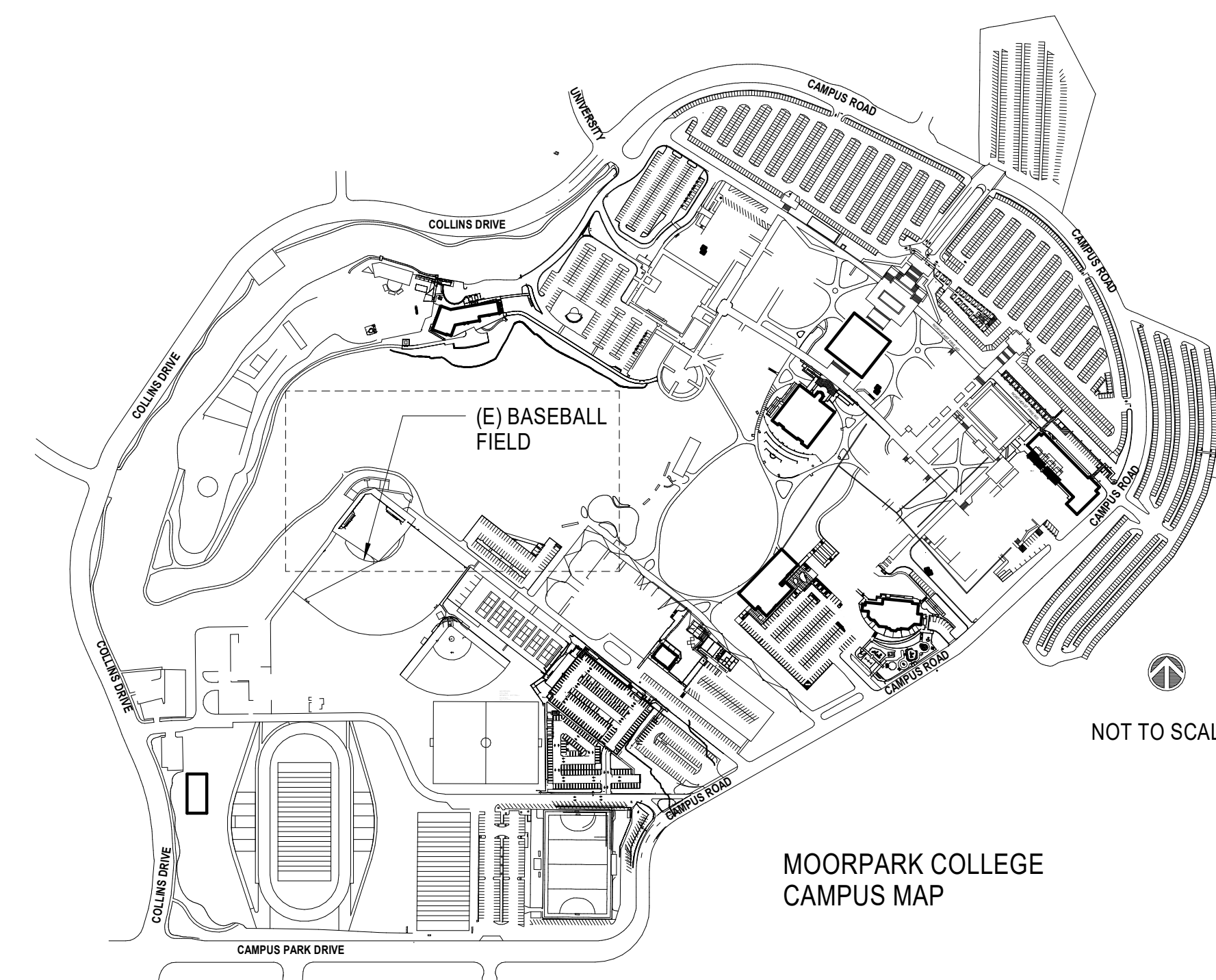
AMADOR WHITTLE ARCHITECTS, INC.
28328 AGOURA ROAD, SUITE 200
AGOURA HILLS, CA 91301
(805) 530-3938 OR (818) 874-0071

STRUCTURAL ENGINEER

ORION STRUCTURAL GROUP, INC.
223 EAST THOUSAND OAKS BLVD., SUITE 304
THOUSAND OAKS, CA 91360
(805) 390-9242

DRAWING LIST

SHT NO.	DRAWING TITLE
ARCHITECTURAL	
A101	SITE PLAN
A201	1ST BASE DUGOUT PLANS
A202	3RD BASE DUGOUT PLANS
A301	1ST BASE DUGOUT ELEVATIONS
A302	3RD BASE DUGOUT ELEVATIONS
A401	1ST BASE DUGOUT SECTIONS
A402	3RD BASE DUGOUT SECTIONS
A501	DETAILS
A502	DETAILS
STRUCTURAL	
S000	GENERAL NOTES
S001	GENERAL NOTES
S010	TYPICAL DETAILS
S011	TYPICAL DETAILS
S020	TYPICAL WOOD DETAILS
S100	STRUCTURAL PLANS
S200	STRUCTURAL DETAILS
S201	STRUCTURAL DETAILS

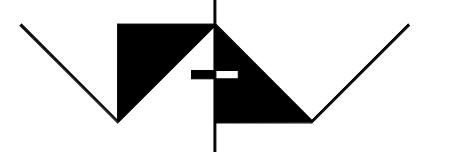


PROJECT TITLE
20-MPC-037

**REPAIR BASEBALL
DUGOUTS**

7075 CAMPUS ROAD
MOORPARK, CALIFORNIA 93021

COMMISSIONED ARCHITECT



**AMADOR WHITTLE
ARCHITECTS, INC.**

28328 AGOURA ROAD, SUITE 203
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CONSULTANT

STAMPS/SEALS



Project Status

SHEET TITLE:

TITLE SHEET

PROJECT NO. 020-MPC-037 PROJECT ARCH. Designer
DRAWN: LJA CHECKED: WJA

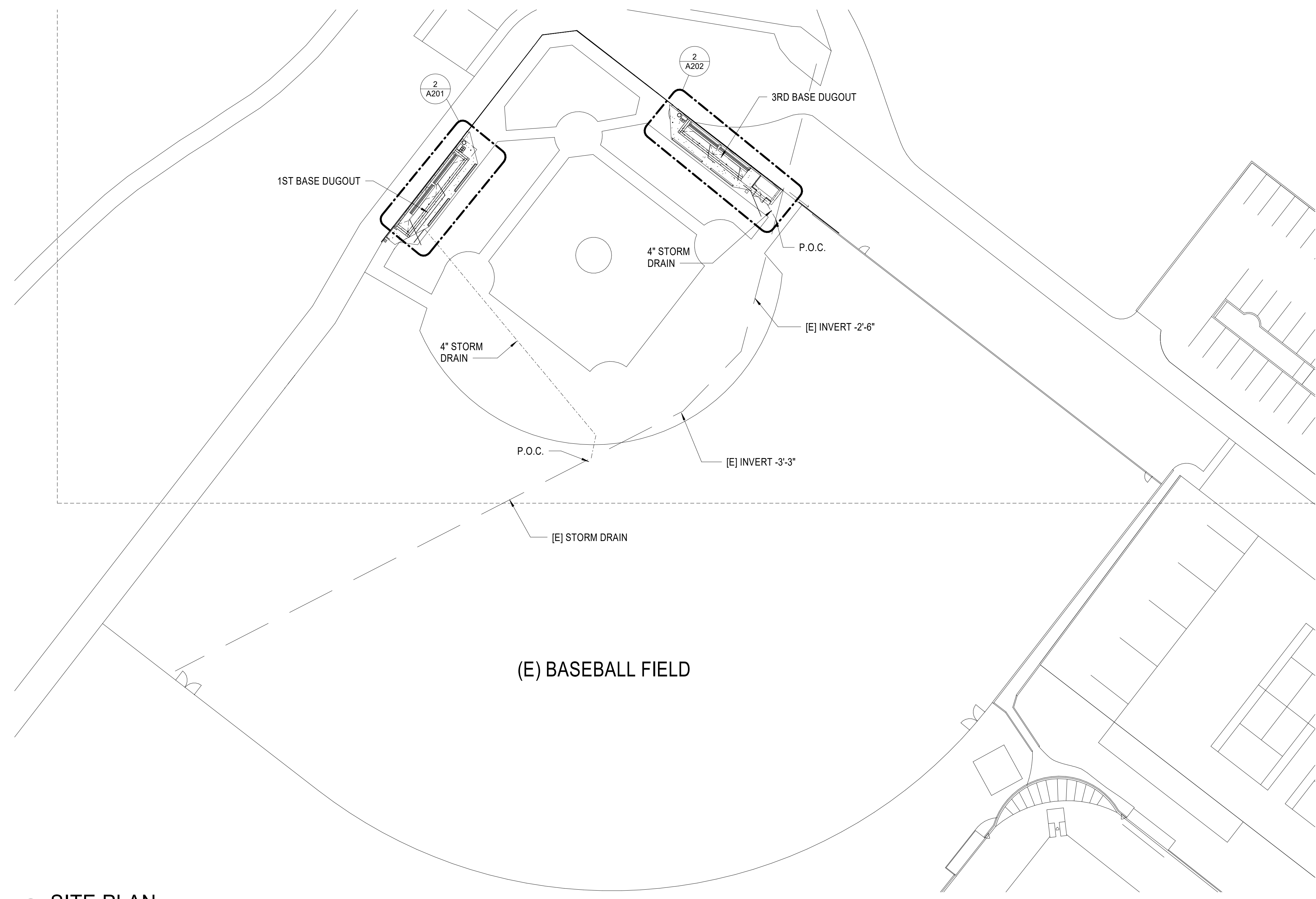
SHEET NUMBER:

G000

DATE: 6/28/21 SHEET: ___ OF ___

GENERAL NOTES

1. ALL ITEMS SHOWN ARE NEW UNLESS NOTED AS EXISTING.
2. SCHEDULING OF STORM DRAIN WORK ON FIELD SHALL BE CLOSELY COORDINATED WITH MOORPARK COLLEGE STAFF. TURF AND INFIELD SHALL BE FULLY RESTORED TO ORIGINAL CONDITION AFTER INSTALLATION OF STORM DRAIN LINES.



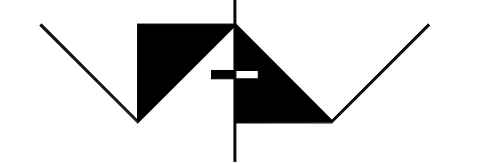
1 SITE PLAN
1" = 30'-0"

PROJECT TITLE
20-MPC-037

REPAIR BASEBALL DUGOUTS

7075 CAMPUS ROAD
MOORPARK, CALIFORNIA 93021

COMMISSIONED ARCHITECT



AMADOR WHITTLE ARCHITECTS, INC.

28328 AGOURA ROAD, SUITE 203
AGOURA HILLS, CA 91301
(805) 530-3938 , (818) 874-0071

CONSULTANT

STAMPS/SEALS



Project Status

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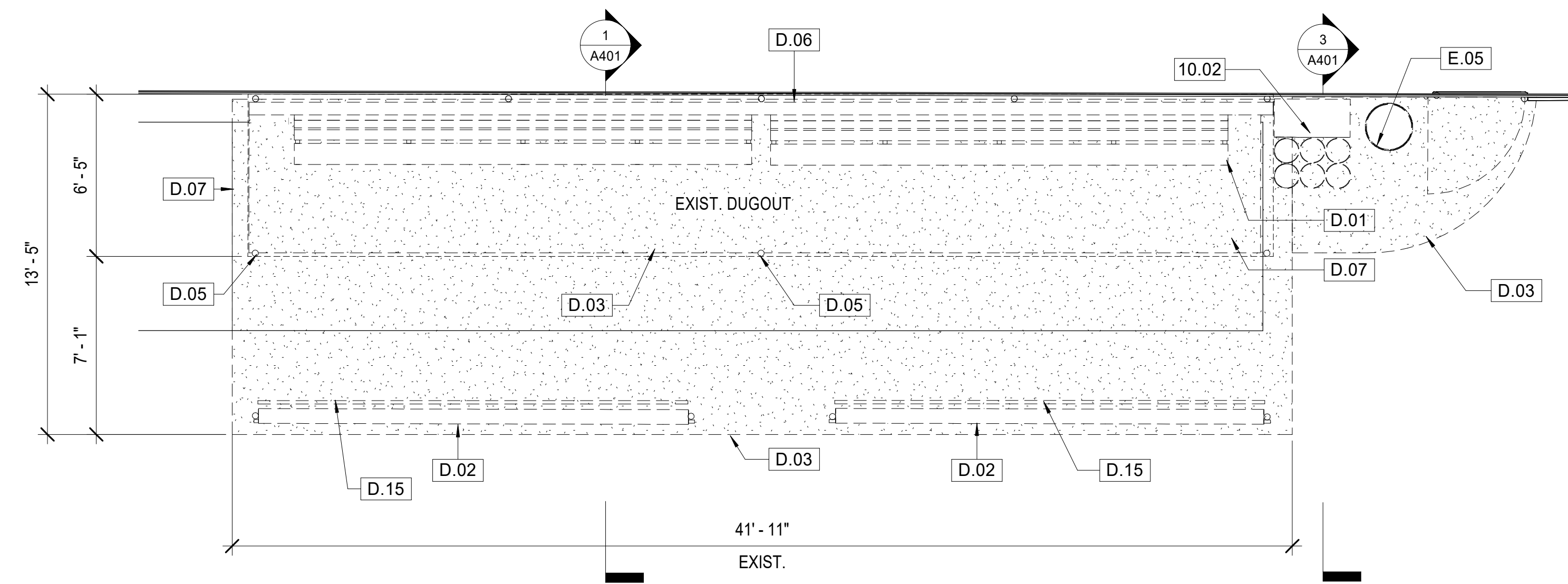
SITE PLAN

PROJECT NO.	020-MPC-037	PROJECT ARCH.	Designer
DRAWN	SAN	CHECKED	WJA

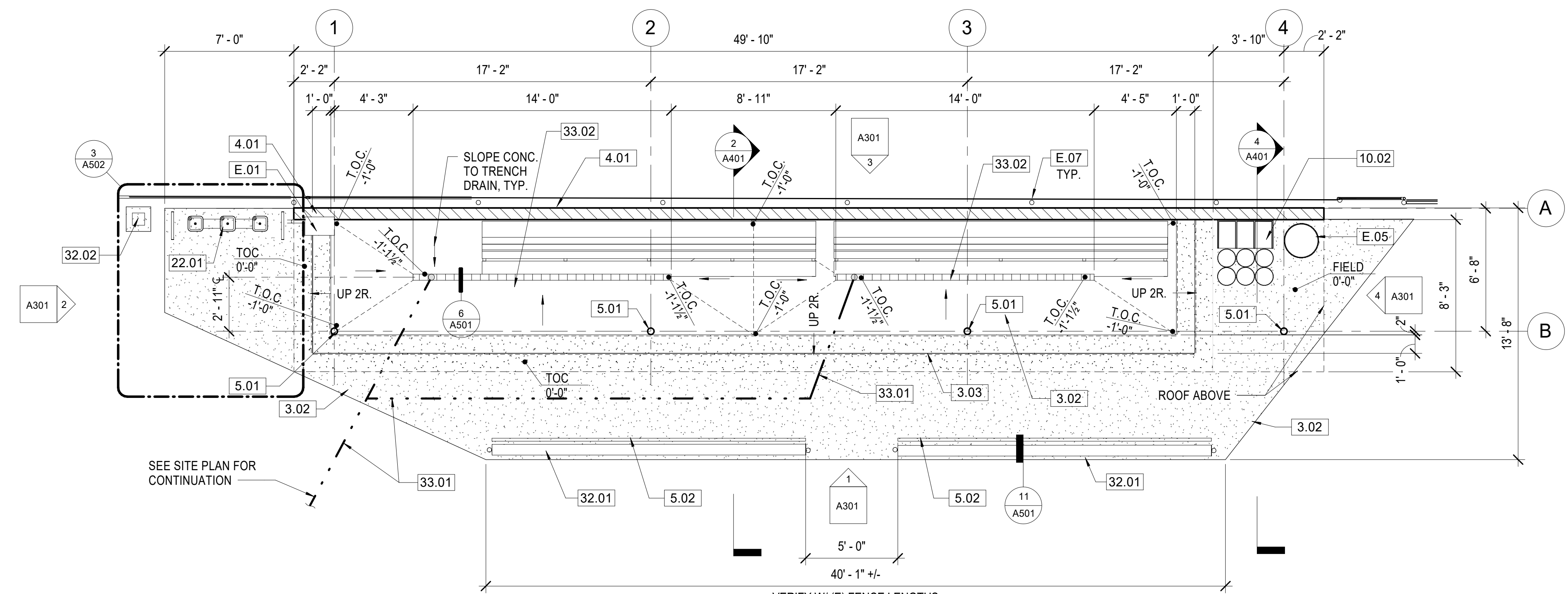
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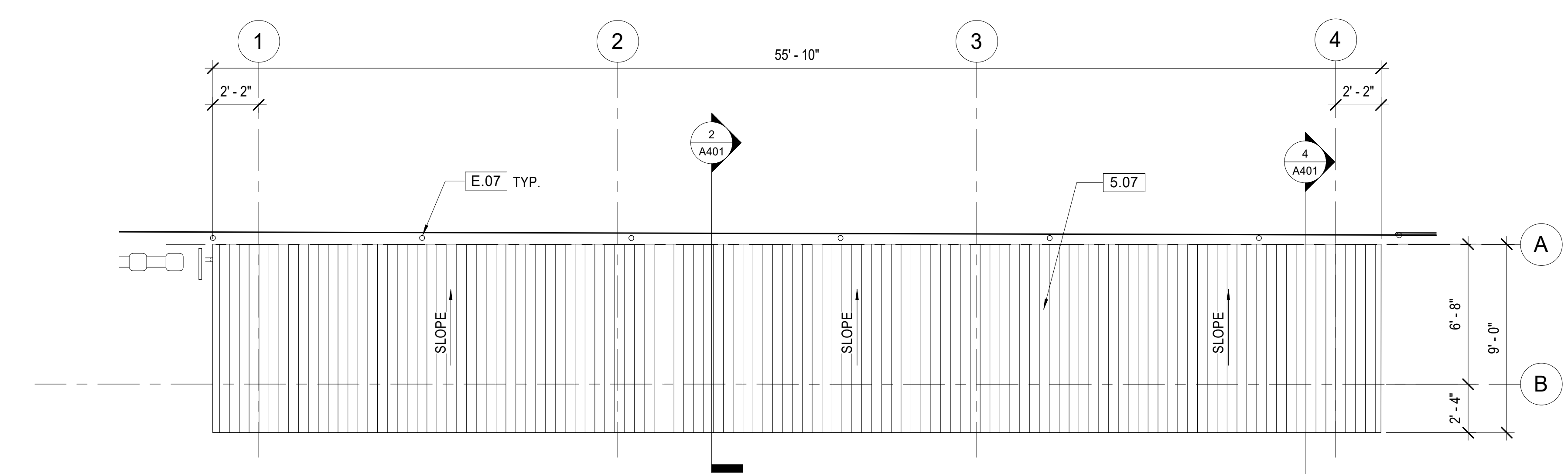
DATE: 6/28/21 SHEET: ___ OF ___



1 1ST BASE DUGOUT DEMOLITION PLAN
1/4" = 1'-0"



2 1ST BASE DUGOUT FLOOR PLAN
1/4" = 1'-0"



3 1ST BASE DUGOUT ROOF PLAN
1/4" = 1'-0"

DEMOLITION KEYNOTES

- D.01 CUT (E) BENCH & FLOOR LINE, STORE FOR REPAIR AND REUSE
- D.02 REMOVE & STORE FOR REUSE (E) SLATTED CHAIN LINK FENCING & TOP RAIL CAP GUARD
- D.03 DEMOLISH (E) 4" CONCRETE SLAB ON GRADE WITH #4 REBAR @ 16" O.C. EA. WAY
- D.05 DEMOLISH (E) STEEL POSTS & (E) 4X12 WOOD BEAMS
- D.06 DEMOLISH (E) PLYWOOD & 2x4 STUD WALL
- D.07 DEMOLISH (E) BUILT UP ROOFING OVER PLYWOOD ON 2X4 ROOF JOIST @ 2'-0" O.C.
- D.15 REMOVE AND REINSTALL (E) STEEL FOOT RAIL

EXISTING KEYNOTES

- E.01 (E) IRRIGATION CONTROLLER, PROTECT DURING DEMO & CONSTRUCTION. ANCHOR TO CMU WALL
- E.05 (E) TRASH CAN
- E.07 (E) FENCE POST

KEYNOTE LEGEND

- 3.02 CONCRETE SLAB W/ #4 REBAR @ 16" O.C. EA. WAY, SEE STRUCT. DWGS.
- 3.03 CONCRETE STEPS, SEE STRUCT. DWGS. & 2/A501
- 4.01 8" X 8" X 16" PRECISION CMU W/ REBAR, SEE STRUCT. DWGS.
- 5.01 PAINTED STEEL POST & BEAM BUCKET, SEE STRUCT. DWGS.
- 5.02 REINSTALL (E) STEEL FOOT RAIL
- 5.07 PAINTED BONDERIZED METAL DECK, SEE STRUCT. DWGS.
- 10.02 REMOVE AND REINSTALL (E) HELMET & BAT RACK
- 22.01 FREESTANDING HI-LO DRINKING FOUNTAIN WITH HYDRATION STATION AND FILTER. HAWS MODEL 3612, FILTER 6425
- 32.01 REINSTALL (E) TOP RAIL GUARD AND FENCE
- 32.02 DRYWELL FOR DRINKING FOUNTAIN DRAINAGE, SEE 1/A502
- 33.01 4" STORM DRAIN
- 33.02 TRENCH DRAIN, SEE 6/A501. OWNER FURNISHED, CONTRACTOR INSTALLED. CONTRACTOR TO PROVIDE TRANSITIONS TO STORM DRAIN LINE WITH STRAINERS

FINISHES

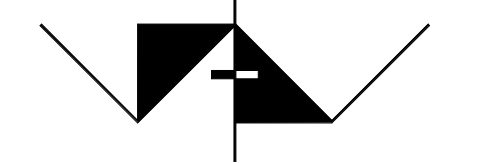
THE FOLLOWING NEW AND EXISTING SURFACES SHALL BE PAINTED:
 CMU
 STEEL (UNLESS NOTED TO BE GALVANIZED)
 MISCELLANEOUS METALS
 SHEET METAL (UNLESS PREFINISHED)
 WOOD AND PLYWOOD
 DOORS
 FENCE POSTS
 CHAIN LINK FENCING SHALL BE VINYL COATED

PROJECT TITLE
20-MPC-037

REPAIR BASEBALL DUGOUTS

7075 CAMPUS ROAD
MOORPARK, CALIFORNIA 93021

COMMISSIONED ARCHITECT



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(805) 530-3938 (818) 874-0071

CONSULTANT

STAMPS/SEALS



Project Status

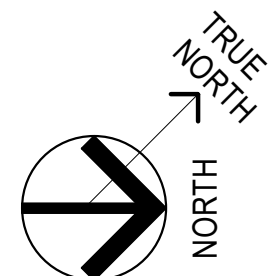
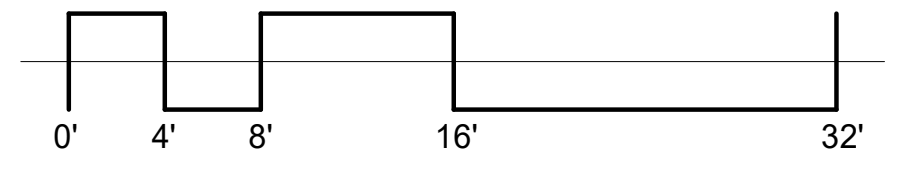
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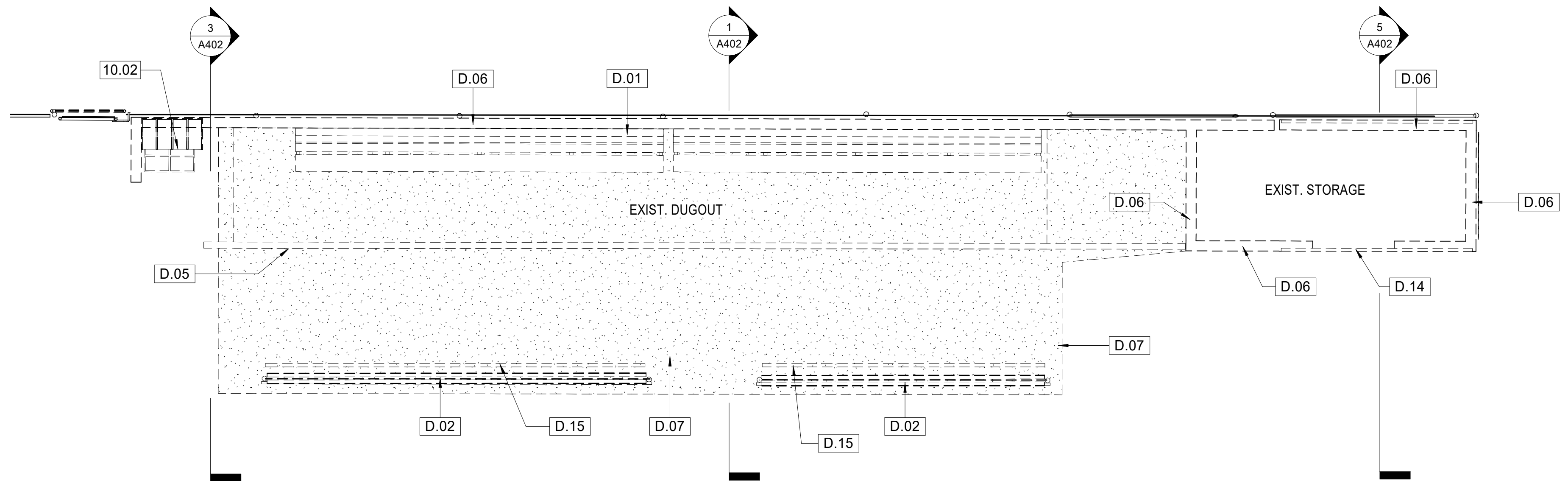
1ST BASE DUGOUT PLANS

PROJECT NO.	020-MPC-037	PROJECT ARCH.	Designer
DRAWN	SAN	CHECKED	WJA
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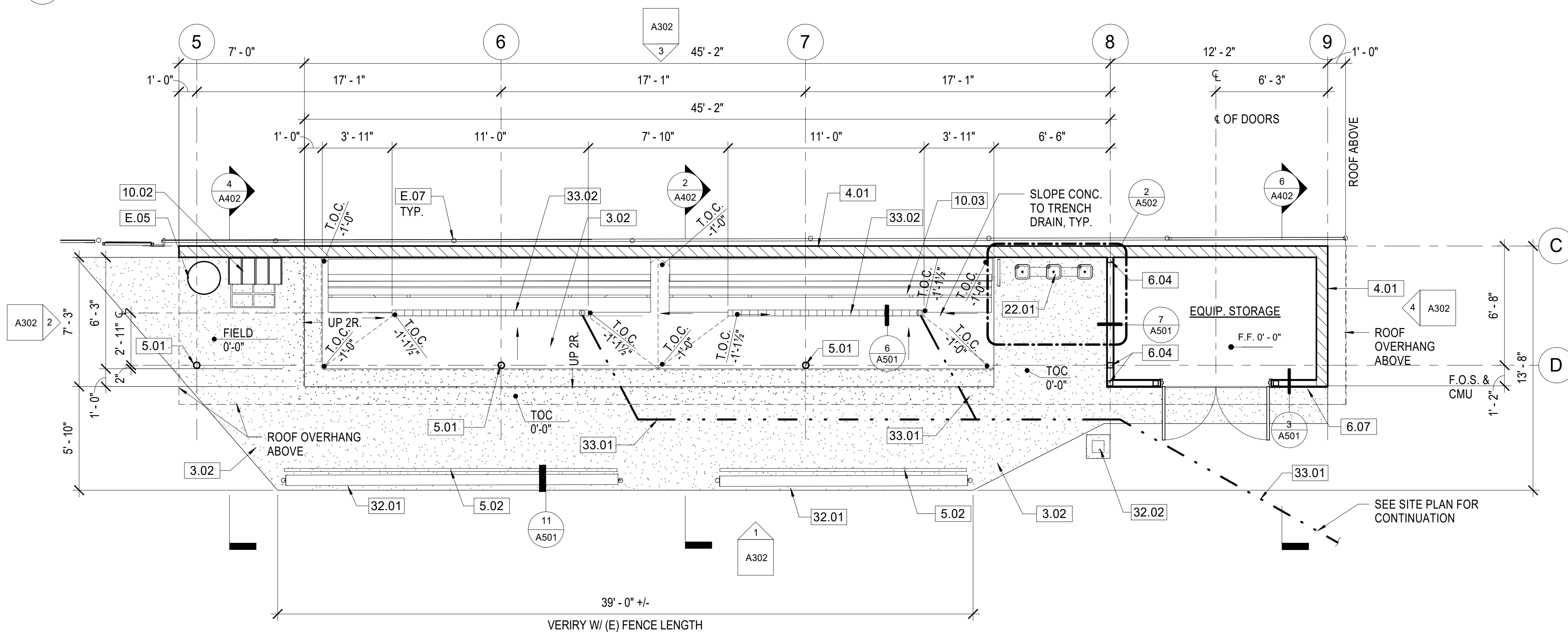
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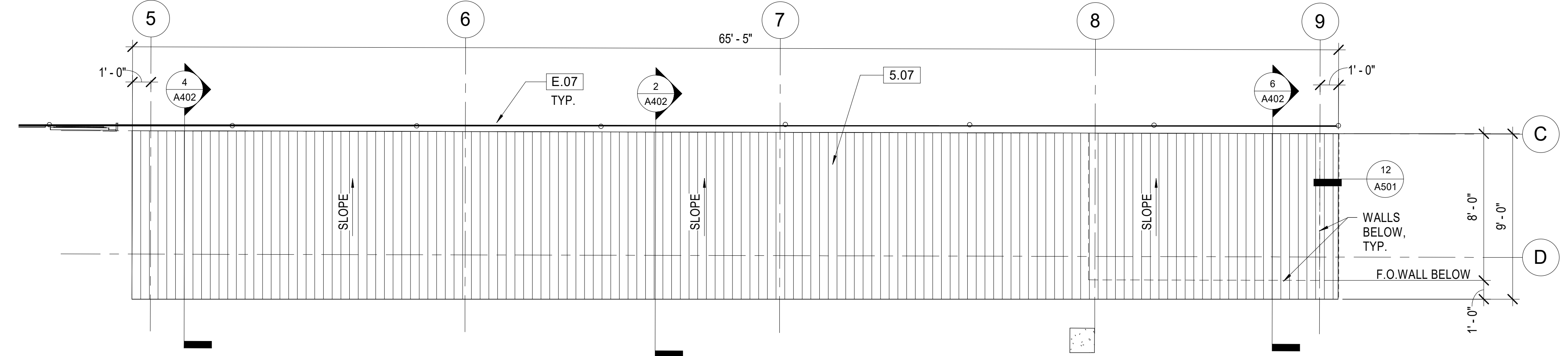




1 3RD BASE DUGOUT DEMOLITION PLAN
1/4" = 1'-0"



2 3RD BASE DUGOUT FLOOR PLAN
1/4" = 1'-0"



3 3RD BASE DUGOUT ROOF PLAN
1/4" = 1'-0"

DEMOLITION KEYNOTES

- D.01 CUT (E) BENCH & FLOOR LINE, STORE FOR REPAIR AND REUSE
- D.02 REMOVE & STORE FOR REUSE (E) SLATTED CHAIN LINK FENCING & TOP RAIL CAP GUARD
- D.05 DEMOLISH (E) STEEL POSTS & (E) 4X12 WOOD BEAMS
- D.06 DEMOLISH (E) PLYWOOD & 2X4 STUD WALL
- D.07 DEMOLISH (E) BUILT UP ROOFING OVER PLYWOOD ON 2X4 ROOF JOIST @ 2'-0" O.C.
- D.14 DEMOLISH (E) WOOD DOOR
- D.15 REMOVE AND REINSTALL (E) STEEL FOOT RAIL

EXISTING KEYNOTES

- E.05 (E) TRASH CAN
- E.07 (E) FENCE POST

KEYNOTE LEGEND

- 3.02 CONCRETE SLAB W/ #4 REBAR @ 16" O.C. EA. WAY, SEE STRUCT. DWGS.
- 4.01 8" X 8" X 16" PRECISION CMU W/ REBAR, SEE STRUCT. DWGS.
- 5.01 PAINTED STEEL POST & BEAM BUCKET, SEE STRUCT. DWGS.
- 5.02 REINSTALL (E) STEEL FOOT RAIL
- 5.07 PAINTED BONDERIZED METAL DECK, SEE STRUCT. DWGS.
- 6.04 4x4 WD. POST, SEE STRUCT. DWGS.
- 6.07 PAINTED EXTERIOR PLYWOOD SIDING ON STUD WALL, SEE 3/A501
- 10.02 REMOVE AND REINSTALL (E) HELMET & BAT RACK
- 10.03 REMOVE AND REINSTALL (E) BENCH: EXTEND TOP, SEE 4/A501
- 22.01 FREESTANDING HI-LO DRINKING FOUNTAIN WITH HYDRATION STATION AND FILTER. HAWS MODEL 3612, FILTER 6425
- 32.01 REINSTALL (E) TOP RAIL GUARD AND FENCE
- 32.02 DRYWELL FOR DRINKING FOUNTAIN DRAINAGE, SEE 1/A502
- 33.01 4" STORM DRAIN
- 33.02 TRENCH DRAIN, SEE 6/A501. OWNER FURNISHED, CONTRACTOR INSTALLED. CONTRACTOR TO PROVIDE TRANSITIONS TO STORM DRAIN LINE WITH STRAINERS

FINISHES

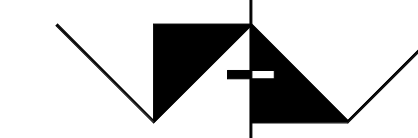
THE FOLLOWING NEW AND EXISTING SURFACES SHALL BE PAINTED:
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 WOOD AND PLYWOOD
 DOORS
 FENCE POSTS
 CHAIN LINK FENCING SHALL BE VINYL COATED

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CONSULTANT

STAMPS/SEALS



Project Status

SHEET TITLE:

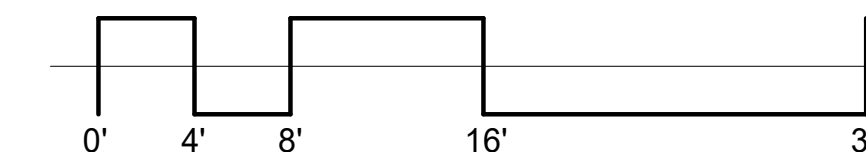
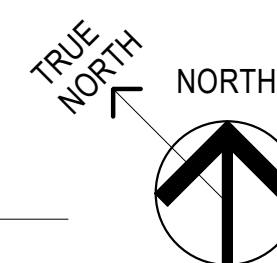
3RD BASE DUGOUT PLANS

PROJECT NO. 020-MPC-037 PROJECT ARCH. Designer
 DRAWN Author CHECKED: Checker

SHEET NUMBER:

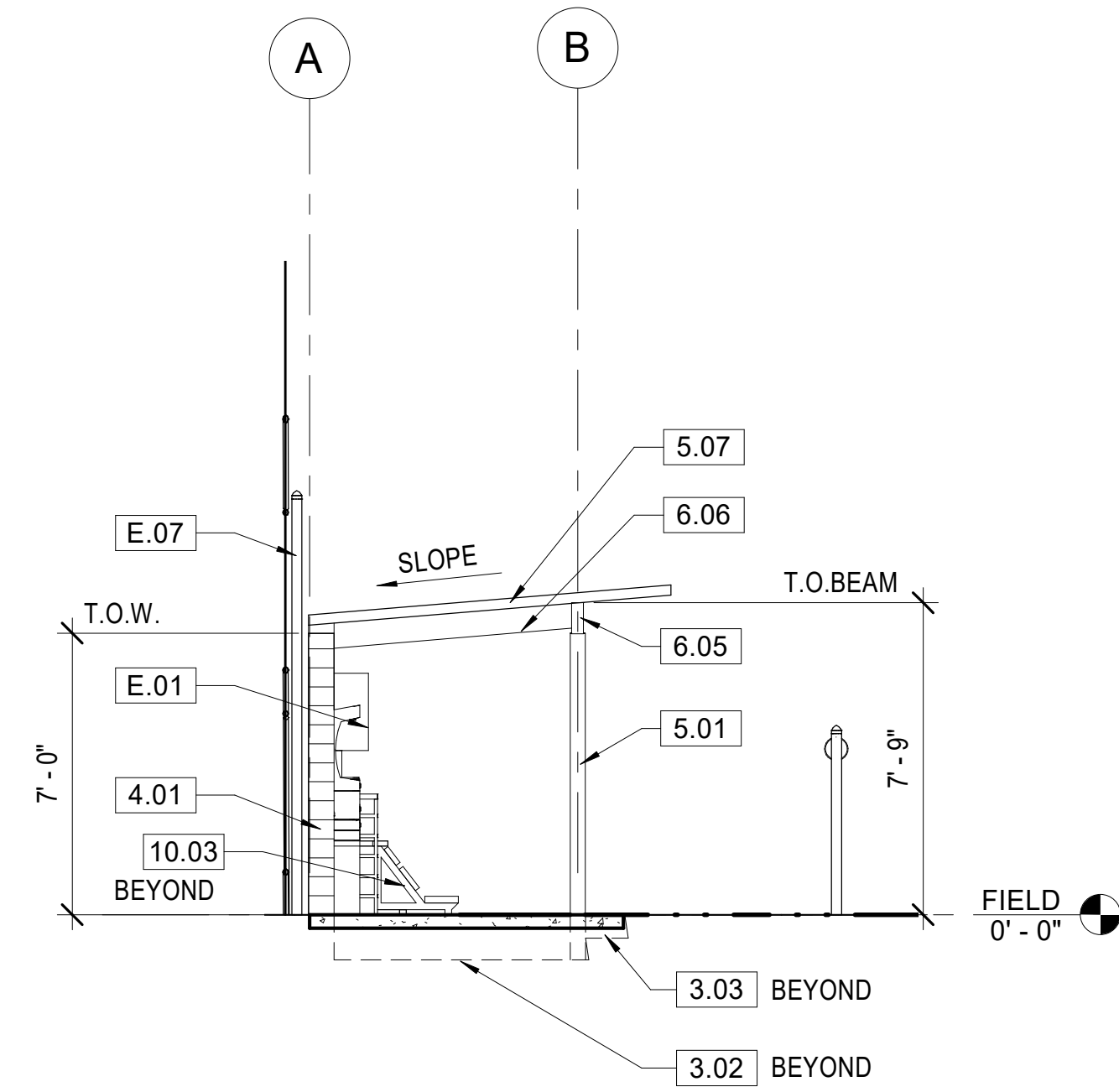
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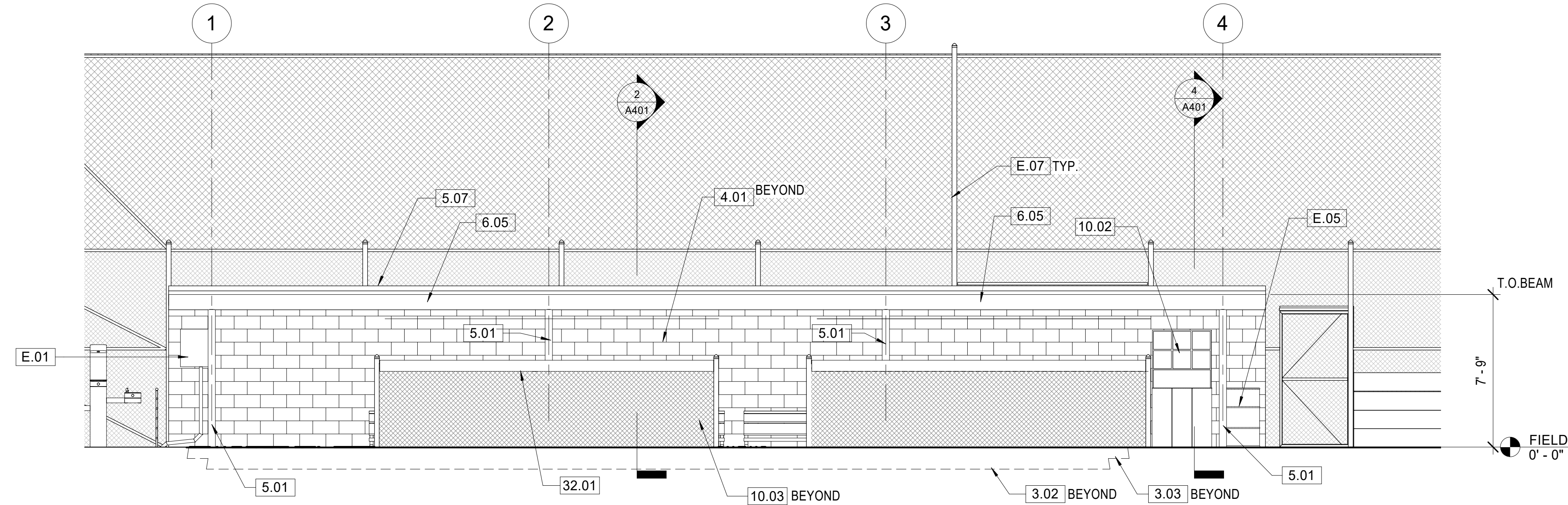


KEYNOTE LEGEND

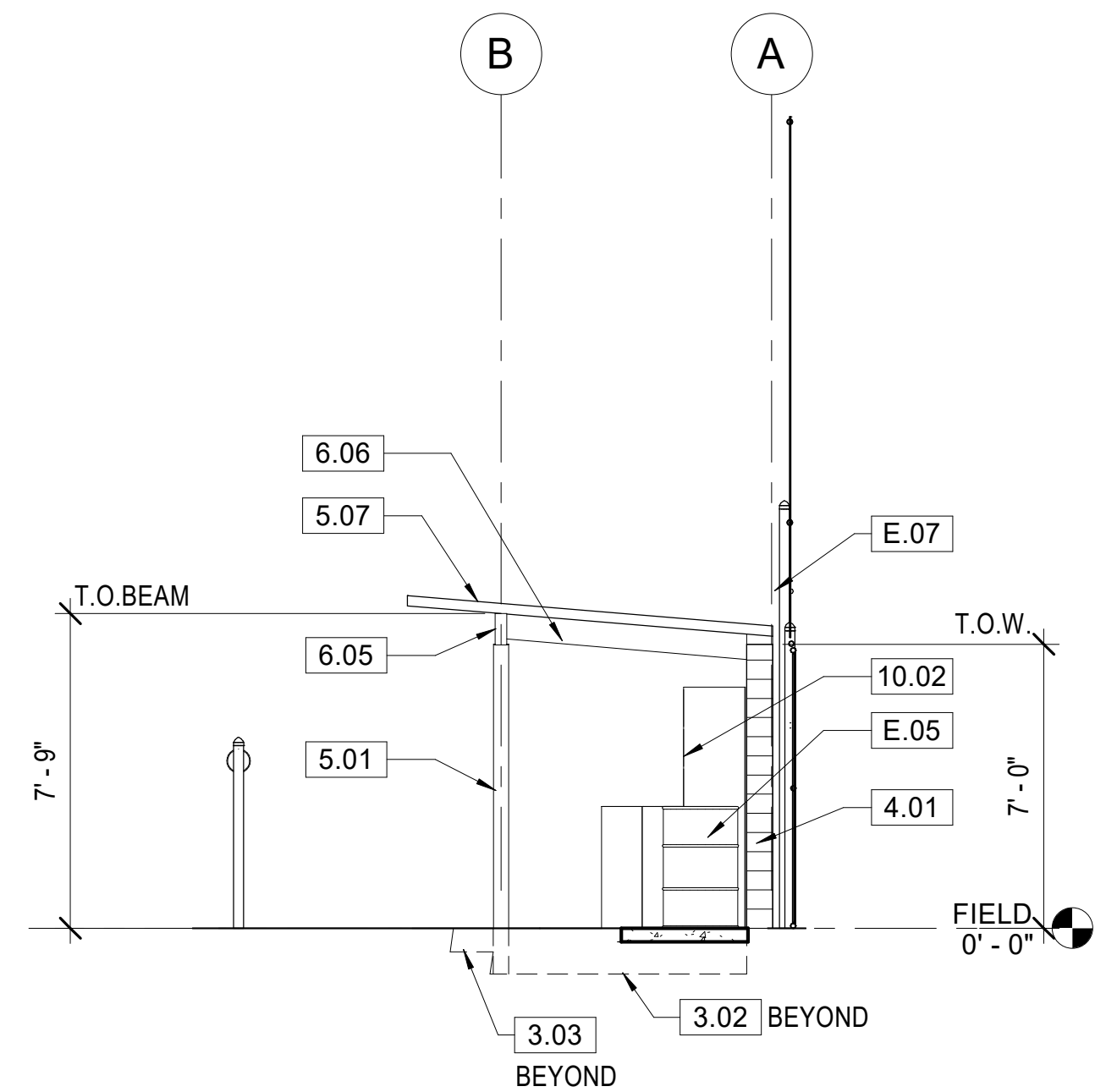
- 3.02 CONCRETE SLAB W/ #4 REBAR @ 16" O.C. EA. WAY, SEE STRUCT. DWGS.
- 3.03 CONCRETE STEPS, SEE STRUCT. DWGS. & 2/A501
- 4.01 8" X 8" X 16" PRECISION CMU W/ REBAR, SEE STRUCT. DWGS.
- 5.01 PAINTED STEEL POST & BEAM BUCKET, SEE STRUCT. DWGS.
- 5.07 PAINTED BONDERIZED METAL DECK. SEE STRUCT. DWGS.
- 6.03 3x P.T. WOOD NAILER, SEE STRUCT. DWGS.
- 6.05 PAINTED 4x10 WOOD BEAM, SEE STRUCT. DWGS.
- 6.06 PAINTED 4x8 WOOD BEAM, SEE STRUCT. DWGS.
- 10.02 REMOVE AND REINSTALL (E) HELMET & BAT RACK
- 10.03 REMOVE AND REINSTALL (E) BENCH: EXTEND TOP, SEE 4/A501
- 32.01 REINSTALL (E) TOP RAIL GUARD AND FENCE



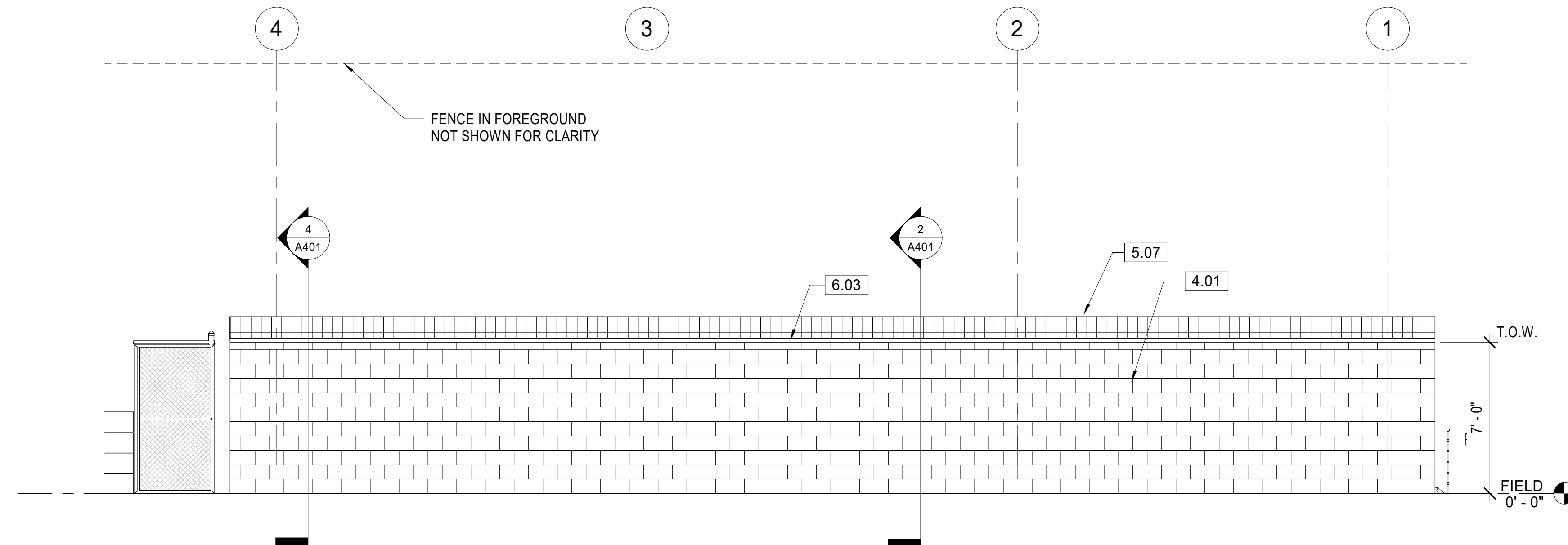
2 1ST BASE DUGOUT SOUTH ELEVATION
1/4" = 1'-0"



1 1ST BASE DUGOUT EAST ELEVATION
1/4" = 1'-0"



4 1ST BASE DUGOUT NORTH ELEVATION
1/4" = 1'-0"



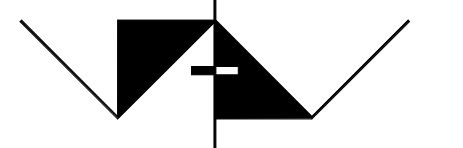
3 1ST BASE DUGOUT WEST ELEVATION
1/4" = 1'-0"

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SHEET TITLE:

1ST BASE DUGOUT ELEVATIONS

PROJECT NO. 020-MPC-037	PROJECT ARCH. Designer
DRAWN Author	CHECKED Checker

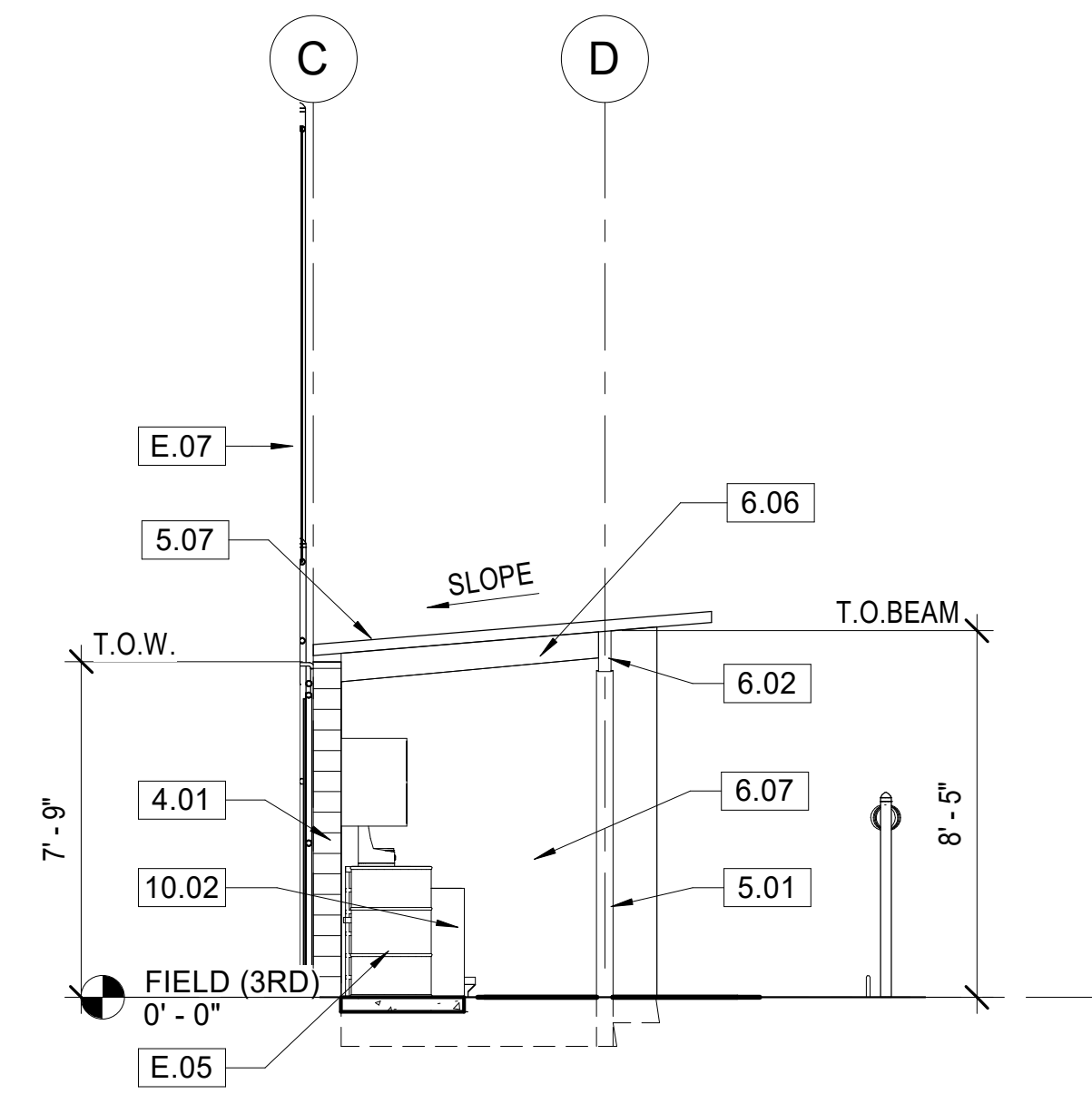
SHEET NUMBER:

A301

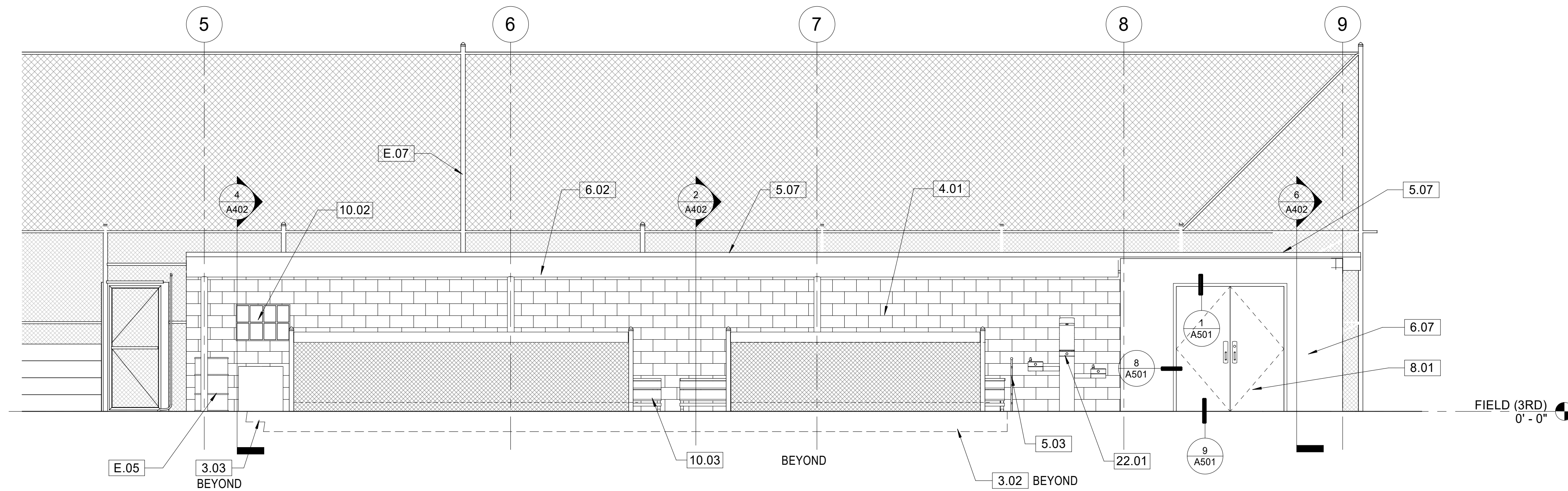
DATE: 6/28/21 SHEET: ___ OF ___

KEYNOTE LEGEND

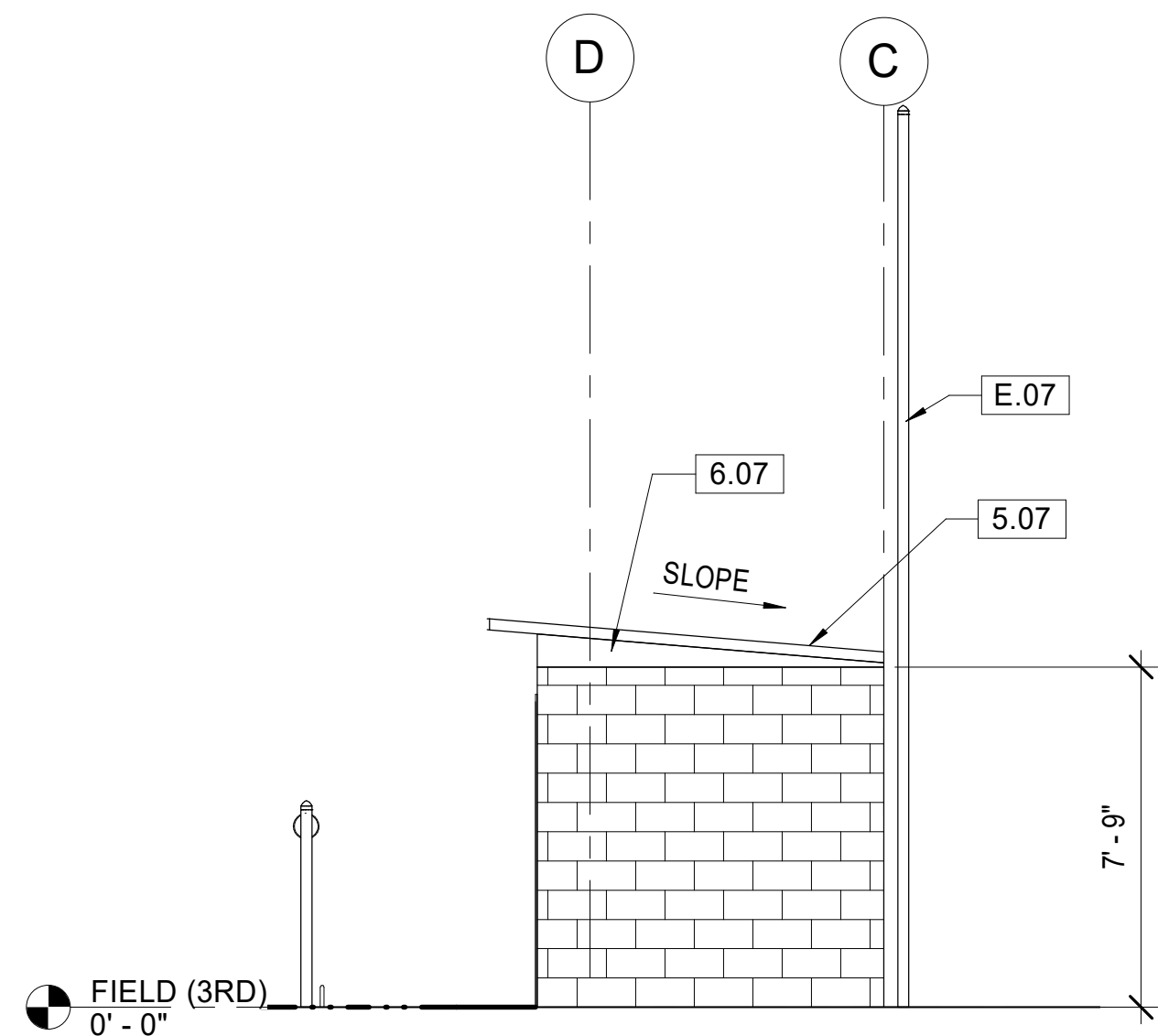
- 3.02 CONCRETE SLAB W/ #4 REBAR @ 16" O.C. EA. WAY, SEE STRUCT. DWGS.
- 3.03 CONCRETE STEPS, SEE STRUCT. DWGS. & 2/A501
- 4.01 8" X 8" X 16" PRECISION CMU W/ REBAR, SEE STRUCT. DWGS.
- 5.01 PAINTED STEEL POST & BEAM BUCKET, SEE STRUCT. DWGS.
- 5.03 METAL WING RAIL, SEE 4/A502
- 5.07 PAINTED BONDERIZED METAL DECK, SEE STRUCT. DWGS.
- 6.02 PAINTED 4x12 WD.BEAM, SEE STRUCT. DWGS.
- 6.06 PAINTED 4x8 WOOD BEAM, SEE STRUCT. DWGS.
- 6.07 PAINTED EXTERIOR PLYWOOD SIDING ON STUD WALL, SEE 3/A501
- 8.01 PAINTED PAIR 6'-0" x 7'-0" H.M. DOORS, LOCKABLE, SEE 1.8.9/A501
- 10.02 REMOVE AND REINSTALL (E) HELMET & BAT RACK
- 10.03 REMOVE AND REINSTALL (E) BENCH: EXTEND TOP, SEE 4/A501
- 22.01 FREESTANDING HI-LO DRINKING FOUNTAIN WITH HYDRATION STATION AND FILTER. HAWS MODEL 3612, FILTER 6425



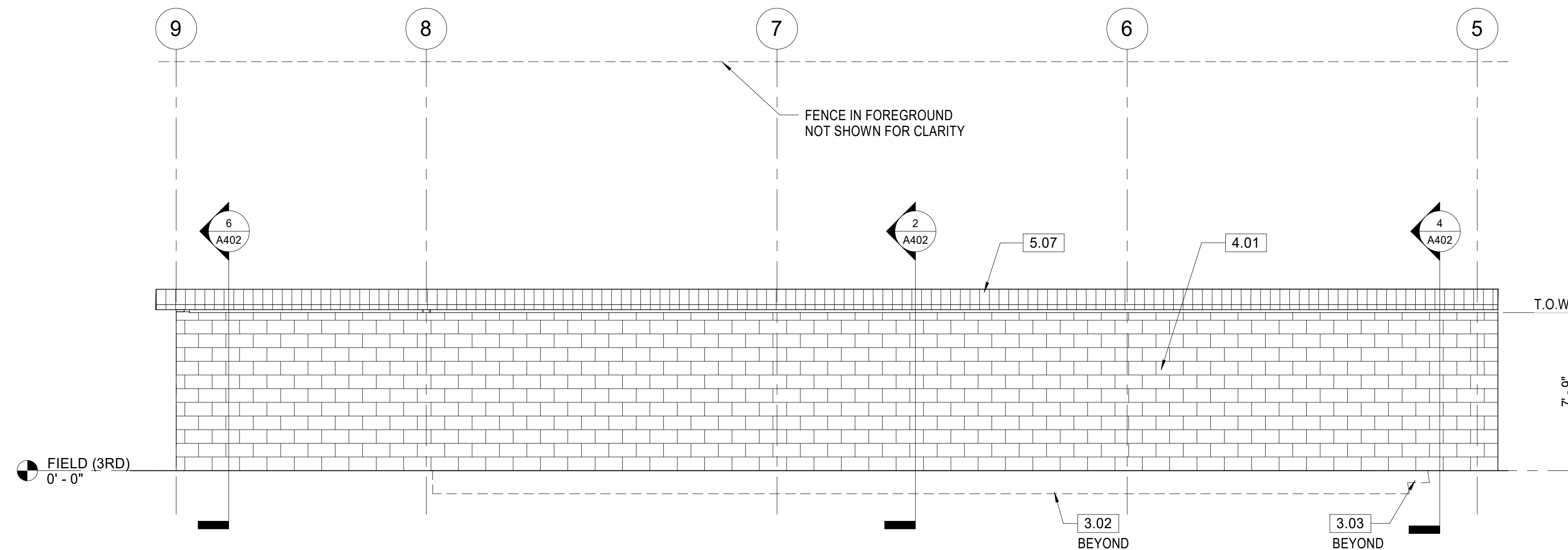
2 3RD BASE DUGOUT WEST ELEVATION
1/4" = 1'-0"



1 3RD BASE DUGOUT SOUTH ELEVATION
1/4" = 1'-0"



4 3RD BASE DUGOUT EAST ELEVATION
1/4" = 1'-0"

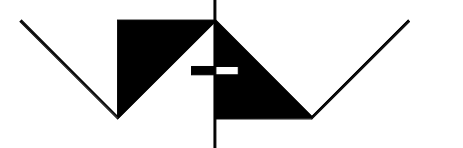


3 3RD BASE DUGOUT NORTH ELEVATION
1/4" = 1'-0"

PROJECT TITLE
20-MPC-037
**REPAIR BASEBALL
DUGOUTS**

7075 CAMPUS ROAD
MOORPARK, CALIFORNIA 93021

COMMISSIONED ARCHITECT



**AMADOR WHITTLE
ARCHITECTS, INC.**

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(805) 530-3938 | (818) 874-0071

CONSULTANT

STAMPS/SEALS



Project Status

SHEET TITLE:

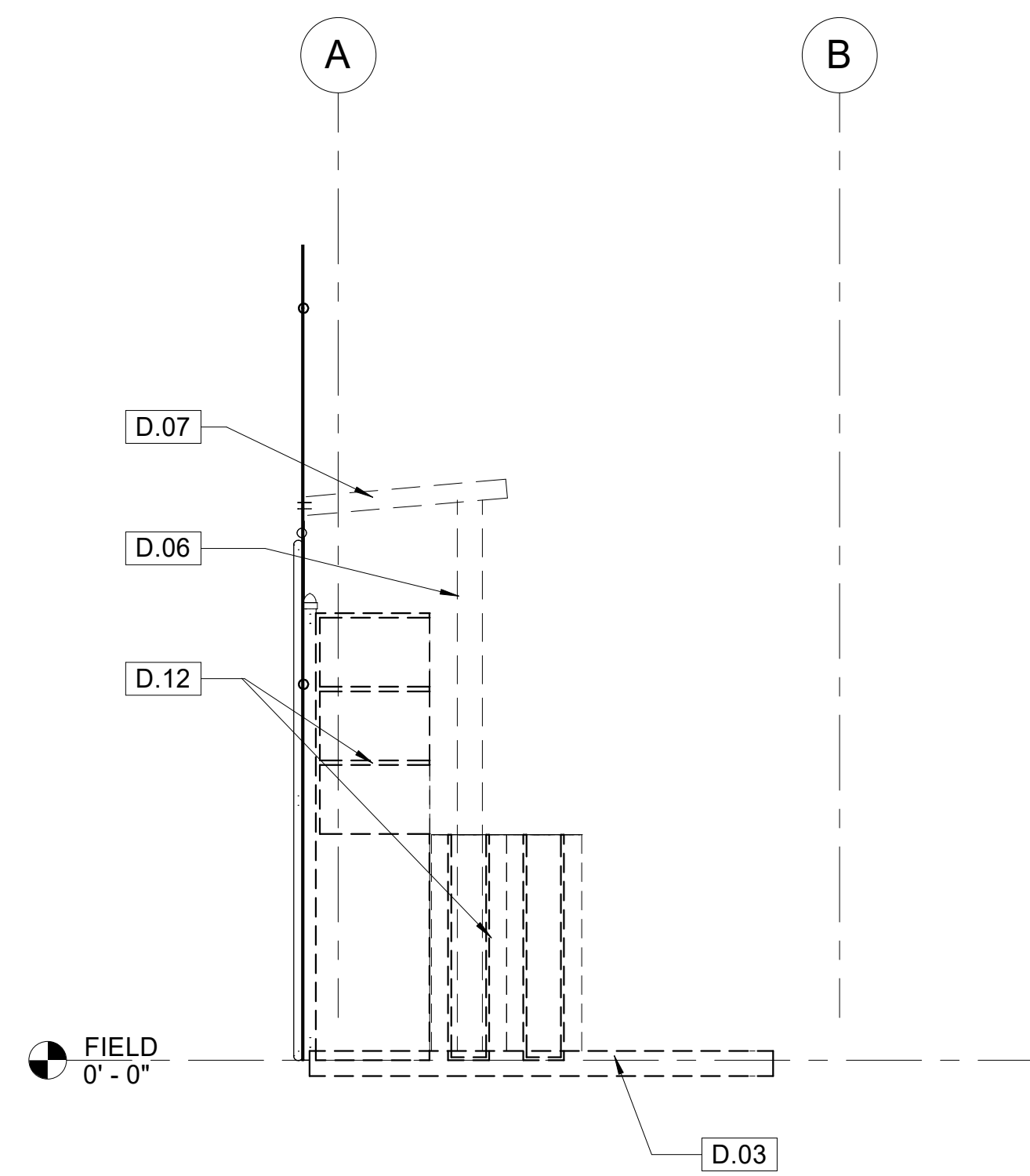
**3RD BASE DUGOUT
ELEVATIONS**

PROJECT NO. 020-MPC-037 PROJECT ARCH: Designer
DRAWN: Author CHECKED: Checker

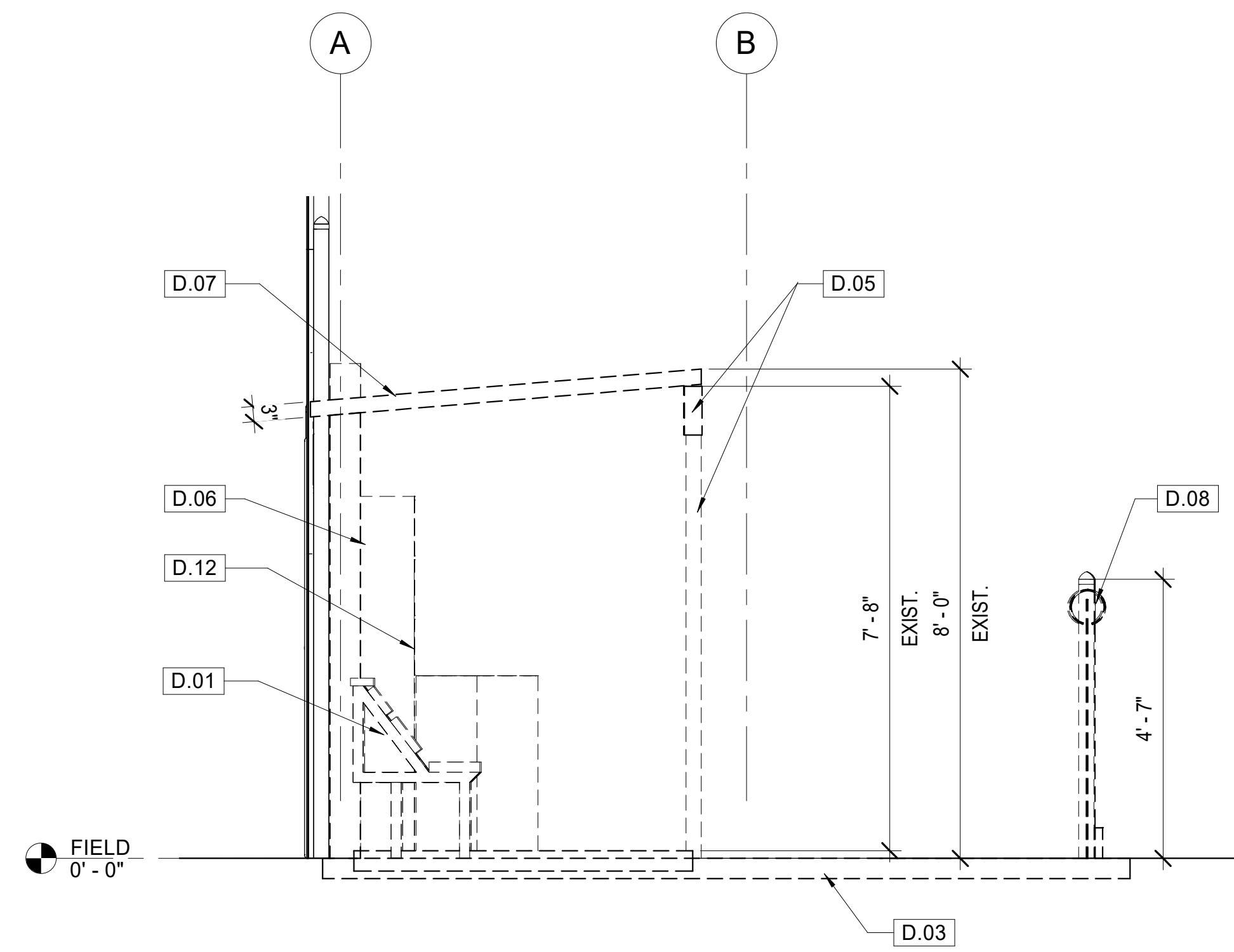
SHEET NUMBER:

A302

DATE: 6/28/21 SHEET: OF



3 1ST BASE - DEMO. SECTION @ EQUIPMENT
1/2" = 1'-0"



1 1ST BASE - DEMO. SECTION @ DUGOUT
1/2" = 1'-0"

DEMOLITION KEYNOTES

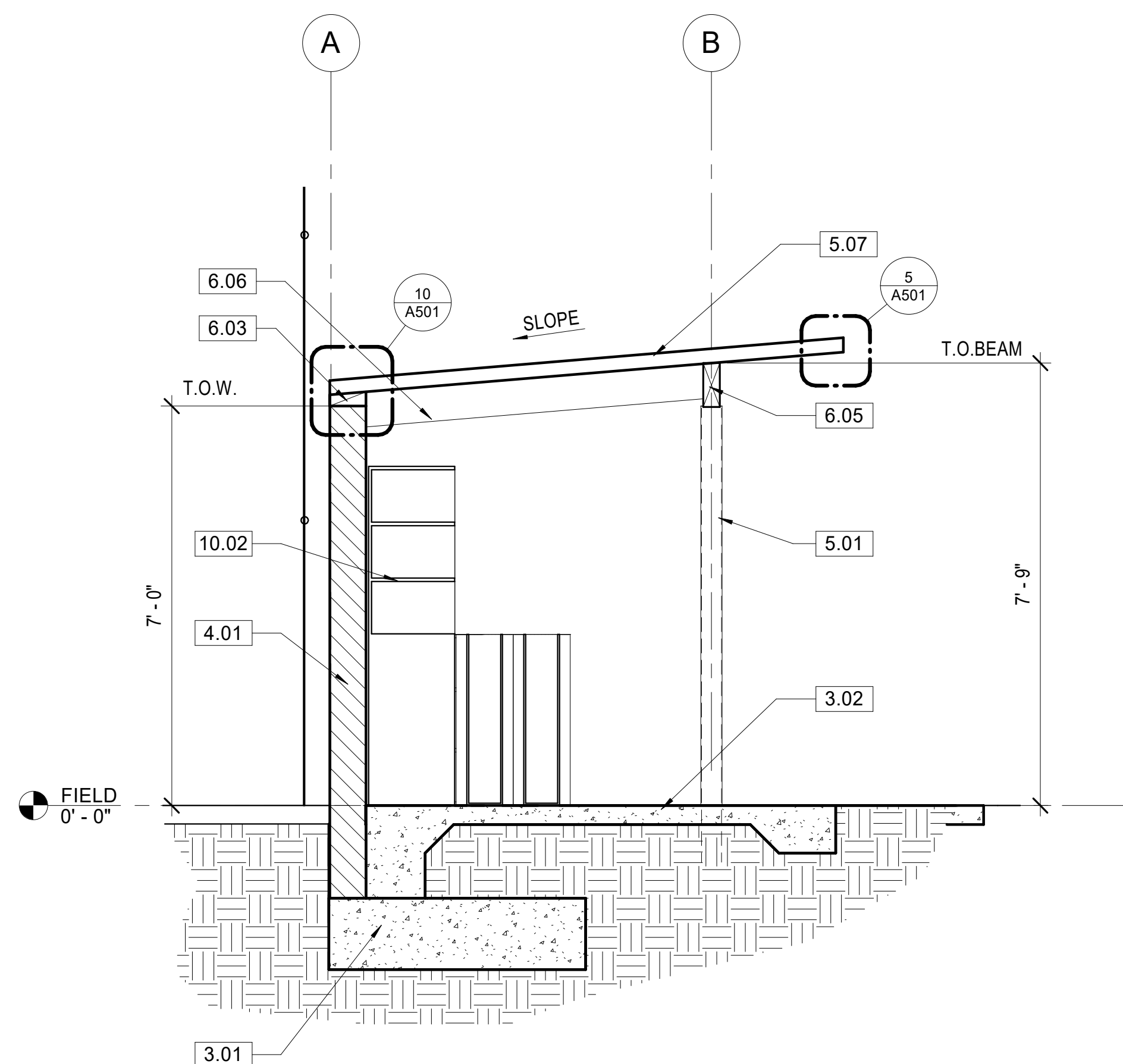
- D.01 CUT (E) BENCH & FLOOR LINE, STORE FOR REPAIR AND REUSE
- D.03 DEMOLISH (E) 4" CONCRETE SLAB ON GRADE WITH #4 REBAR @ 16" O.C. EA. WAY
- D.05 DEMOLISH (E) STEEL POSTS & (E) 4X12 WOOD BEAMS
- D.06 DEMOLISH (E) PLYWOOD & 2X4 STUD WALL
- D.07 DEMOLISH (E) BUILT UP ROOFING OVER PLYWOOD ON 2X4 ROOF JOIST @ 2'-0" O.C.
- D.08 REMOVE AND PROTECT TOP RAIL GUARD
- D.12 REMOVE AND REUSE (E) EQUIPMENT BAT AND HELMET STORAGE UNITS

EXISTING KEYNOTES

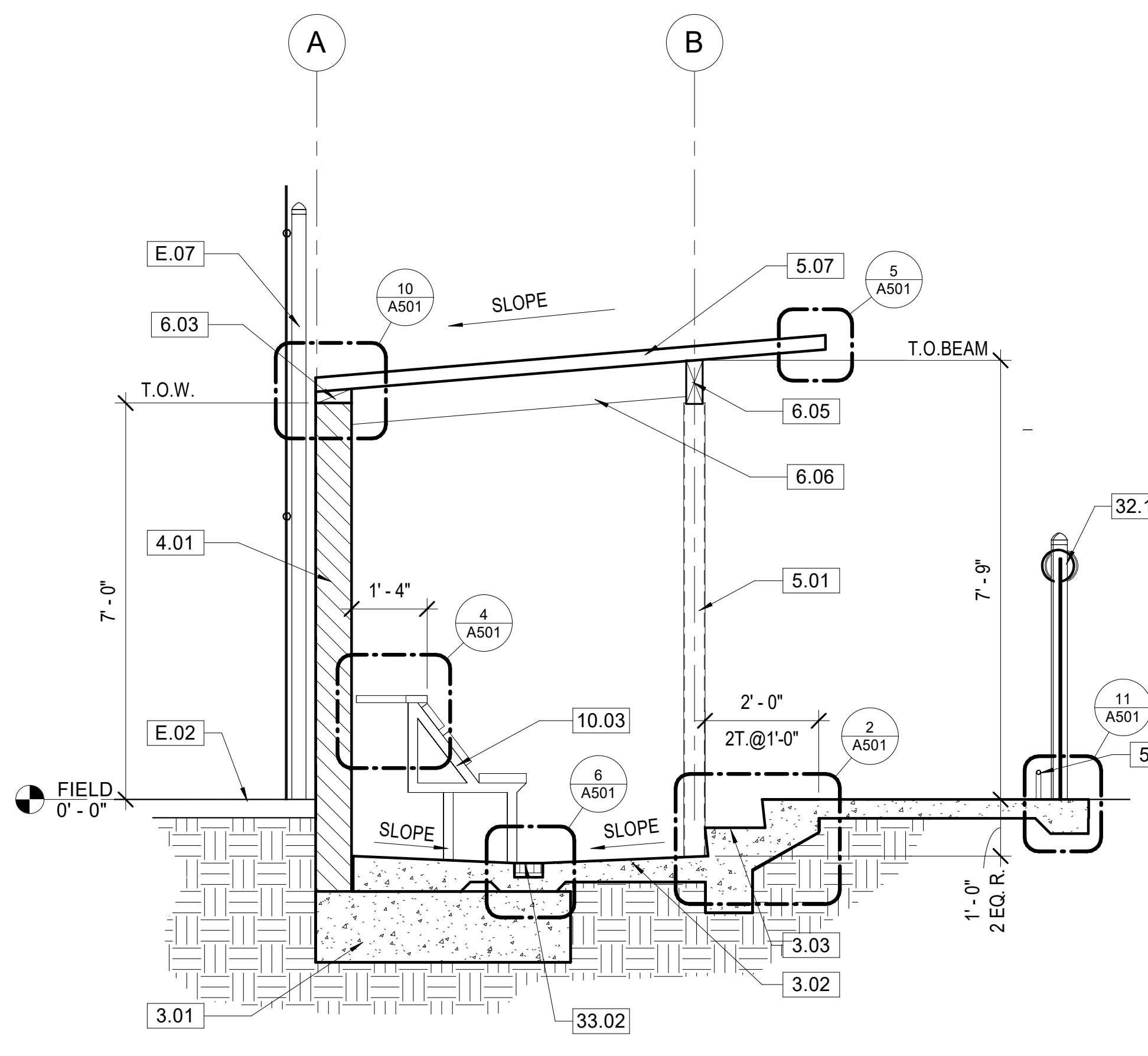
- E.02 (E) A/C PAVING
- E.07 (E) FENCE POST

KEYNOTE LEGEND

- 3.01 CONCRETE FOOTING, SEE STRUCT. DWGS.
- 3.02 CONCRETE SLAB W/ #4 REBAR @ 16" O.C. EA. WAY, SEE STRUCT. DWGS.
- 3.03 CONCRETE STEPS, SEE STRUCT. DWGS. & 2/A501
- 4.01 8" X 8" X 16" PRECISION CMU W/ REBAR, SEE STRUCT. DWGS.
- 5.01 PAINTED STEEL POST & BEAM BUCKET, SEE STRUCT. DWGS.
- 5.02 REINSTALL (E) STEEL FOOT RAIL
- 5.07 PAINTED BONDERIZED METAL DECK, SEE STRUCT. DWGS.
- 6.03 3x P.T. WOOD NAILER, SEE STRUCT. DWGS.
- 6.05 PAINTED 4x10 WOOD BEAM, SEE STRUCT. DWGS.
- 6.06 PAINTED 4x8 WOOD BEAM, SEE STRUCT. DWGS.
- 10.02 REMOVE AND REINSTALL (E) HELMET & BAT RACK
- 10.03 REMOVE AND REINSTALL (E) BENCH: EXTEND TOP, SEE 4/A501
- 32.10 REUSED (E) FENCE PADDING
- 33.02 TRENCH DRAIN, SEE 6/A501. OWNER FURNISHED, CONTRACTOR INSTALLED. CONTRACTOR TO PROVIDE TRANSITIONS TO STORM DRAIN LINE WITH STRAINERS



4 1ST BASE - EQUIP. STORAGE SECTION
1/2" = 1'-0"



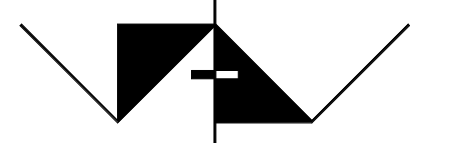
2 1ST BASE - DUGOUT SECTION
1/2" = 1'-0"

PROJECT TITLE
20-MPC-037

**REPAIR BASEBALL
DUGOUTS**

7075 CAMPUS ROAD
MOORPARK, CALIFORNIA 93021

COMMISSIONED ARCHITECT



**AMADOR WHITTLE
ARCHITECTS, INC.**

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(805) 530-3938 | (818) 874-0071

CONSULTANT

STAMPS/SEALS



Project Status

SHEET TITLE:

**1ST BASE DUGOUT
SECTIONS**

PROJECT NO.	020-MPC-037	PROJECT ARCH.	Designer
DRAWN	Author	CHECKED	Checker

SHEET NUMBER:

A401

DATE: 6/28/21 SHEET: ___ OF ___

DEMOLITION KEYNOTES

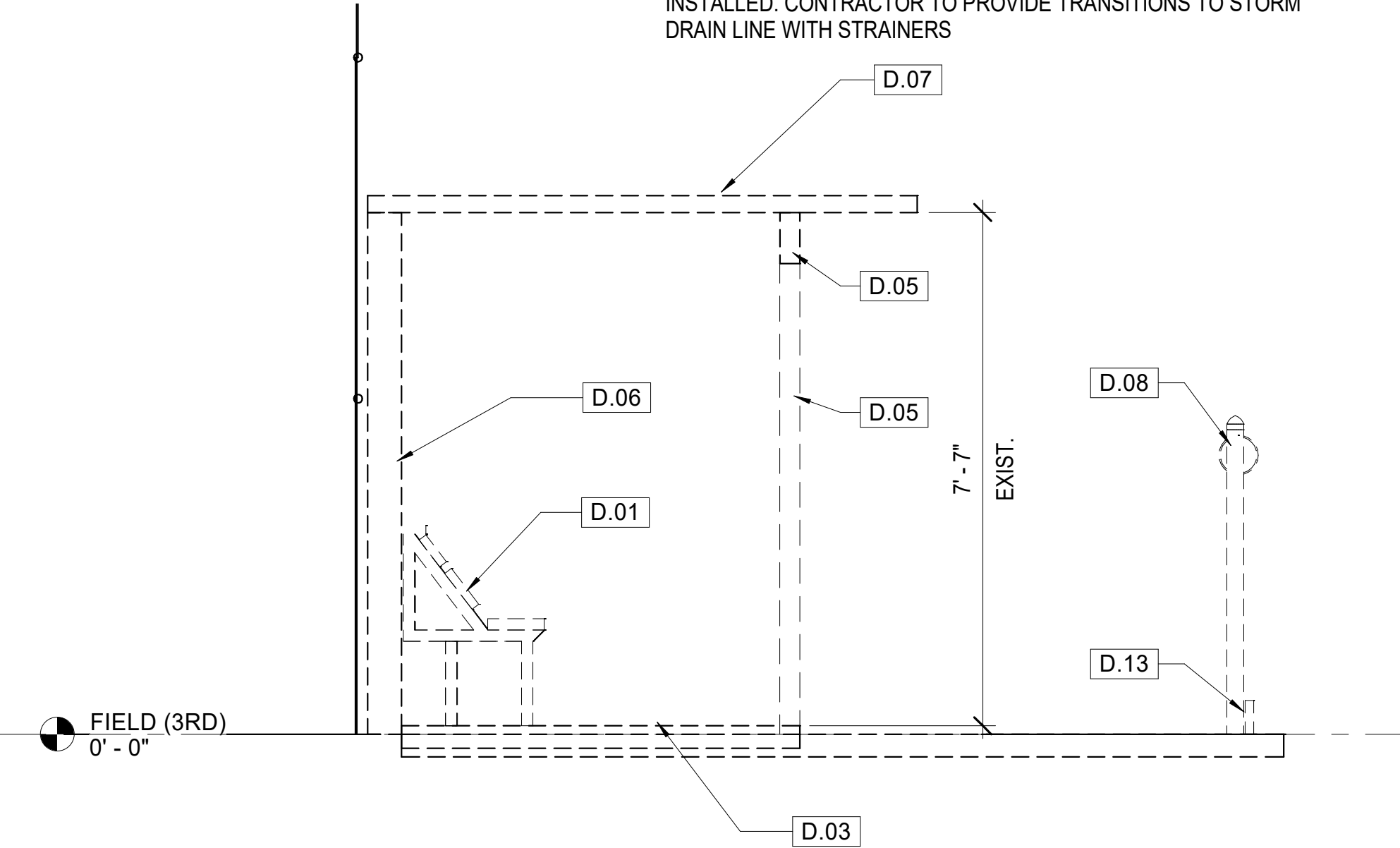
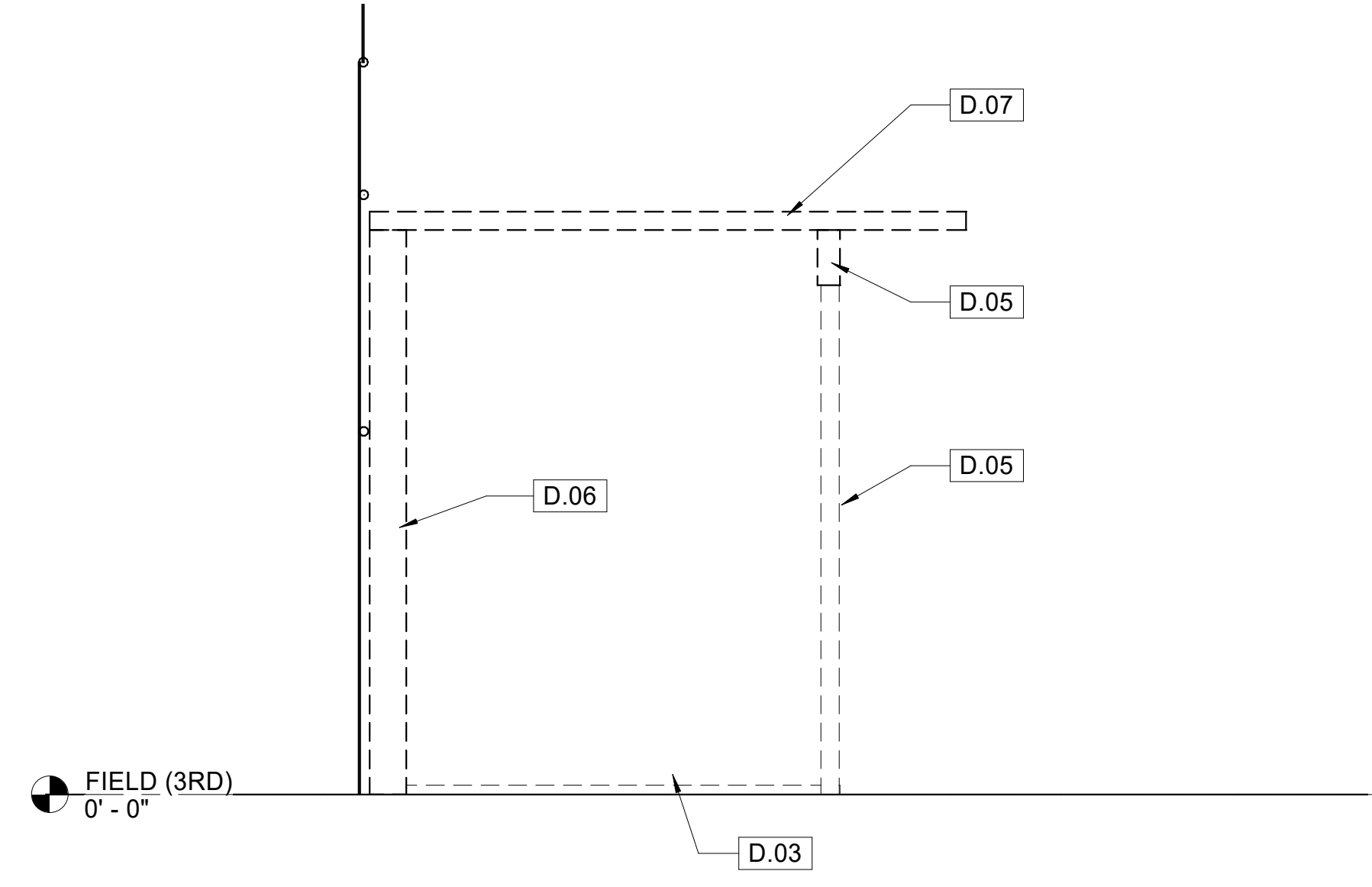
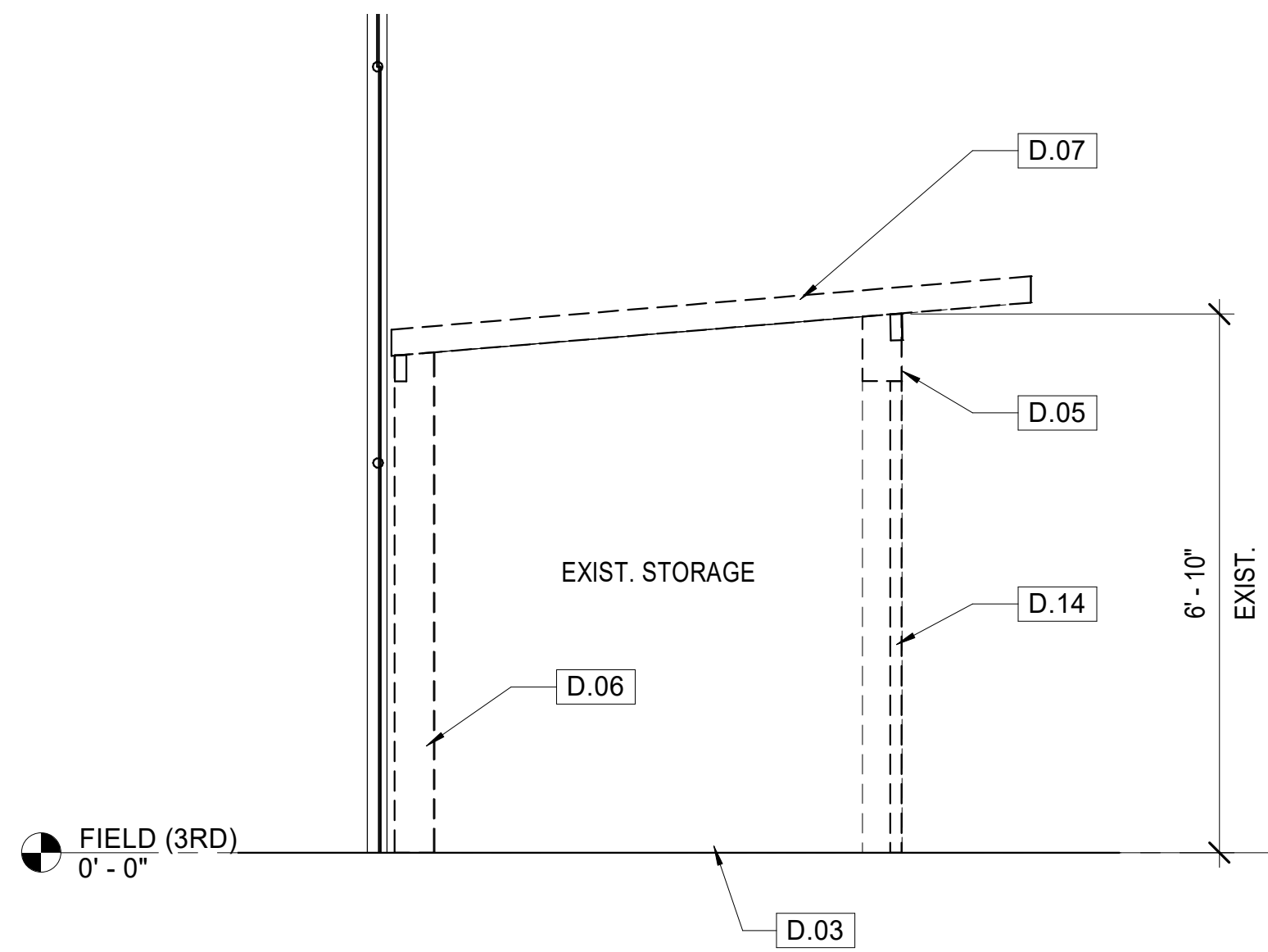
- D.01 CUT (E) BENCH & FLOOR LINE, STORE FOR REPAIR AND REUSE
- D.03 DEMOLISH (E) 4" CONCRETE SLAB ON GRADE WITH #4 REBAR @ 16" O.C. EA. WAY
- D.05 DEMOLISH (E) STEEL POSTS & (E) 4X12 WOOD BEAMS
- D.06 DEMOLISH (E) PLYWOOD & 2X4 STUD WALL
- D.07 DEMOLISH (E) BUILT UP ROOFING OVER PLYWOOD ON 2X4 ROOF JOIST @ 2'-0" O.C.
- D.08 REMOVE AND PROTECT TOP RAIL GUARD
- D.13 DEMOLISH (E) WOOD BASE
- D.14 DEMOLISH (E) WOOD DOOR

EXISTING KEYNOTES

- E.02 (E) A/C PAVING
- E.07 (E) FENCE POST

KEYNOTE LEGEND

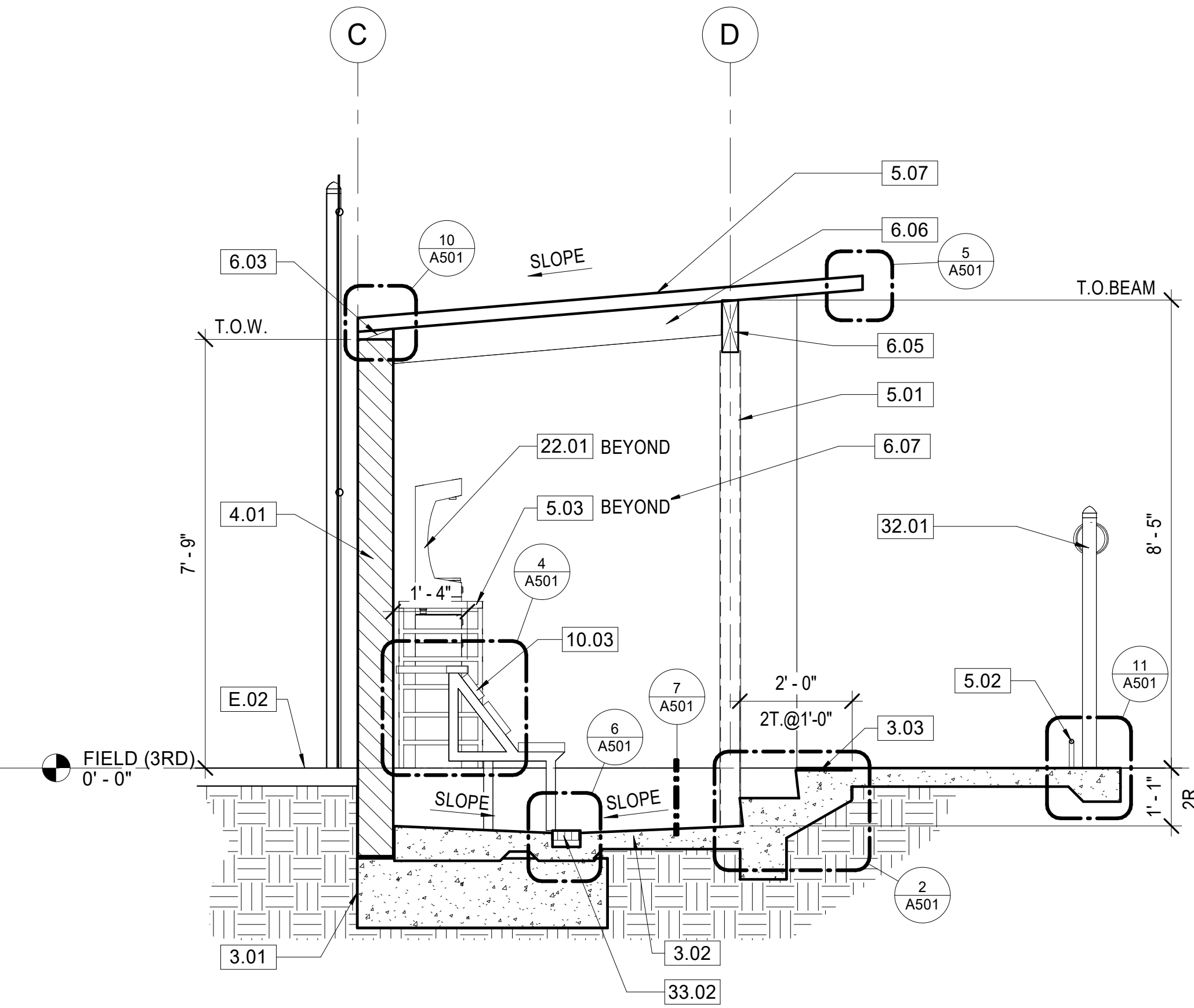
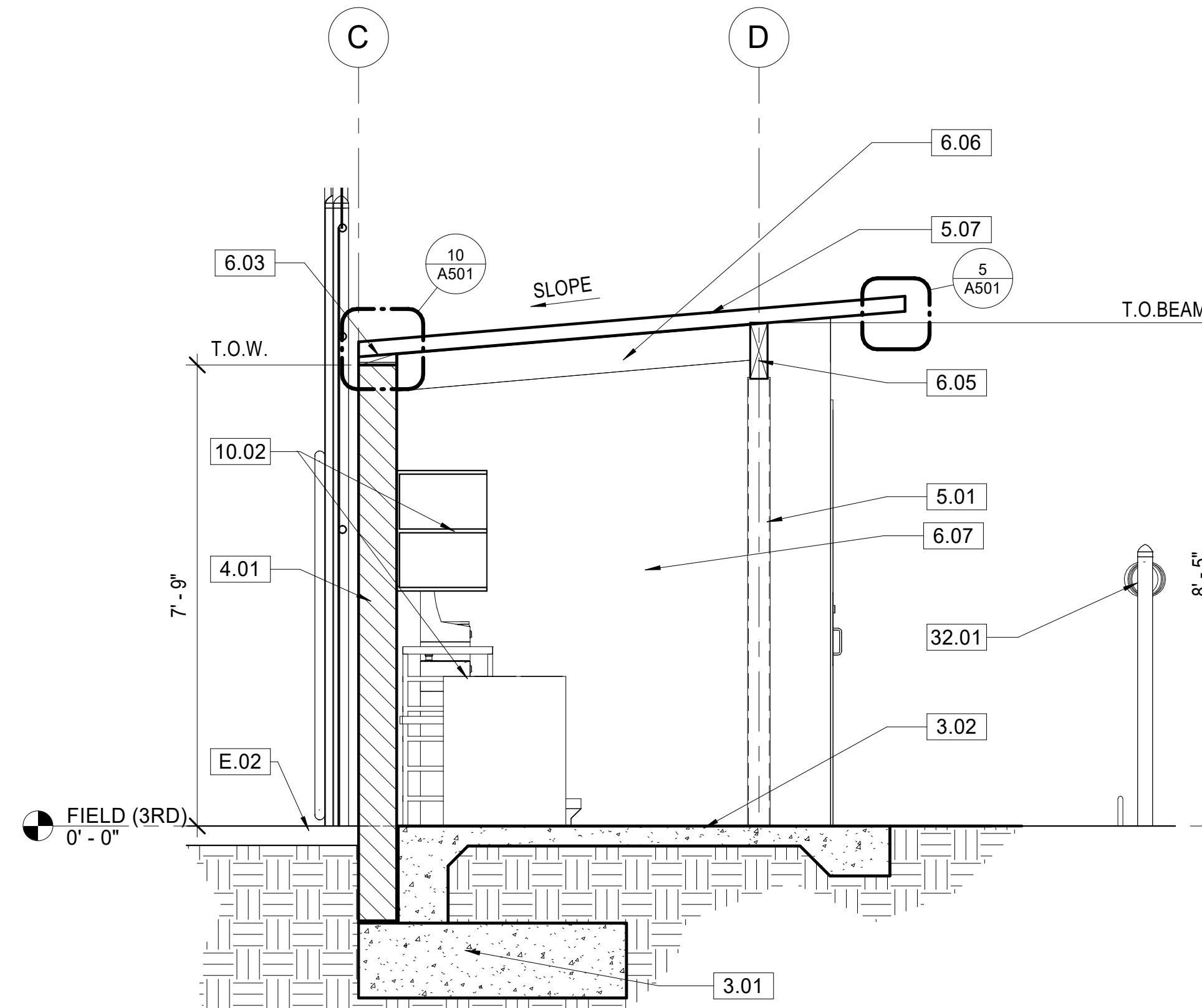
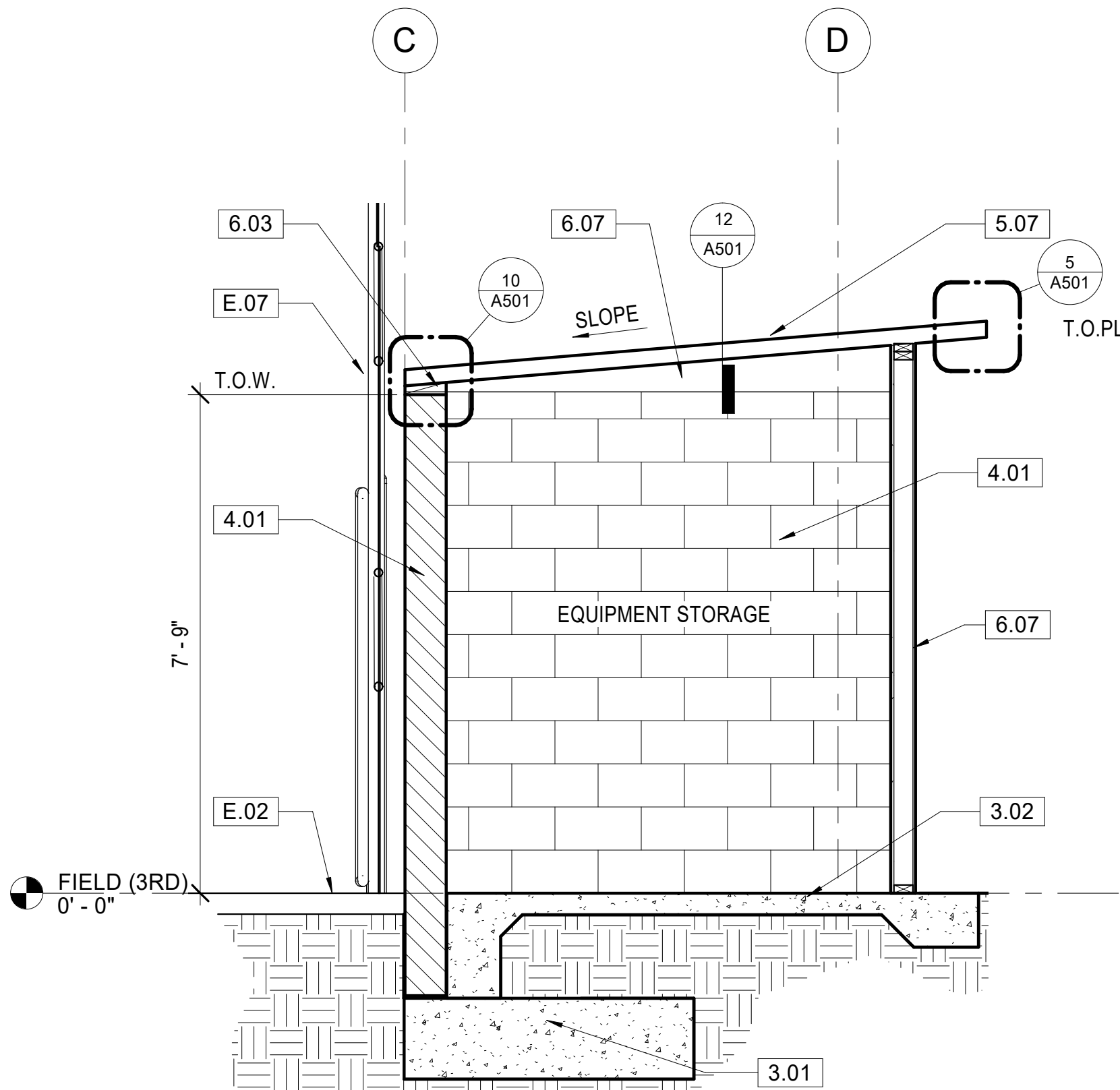
- 3.01 CONCRETE FOOTING, SEE STRUCT. DWGS.
- 3.02 CONCRETE SLAB W/ #4 REBAR @ 16" O.C. EA. WAY, SEE STRUCT. DWGS.
- 3.03 CONCRETE STEPS, SEE STRUCT. DWGS. & 2/A501
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- 5.03 METAL WING RAIL, SEE 4/A502
- 5.07 PAINTED BONDERIZED METAL DECK, SEE STRUCT. DWGS.
- 6.03 3x P.T. WOOD NAILER, SEE STRUCT. DWGS.
- 6.05 PAINTED 4x10 WOOD BEAM, SEE STRUCT. DWGS.
- 6.06 PAINTED 4x8 WOOD BEAM, SEE STRUCT. DWGS.
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- 10.02 REMOVE AND REINSTALL (E) HELMET & BAT RACK
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- 22.01 FREESTANDING HI-LO DRINKING FOUNTAIN WITH HYDRATION STATION AND FILTER. HAWS MODEL 3612, FILTER 6425
- 32.01 REINSTALL (E) TOP RAIL GUARD AND FENCE
- 33.02 TRENCH DRAIN, SEE 6/A501. OWNER FURNISHED, CONTRACTOR INSTALLED. CONTRACTOR TO PROVIDE TRANSITIONS TO STORM DRAIN LINE WITH STRAINERS



5 3RD BASE - DEMO. SECTION @ (E) STOR. RM.
1/2" = 1'-0"

3 3RD BASE - DEMO. SECTION @ (E) EQUIP. AREA
1/2" = 1'-0"

1 3RD BASE - DEMO. SECTION @ (E) DUGOUT
1/2" = 1'-0"



6 3RD BASE - PROPOSED SECTION STORAGE
1/2" = 1'-0"

4 3RD BASE - PROPOSED SECTION @ EQUIPMENT
1/2" = 1'-0"

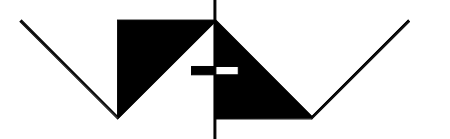
2 3RD BASE - PROPOSED SECTION @ DUGOUT
1/2" = 1'-0"

PROJECT TITLE
20-MPC-037

REPAIR BASEBALL DUGOUTS

7075 CAMPUS ROAD
MOORPARK, CALIFORNIA 93021

COMMISSIONED ARCHITECT



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28328 AGOURA ROAD, SUITE 203
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CONSULTANT

STAMPS/SEALS



Project Status

SHEET TITLE:

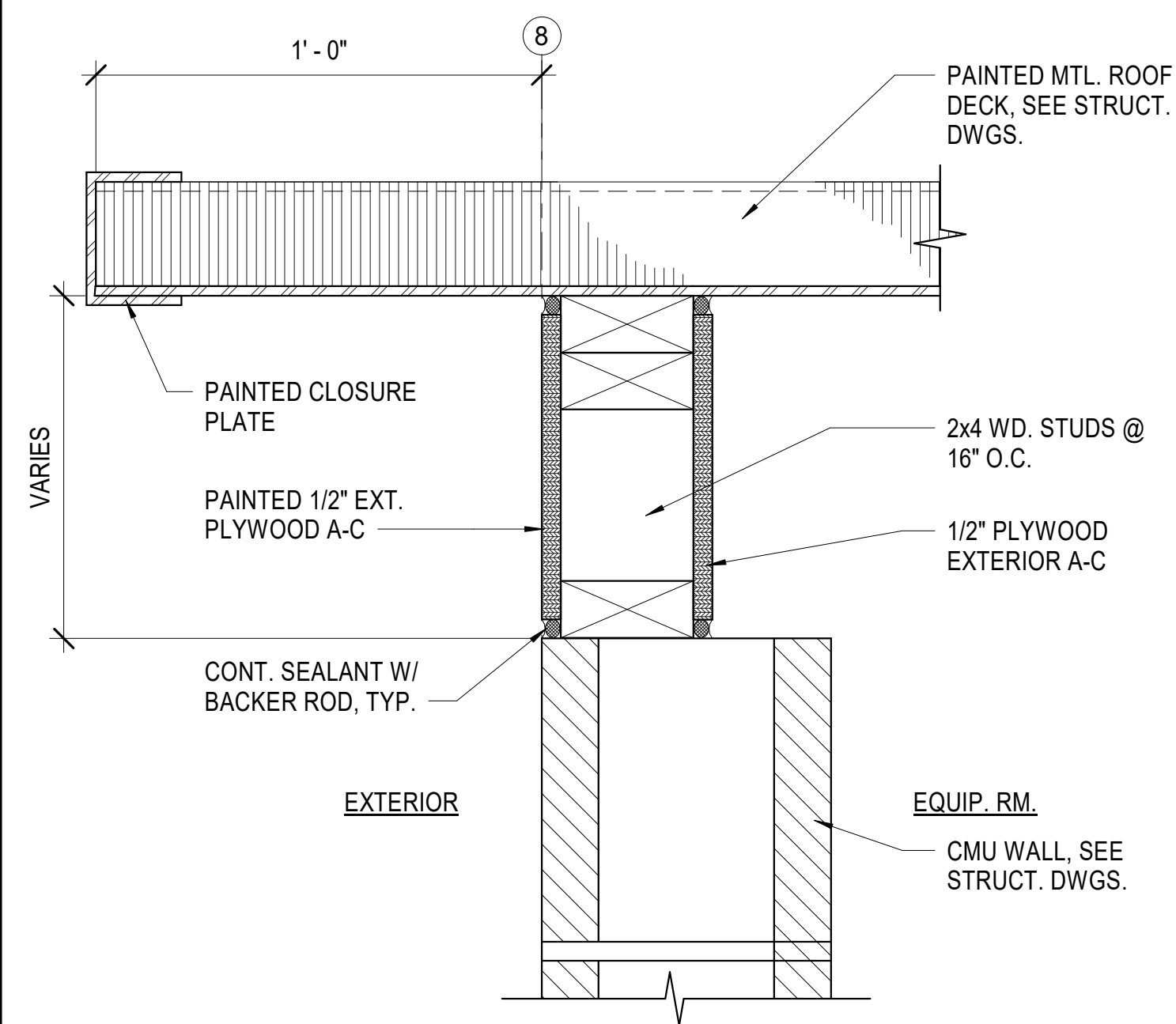
3RD BASE DUGOUT SECTIONS

PROJECT NO. 020-MPC-037 PROJECT ARCH. Designer
DRAWN Author CHECKED Checker

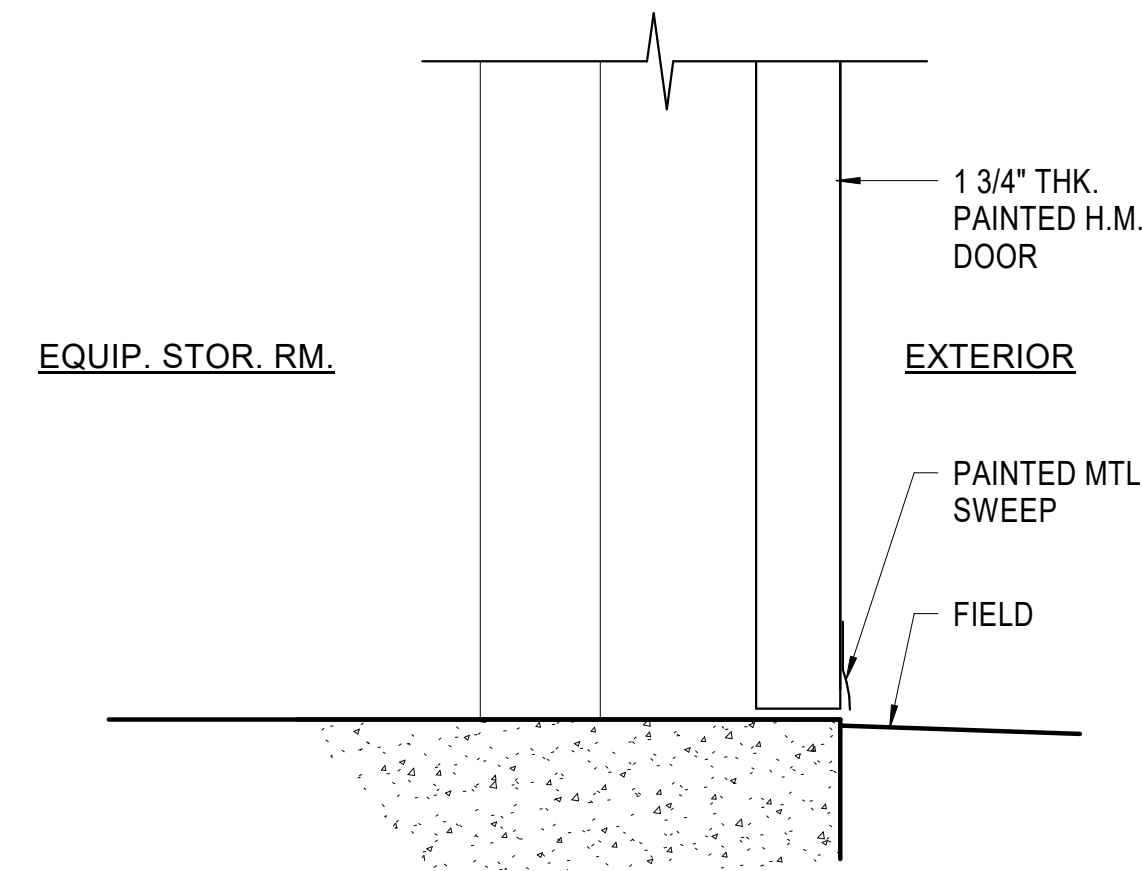
SHEET NUMBER:

A402

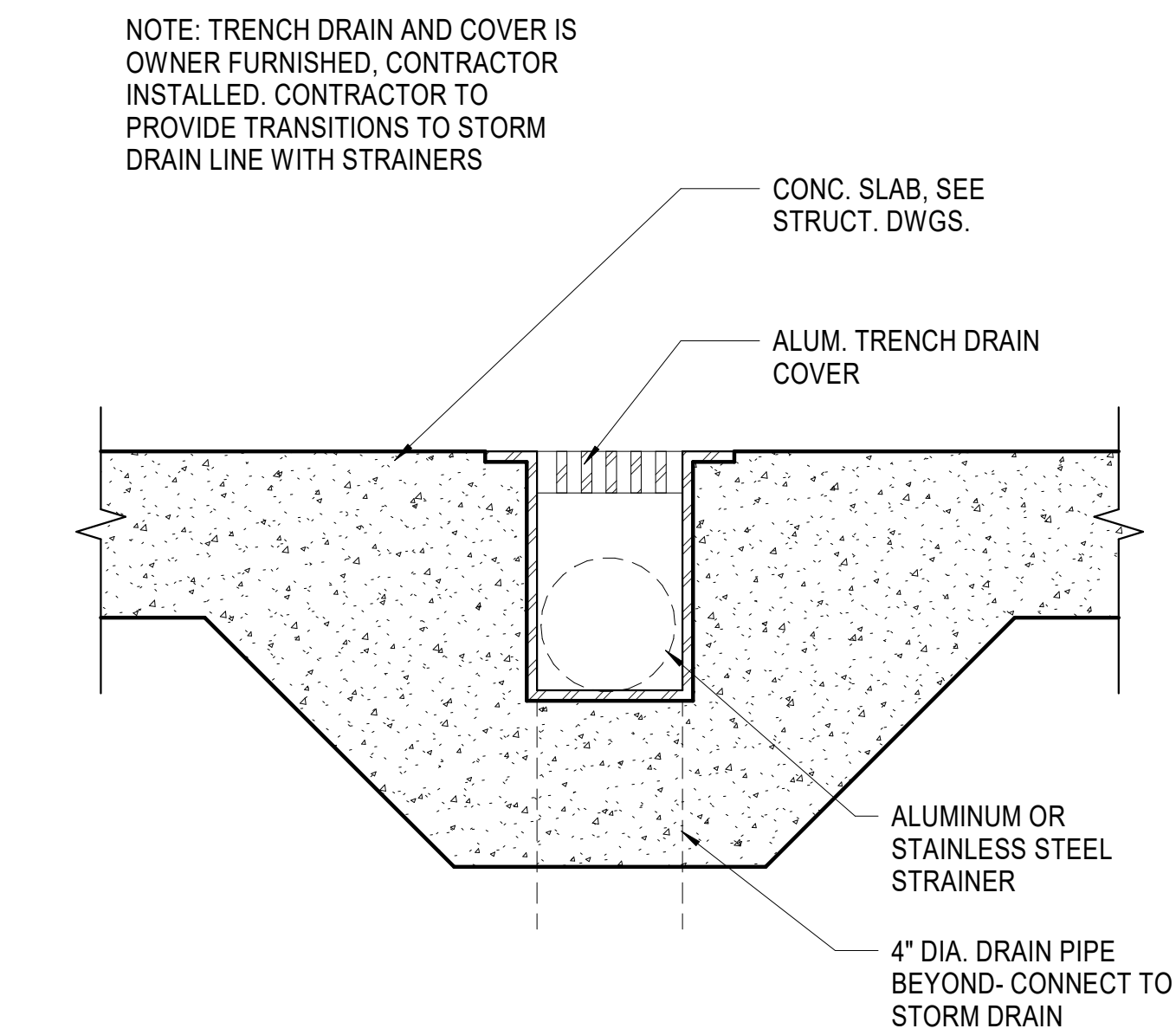
DATE: 6/28/21 SHEET: OF



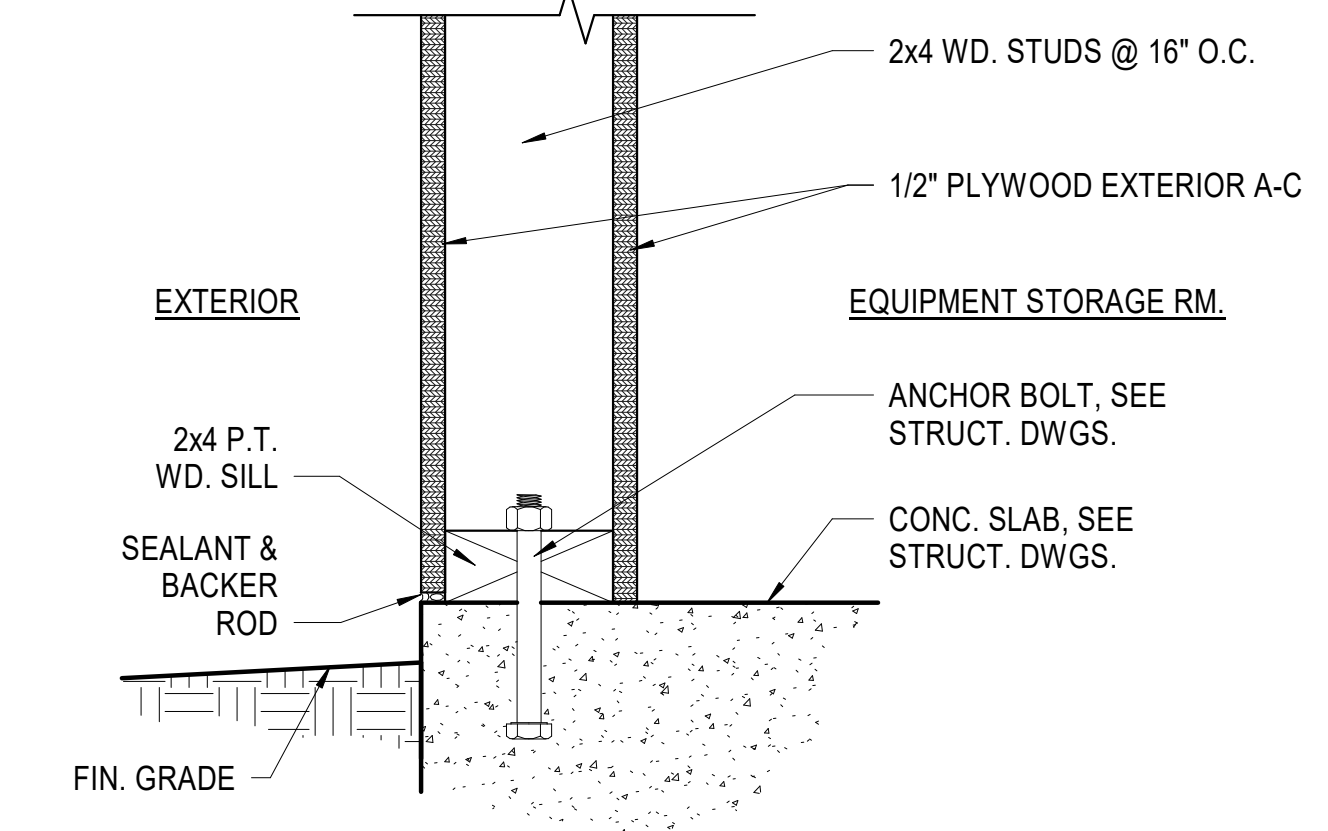
12 ROOF EDGE @ EQUIPMENT ROOM
3" = 1'-0"



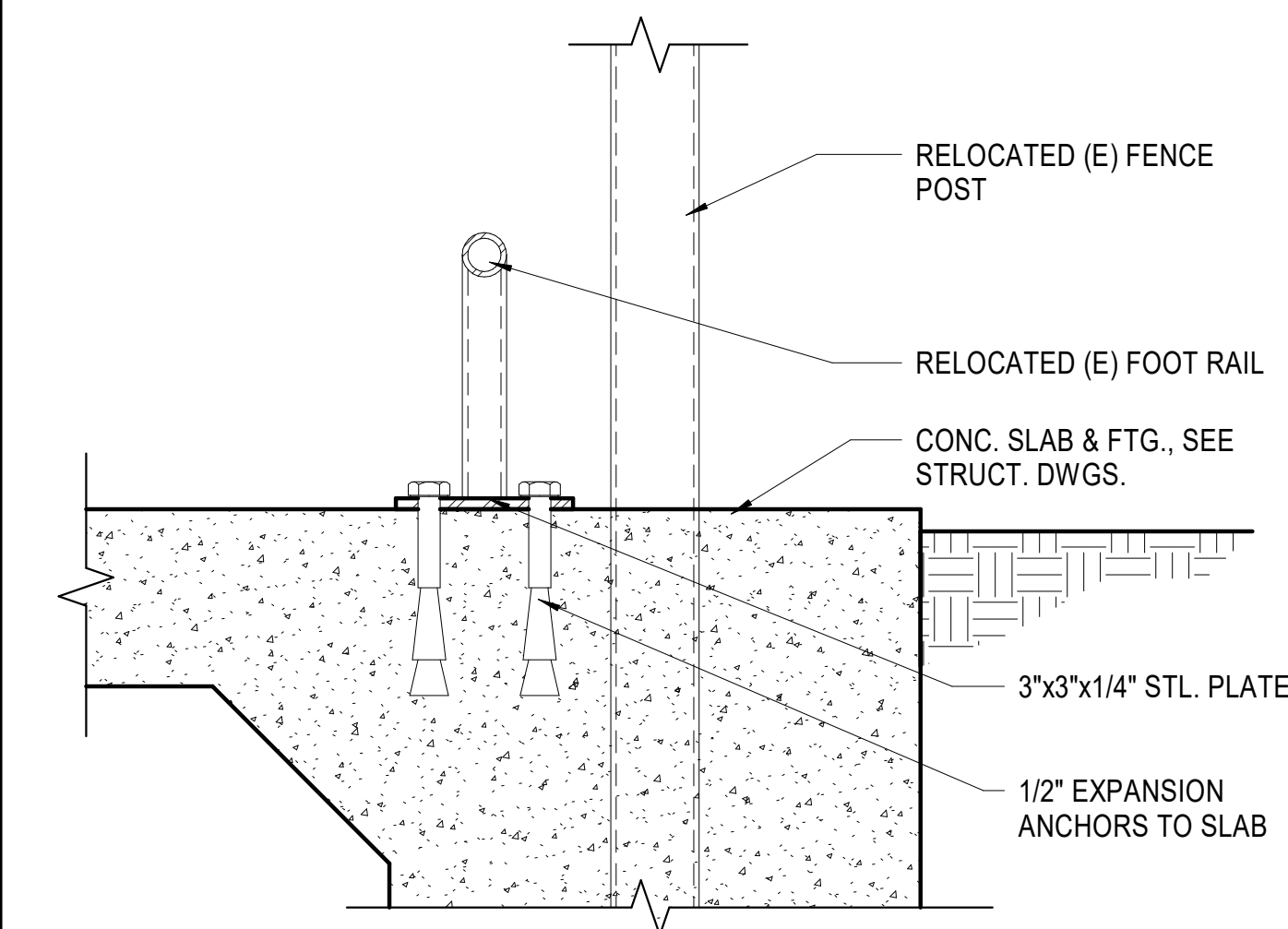
9 DOOR THRESHOLD
3" = 1'-0"



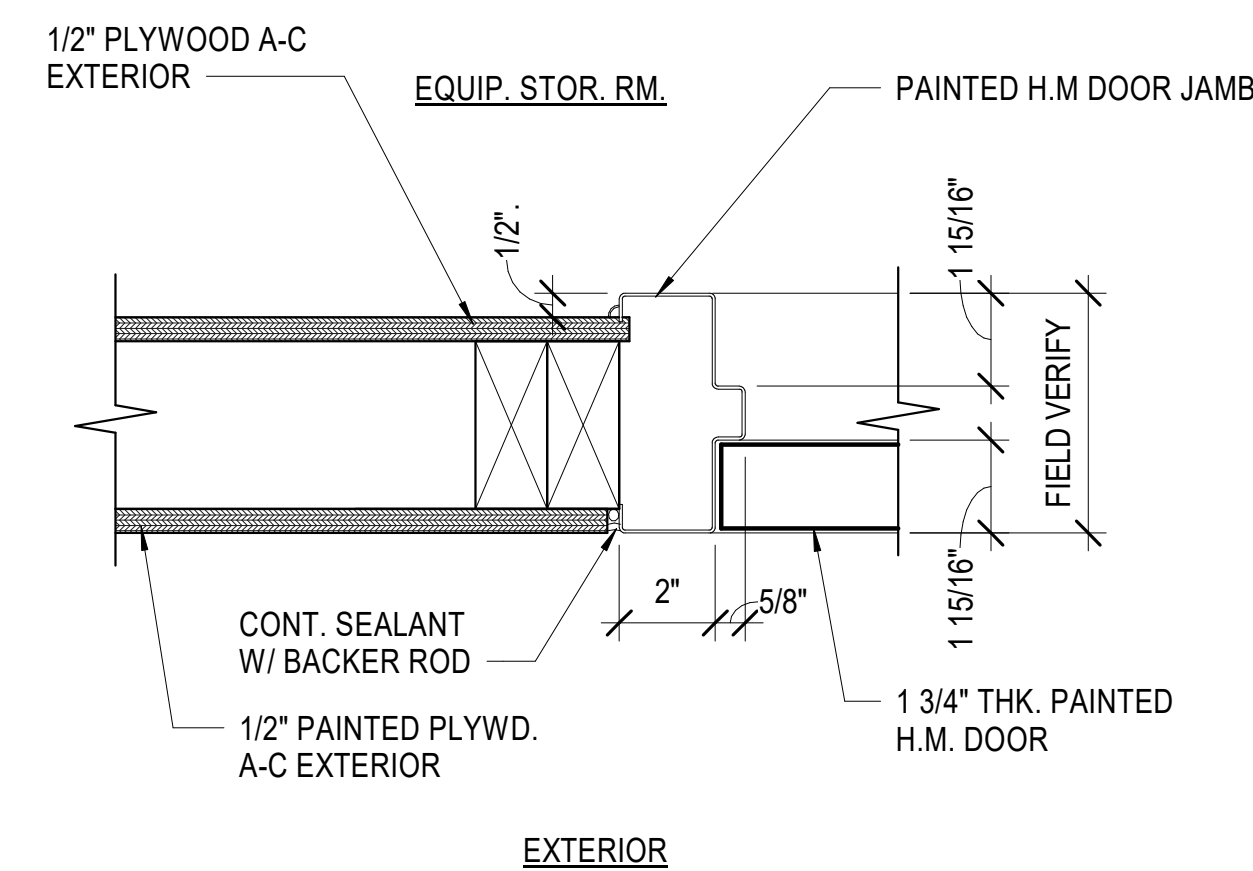
6 TRENCH DRAIN
3" = 1'-0"



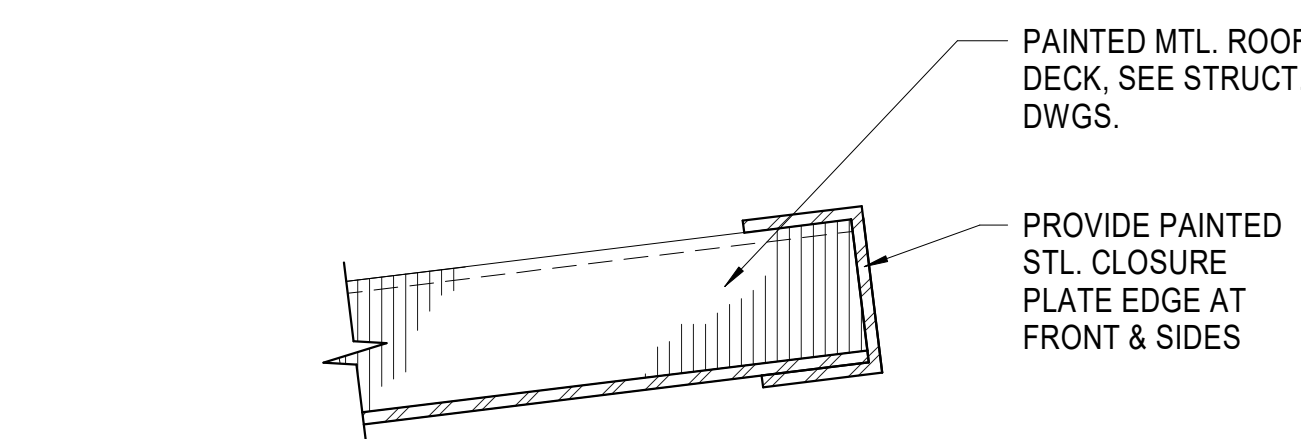
3 STORAGE EQUIPMENT ROOM WALL
3" = 1'-0"



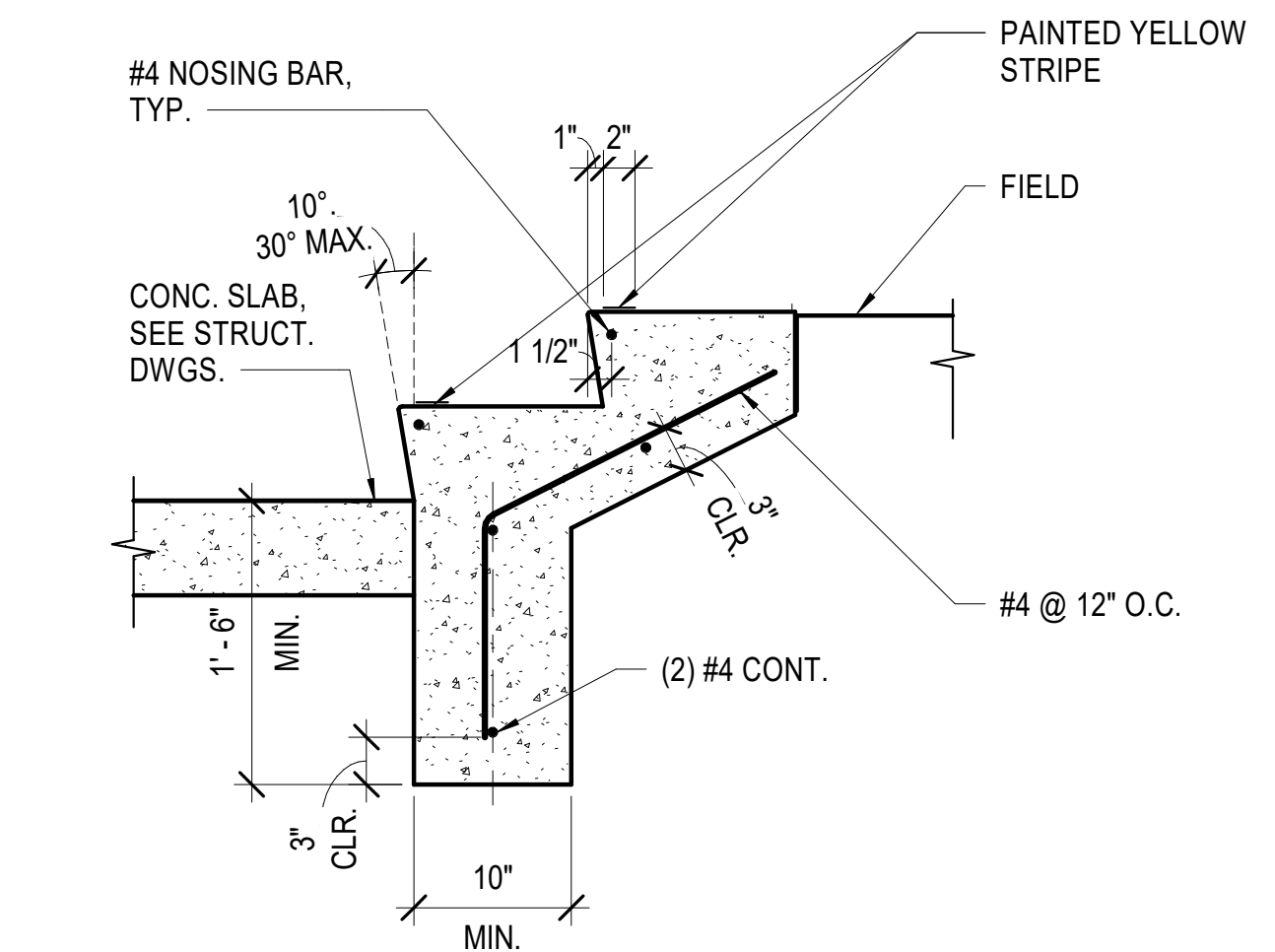
11 FENCE POST & FOOT RAIL
3" = 1'-0"



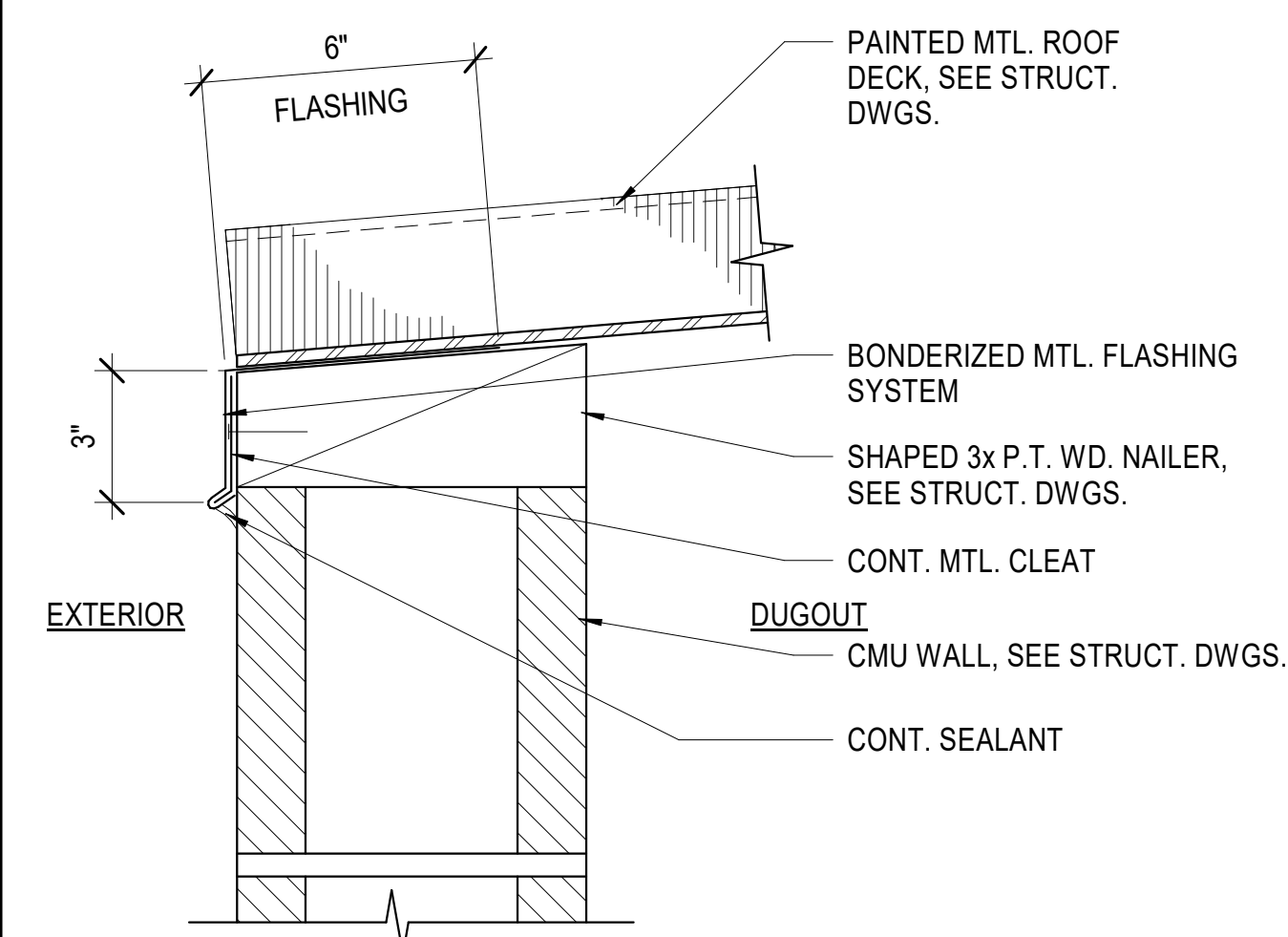
8 DOOR JAMB
3" = 1'-0"



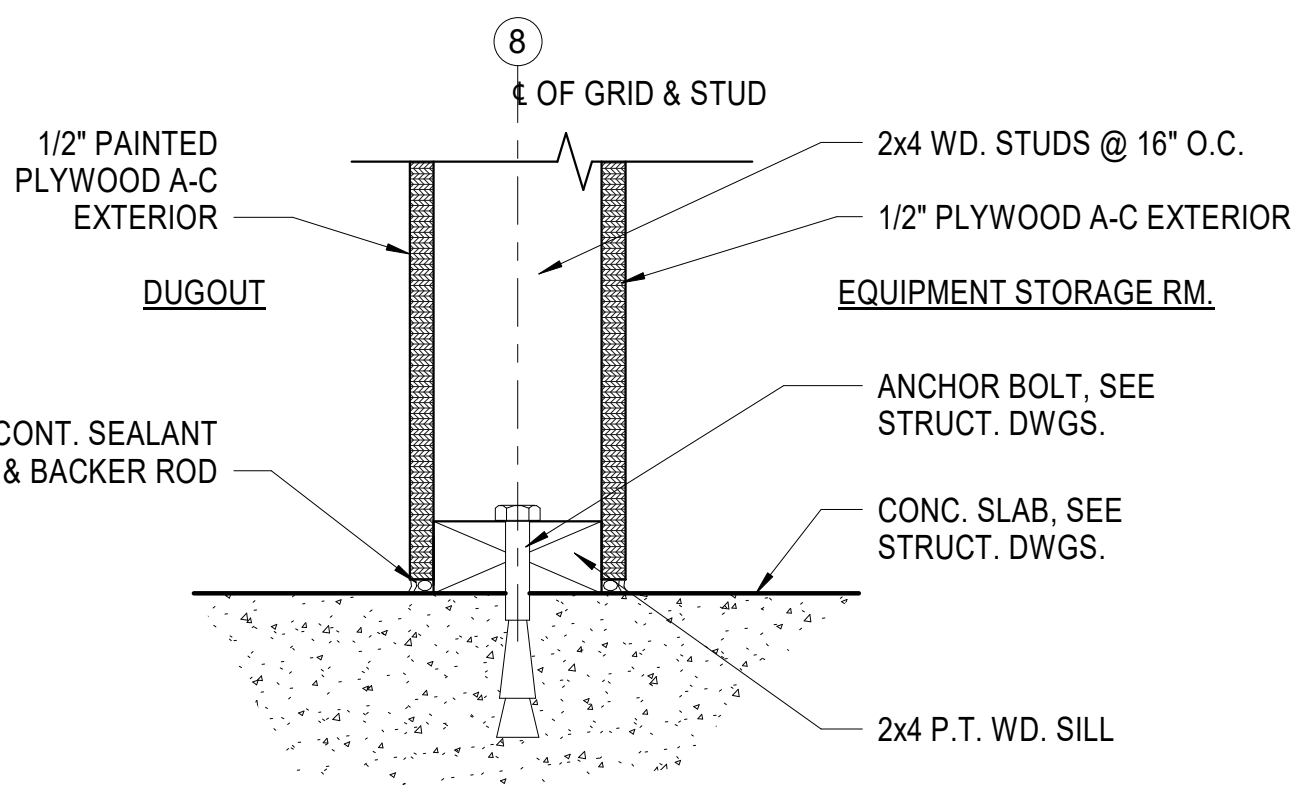
5 ROOF EDGE AT FRONT
3" = 1'-0"



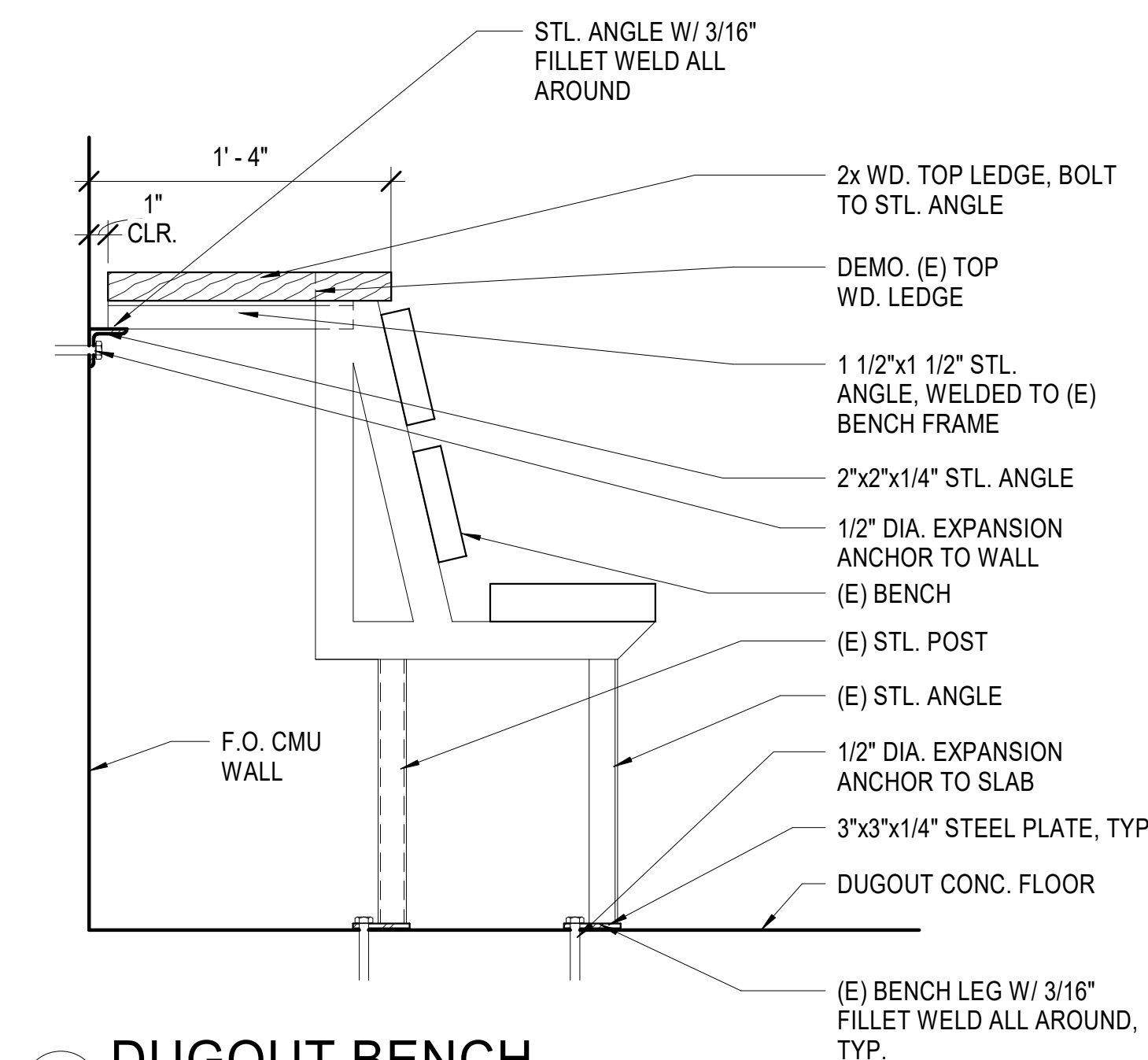
2 DUGOUT STEPS
1" = 1'-0"



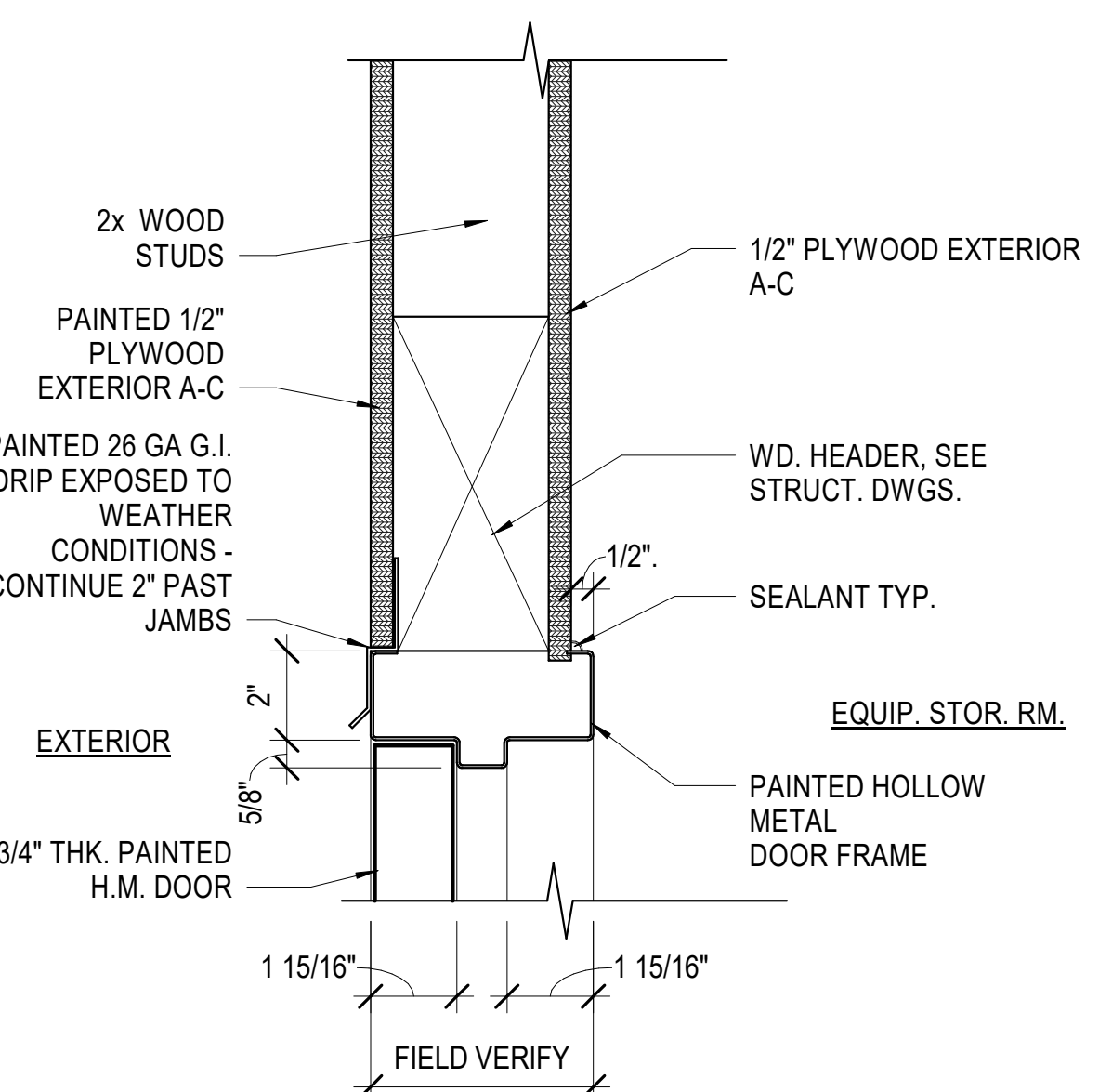
10 ROOF EDGE AT BACK
3" = 1'-0"



7 WALL BASE
3" = 1'-0"



4 DUGOUT BENCH
1 1/2" = 1'-0"

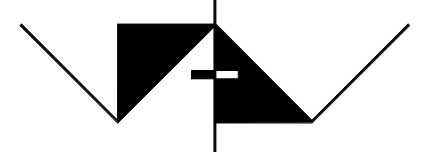


1 DOOR HEAD
3" = 1'-0"

PROJECT TITLE
20-MPC-037
**REPAIR BASEBALL
DUGOUTS**

7075 CAMPUS ROAD
MOORPARK, CALIFORNIA 93021

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CONSULTANT

STAMPS/SEALS



Project Status

SHEET TITLE:

DETAILS

PROJECT NO. 020-MPC-037 PROJECT ARCH. Designer
DRAWN Author CHECKED: Checker

SHEET NUMBER:

A501

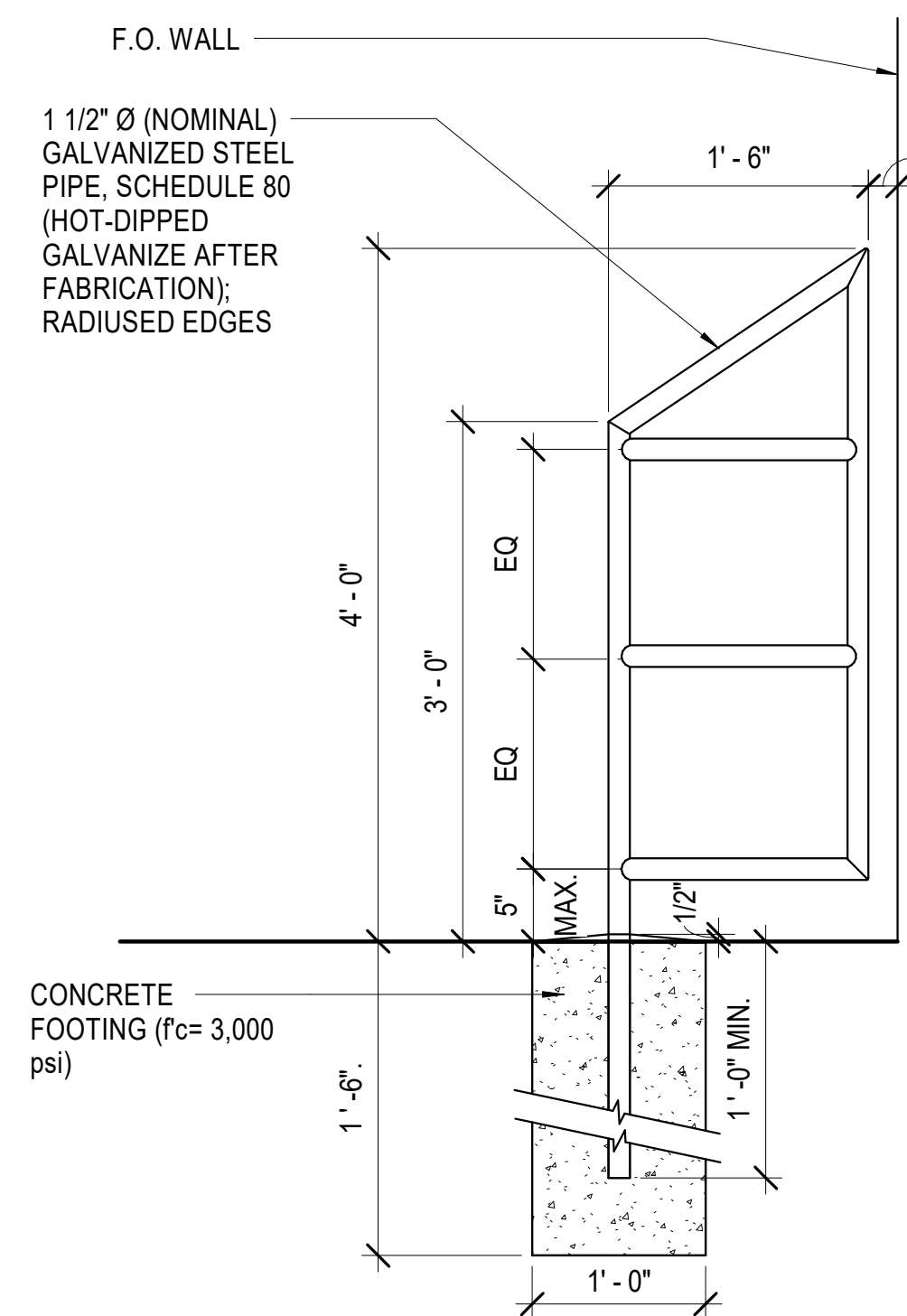
DATE: 6/28/21 SHEET: OF

EXISTING KEYNOTES

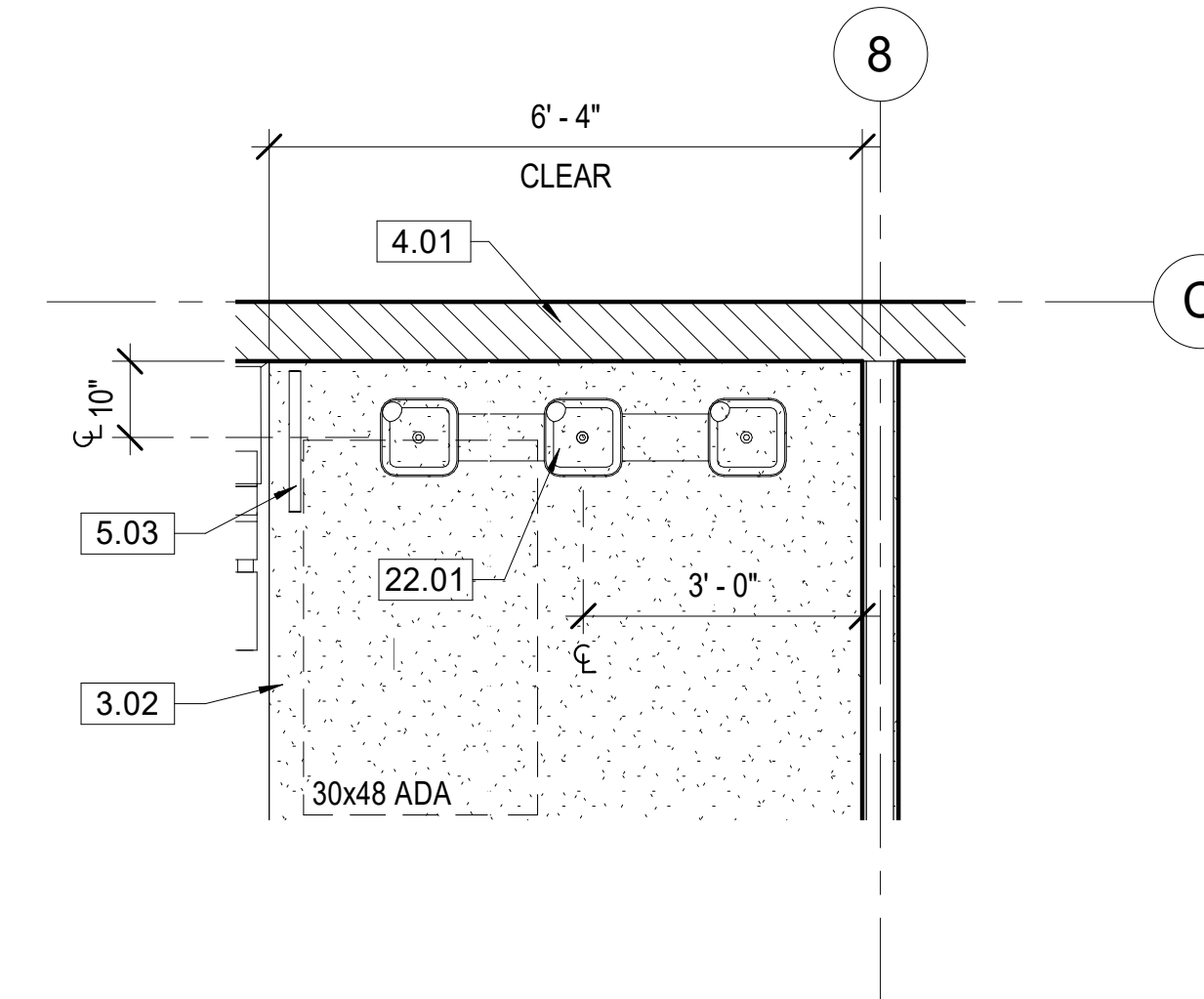
E.04 (E) CHAINLINK FENCING

KEYNOTE LEGEND

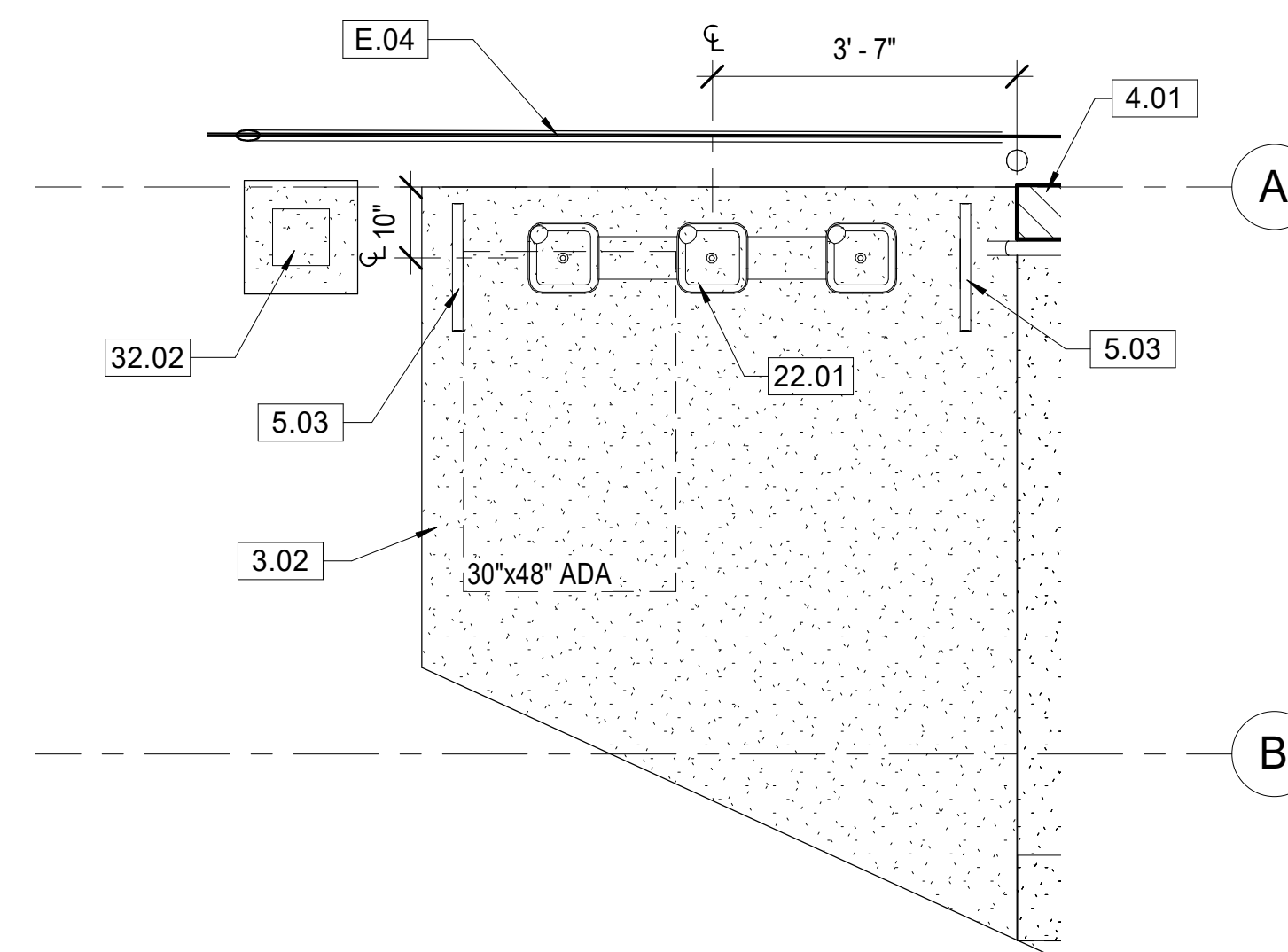
- 3.02 CONCRETE SLAB W/ #4 REBAR @ 16" O.C. EA. WAY, SEE STRUCT. DWGS.
- 4.01 8" X 8" X 16" PRECISION CMU W/ REBAR, SEE STRUCT. DWGS.
- 5.03 METAL WING RAIL, SEE 4/A502
- 22.01 FREESTANDING HI-LO DRINKING FOUNTAIN WITH HYDRATION STATION AND FILTER. HAWS MODEL 3612, FILTER 6425
- 32.02 DRYWELL FOR DRINKING FOUNTAIN DRAINAGE, SEE 1/A502



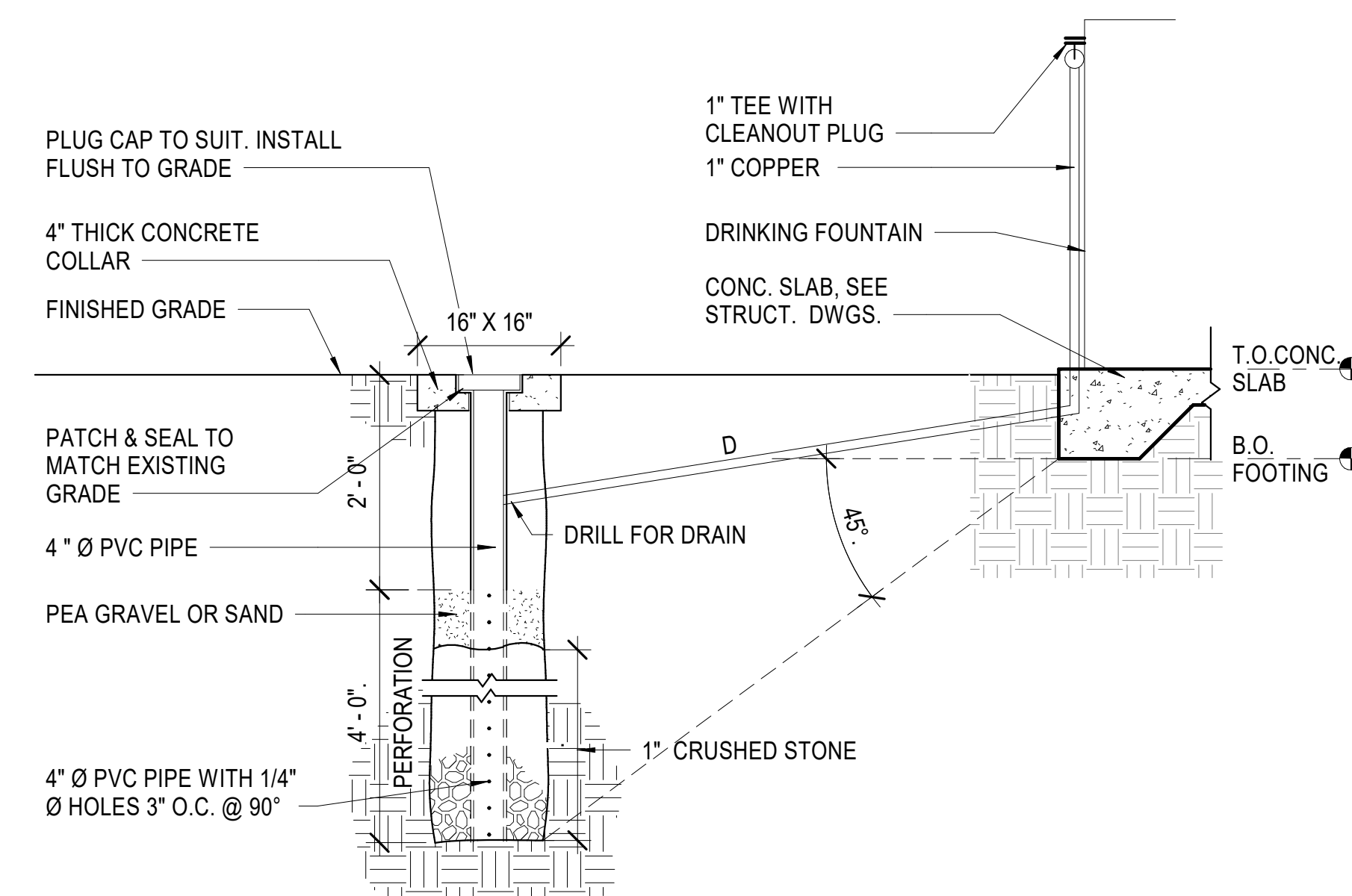
4 WING GUARD DETAIL
1" = 1'-0"



2 DRINKING FOUNTAIN PLAN AT THIRD BASE
1/2" = 1'-0"



3 DRINKING FOUNTAIN PLAN AT FIRST BASE
1/2" = 1'-0"



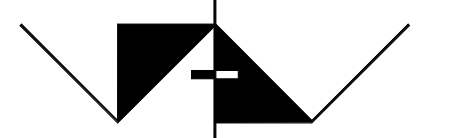
1 DRYWELL
3/4" = 1'-0"

PROJECT TITLE
20-MPC-037

**REPAIR BASEBALL
DUGOUTS**

7075 CAMPUS ROAD
MOORPARK, CALIFORNIA 93021

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CONSULTANT

STAMPS/SEALS



Project Status

SHEET TITLE:

DETAILS

PROJECT NO. 020-MPC-037 PROJECT ARCH. Designer
DRAWN Author CHECKED Checker

SHEET NUMBER:

A502

DATE: 06/29/21 SHEET: ___ OF ___

REINFORCING STEEL NOTES

- ALL TYPICAL REINFORCING BARS SHALL CONFORM TO ASTM A-615, GRADE 60, UNLESS NOTED OTHERWISE ON THE DRAWINGS (#3 BARS MAY BE GRADE 40 FOR AVAILABILITY)
- SPIRALS SHALL BE COLD DRAWN BARS CONFORMING TO ASTM A-82. REINFORCING FOR DIAPHRAGMS AND FOUNDATIONS MAY BE GRADE 75 IN LIEU OF GRADE 60, AT THE CONTRACTOR'S OPTION. MAINTAIN OVERALL CAPACITY OF ELEMENTS WHERE GRADE 75 REINFORCING IS PROPOSED FOR USE. IN GENERAL, REDUCE REQUIRED STEEL AREA IN PROPORTION TO RATIO OF YIELD STRENGTH. MAINTAIN BAR SPACING SHOWN ON PLANS, DETAILS, AND SCHEDULES.
 - MOMENT FRAME LONGITUDINAL REBAR, SHEAR WALL VERTICAL REBAR, AND COUPLING BEAM LONGITUDINAL REBAR SHALL BE ASTM A-706 (Fy=60 KSI). SMOOTH DOWELS IN SLAB ON GRADE: ASTM A36, 36 KSI
- WELDING OF REINFORCEMENT (INCLUDING TACK WELDING) SHALL BE NOT BE DONE UNLESS SPECIFICALLY SHOWN ON THE DRAWINGS. WHERE SHOWN ON THE DRAWINGS, THE FOLLOWING SHALL APPLY:
 - WELDED REBAR SHALL COMPLY WITH ASTM A-706 (Fy=60 KSI)
 - WELDING SHALL CONFORM TO AWS D1.4
 - WELDING OF REINFORCING STEEL SHALL BE PERFORMED BY WELDERS CERTIFIED BY THE CITY OF LA
 - USE E90XX ELECTRODES
- WELDED WIRE FABRIC SHALL BE MADE OF COLD DRAWN WIRE AND SHALL CONFORM TO ASTM A-185 (Fy=65 KSI). MINIMUM LAP AT SPLICES OF 12 INCHES. PROVIDE MESH IN FLAT SHEETS ONLY. ROLLED MESH IS NOT ACCEPTABLE. OFFSET END-LAPS IN ADJACENT SHEETS TO PREVENT CONTINUOUS LAPS.
- REINFORCING STEEL SHALL HAVE THE FOLLOWING CONCRETE COVER. SEE ACI FOR TOLERANCES:

A. CONCRETE POURED AGAINST EARTH:	3"
B. FORMED CONCRETE IN CONTACT WITH EARTH:	2"
C. CONCRETE EXPOSED TO WEATHER (#6 AND LARGER):	2"
D. CONCRETE EXPOSED TO WEATHER (#5 AND SMALLER):	1½"
E. SLABS (INCLUDING SLAB SUPPORTING EARTH), WALLS, AND JOISTS NOT EXPOSED TO WEATHER (#11 AND SMALLER):	1"
F. OTHER CONCRETE NOT EXPOSED TO WEATHER:	1½"
G. COLLECTORS	2"
- #5 AND LARGER REINFORCING BARS SHALL NOT BE SPICED EXCEPT AS LOCATED AND DETAILED ON THE DRAWINGS. #4 AND SMALLER BARS WITH LENGTHS NOT SHOWN SHALL BE CONTINUOUS. PROVIDE CLASS 'B' SPLICE UNLESS NOTED OTHERWISE. ALL BARS IN MASONRY SHALL BE CONTINUOUS, LAPPING 48 BAR DIAMETERS, 2'-0" MINIMUM. HORIZONTAL WALL SPLICES SHALL BE STAGGERED. VERTICAL BARS SHALL NOT BE SPICED EXCEPT AT HORIZONTAL SUPPORTS, SUCH AS FLOOR OR ROOF, UNLESS DETAILED OTHERWISE. ALL BARS ENDING AT THE FACE OF A WALL, COLUMN, OR BEAM SHALL EXTEND TO WITHIN 2" OF THE FAR FACE AND HAVE A 90 DEGREE HOOK, UNLESS OTHERWISE SHOWN.
- BARS SHALL BE FIRMLY SUPPORTED AND ACCURATELY PLACED AS REQUIRED BY THE ACI STANDARDS. USING TIE AND SUPPORT BARS IN ADDITION TO REINFORCEMENT SHOWN WHERE NECESSARY FOR FIRM AND ACCURATE PLACING. PROVIDE DOWELS TO MATCH ALL REINFORCEMENT AT POUR JOINTS, UNLESS SHOWN OR NOTED OTHERWISE. ALL DOWELS AND BOLTS SHALL BE ACCURATELY SET IN PLACE BEFORE PLACING CONCRETE. NO WELDING OF REINFORCEMENT (INCLUDING TACK WELDING) SHALL BE DONE UNLESS SHOWN ON THE DRAWINGS OR APPROVED BY THE ENGINEER. ALL SLAB AND BEAM REINFORCEMENT SHALL BE CHAIRED UP.
- IN WALL REINFORCING, CURTAINS CONTAINING VERTICAL AND HORIZONTAL BARS OF THE SAME SIZE, VERTICAL BARS SHALL BE PLACED CLOSEST TO THE WALL SURFACE. IN CURTAINS WHICH VERTICAL AND HORIZONTAL BARS ARE OF DIFFERENT SIZES OR SPACING, THE LAYER WITH THE MOST STEEL SHALL BE PLACED CLOSEST TO THE NEAR SURFACE.
- DRAWINGS SHOW TYPICAL REINFORCING CONDITIONS. CONTRACTOR SHALL PREPARE DETAILED PLACEMENT DRAWINGS OF ALL CONDITIONS SHOWING QUANTITY, SPACING, SIZES, CLEARANCES, LAPS, INTERSECTIONS, AND COVERAGE REQUIRED BY THE STRUCTURAL DETAILS, APPLICABLE CODE, AND TRADE STANDARDS. CONTRACTOR SHALL NOTIFY REINFORCING INSPECTOR OF ANY ADJUSTMENTS FROM TYPICAL CONDITIONS WHICH ARE PROPOSED IN PLACEMENT DRAWINGS TO FACILITATE FIELD PLACEMENT OF REINFORCING STEEL AND CONCRETE.
- ALL PRINCIPAL REBAR SHALL TERMINATE WITH A STANDARD HOOK MINIMUM UNLESS SPECIFICALLY DETAILED OTHERWISE. REBAR BENDS SHALL BE MADE COLD. REBAR SHALL NOT BE BENT AFTER ANY PORTION OF THE BAR IS ENCASED IN CONCRETE.
- ALL LAP SPLICES ARE CLASS 'B' LAP SPLICES UNLESS NOTED OTHERWISE.
- ALL WALL FOOTING REINFORCEMENT SHALL BEND AROUND ALL CORNERS AND EXTEND 36 BAR DIAMETERS OR 18 INCHES WHICHEVER IS LARGER. UNLESS NOTED OTHERWISE.
- ALL SLABS ON GRADE LESS THAN 6" IN THICKNESS SHALL BE REINFORCED WITH #4 REBARS AT 16 INCHES ON CENTERS EACH WAY, UNLESS NOTED OTHERWISE. PROVIDE ONE (1) LAYER OF 6X6/W2.9XW2.9 WELDED WIRE FABRIC CONTINUOUS FOR EVERY 3" ARCHITECTURAL CONCRETE FILLS ABOVE THE STRUCTURAL SLAB.
- ALL MECHANICAL, PLUMBING AND ELECTRICAL EQUIPMENT PADS LESS THAN 4" THICK SHALL BE REINFORCED WITH AT LEAST ONE (1) LAYER OF 6X6/W2.9XW2.9 WELDED WIRE FABRIC AND HAVE HOOKED DOWELS (#3 AT 12" ON CENTERS) INTO THE STRUCTURAL SLAB. UNLESS NOTED OTHERWISE. FOR PADS GREATER THAN 4 INCHES THICK, USE REINFORCING AS SHOWN IN THE TYPICAL DETAILS.
- ADDITIONAL REINFORCEMENT SHALL BE PROVIDED AROUND ALL SLAB AND WALL OPENINGS INCLUDING DIAGONAL BARS WITHOUT EXCEPTION.
- ALL STRUCTURAL CONCRETE ELEMENTS REQUIRE REINFORCEMENT SINCE NO PLAIN CONCRETE ELEMENTS ARE USED. ALL CONCRETE SLABS SHALL HAVE A MINIMUM REINFORCEMENT PERCENTAGE OF 0.0018 EACH WAY CONTINUOUS.

STRUCTURAL STEEL

- ALL STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING STANDARDS:
W-WIDE FLANGE SHAPES ASTM A992, Fy = 50 KSI
PLATES, ANGLES, CHANNELS: ASTM A36, Fy = 36 KSI
HOLLOW TUBE SHAPES: ASTM A500, GRADE B, Fy = 46 KSI
ROUND PIPE SHAPES: ASTM A53, GRADE B, Fy = 35 KSI
- ALL NEW STRUCTURAL STEEL SHALL BE IDENTIFIED PER 2019 CBC SECTION 2203A. DESIGN OF STEEL MEMBERS SHALL BE PER ASD PROVISIONS IN THE 2019 CBC CHAPTER 22A SECTION 2205A.1 & 2205A.2, AISC 341, AND AISC 360 U.N.O.
- EXPOSED STRUCTURAL STEEL SHALL HAVE A SHOP COAT OF METAL PRIMER PER GROWTHPOINT MANUFACTURING FOR THE COMPLETED MODULE.
- AFTER ERECTION, ALL FIELD CONNECTIONS, BOLTS, WELDS, AND ALL ABRADED PLACES ON THE SHOP PAINT SHALL BE TOUCHED UP WITH THE SAME TYPE OF PAINT AS THE SHOP COAT.
- FIELD AND SHOP WELDING SHALL BE DONE BY AN AWS CERTIFIED WELDER PER CRATE MANUFACTURING WELDING PROCESSES AND PROCEDURES, UNLESS NOTED OTHERWISE. CONTINUOUS INSPECTION BY A REGISTERED INSPECTOR IS REQUIRED FOR FIELD WELDING. ALL WELDING SHALL BE PER AWS D1.1, LATEST EDITION, AND AISC SPECIFICATIONS.
- BOLTS SHALL BE OF A307 QUALITY WITH WASHERS, UNLESS OTHERWISE SPECIFIED ON PLANS.

MASONRY

- BLOCK SHALL BE MEDIUM WEIGHT (115 PCF) CONFORMING TO ASTM- C-90 GRADE N-1. USE UNITS OPEN ONE END, AND BOND BEAM UNITS AT HORIZONTAL REINFORCING. WHEN BLOCKS ARE EXPOSED OBTAIN APPROVAL OF SUBMITTAL FROM ARCHITECT. UNITS SHALL ATTAIN A MINIMUM COMPRESSIVE STRENGTH AS REQUIRED TO MEET THE MASONRY COMPRESSIVE STRENGTH OF MASONRY fm SPECIFIED ON THE PLANS AS FOLLOWS:

A. 1,900 PSI FOR SPECIFIED fm UP TO 1,500 PSI
B. 2,800 PSI FOR SPECIFIED fm UP TO 2,000 PSI
C. 3,750 PSI FOR SPECIFIED fm UP TO 2,500 PSI
D. 4,800 PSI FOR SPECIFIED fm UP TO 3,000 PSI
- MIN. SPECIFIED COMPRESSIVE STRENGTH SHALL BE fm = 1,500 PSI, UNLESS OTHERWISE SPECIFIED ON THE PLANS.
- CEMENT: ASTM C-150, LOW ALKALI, TYPE 1 OR 11 PORTLAND CEMENT. (MASONRY CEMENT AND PLASTIC CEMENT SHALL NOT BE USED)
- MORTAR:
 - CONFORMING TO ASTM C-270, TYPE [S].
 - MIX PROPORTIONS SHALL CONFORM TO ASTM C-270.
 - AGGREGATED SHALL CONFORM TO ASTM C-144.
- GROUT:
 - CONFORMING TO ASTM C-476.
 - ATTAINS THE MASONRY COMPRESSIVE STRENGTH fm OR 2,000 PSI AT 28 DAYS, WHICHEVER IS GREATER.
 - MIX PROPORTIONS SHALL CONFORM TO ASTM C-476
 - AGGREGATES SHALL CONFORM TO ASTM C-404
 - USE COARSE GROUT IN GROUT SPACES 2 INCHES OR MORE IN WIDTH AND CELLS TO BE GROUTED SOLID.
- ADMIXTURES: DO NOT USE ANY ADMIXTURES IN MORTAR OR GROUT WITHOUT APPROVAL BY THE ARCHITECT.
- MEASURE MATERIALS FOR MORTAR AND GROUT IN CALIBRATED DEVICES. SHOVEL MEASUREMENTS ARE NOT ACCEPTABLE.
- ADJUST THE WATER CONTENT OF THE MORTAR AND GROUT MIXES TO PROVIDE PROPER WORKABILITY UNDER EXISTING FIELD CONDITIONS WITHOUT SEGREGATION.
- REINFORCING STEEL:
 - REBAR: ASTM A-615, GRADE 60 (FY=60KSI).
 - JOINT REINFORCEMENT: ASTM A-951
- LAP REINFORCING STEEL AT SPLICES WITH A MINIMUM 48 BAR DIAMETERS, UNLESS NOTED OTHERWISE. WHERE CLEAR DISTANCE BETWEEN BARS AT ADJACENT SPICES IS 3 INCHES OR LESS, INCREASE LAP LENGTH 30% UNLESS SPLICES ARE STAGGERED AT LEAST 24 BAR DIAMETERS.
- MASONRY WORK SHALL CONFORM TO THE LATEST ADOPTED EDITION OF THE LABC. AND THE 2019 MSJC SPECIFICATIONS.
- CONCRETE BLOCK UNITS ARE TO BE STAGGERED & TO HAVE VERTICAL CONTINUITY OF CELLS UNOBSERVED.
- IF WORK IS STOPPED AN HOUR OR LONGER, PROVIDE HORIZONTAL CONSTRUCTION JOINT BY STOPPING GROUT 1½" BELOW TOP OF MASONRY UNIT.
- SPECIAL INSPECTION IS REQUIRED FOR ALL MASONRY WORK.
- GROUT ALL MASONRY WALLS SOLID. GROUTING LIFTS SHALL NOT EXCEED 5'-0" IN HEIGHT IN ACCORDANCE WITH 2008 MSJC SPECIFICATIONS.
- THE CLEAR DISTANCE BETWEEN THE SURFACE OF A BAR AND ANY SURFACE OF A MASONRY UNIT SHALL BE NOT LESS THAN ¼" FOR FINE GROUT AND NOT LESS THAN ½" FOR COURSE GROUT.
- SECURE REBAR AGAINST DISPLACEMENT PRIOR TO GROUTING AT INTERVALS NOT GREATER THAN 200 BAR DIAMETERS.
- TERMINATE HORIZONTAL BARS WITH A STANDARD HOOK AT THE JAMBS OF WALL OPENINGS.
- VERIFY SPECIFIED COMPRESSIVE STRENGTH OF MASONRY IN ACCORDANCE WITH ONE OF THE FOLLOWING METHODS: MASONRY PRISM TESTING, MASONRY PRISM TEST RECORD OR UNIT STRENGTH METHOD. FIVE MASONRY PRISM TESTS SHALL BE BUILT AND TESTED PRIOR TO CONSTRUCTION. THREE MASONRY PRISM TESTS (PER 5,000 SQ. FT. OF FLOOR AREA, 3 MIN.) SHALL BE BUILT AND TESTED DURING CONSTRUCTION WHEN FULL STRESSES ARE USED IN DESIGN.

FOUNDATIONS

- THE DESIGN OF THE FOUNDATION SYSTEM IS BASED UPON THE BUILDING CODE MINIMUM RECOMMENDATIONS AND DEFAULT VALUES. THE OWNER MAY ELECT TO HAVE A GEOTECHNICAL ENGINEER REVIEW THE SPECIFIC SOILS ON THE SITE TO VERIFY THE DEFAULT DESIGN VALUES ARE ADEQUATE FOR BEARING, DIFFERENTIAL SETTLEMENT, PROTECTION FROM CORROSIVE SOILS, ETC. IF ANY POTENTIALLY UNFAVORABLE SOILS CONDITIONS ARE ENCOUNTERED DURING CONSTRUCTION, THE SERVICES OF A GEOTECHNICAL ENGINEER WILL BE REQUIRED.
- THE ALLOWABLE SOIL BEARING PRESSURE IS 1,500 PSF (IN COMPETENT NATIVE SOILS OR 90% COMPACTED FILL)
- REMOVE LOOSE SOIL AND STANDING WATER FROM FOUNDATION EXCAVATIONS PRIOR TO PLACING CONCRETE. THE GEOTECHNICAL ENGINEER SHALL INSPECT AND APPROVE ALL EXCAVATIONS, SOIL COMPACTION WORK PRIOR TO PLACEMENT OF ANY REBAR OR CONCRETE, SHORING INSTALLATIONS, BACKFILL MATERIALS AND BACK FILLING PROCEDURES.
- LOCATE AND PROTECT EXISTING UTILITIES TO REMAIN DURING AND/OR AFTER CONSTRUCTION.
- REMOVE ABANDONED FOOTINGS, UTILITIES, ETC. WHICH INTERFERE WITH NEW CONSTRUCTION, UNLESS OTHERWISE INDICATED.
- NOTIFY THE OWNER'S REPRESENTATIVE IF ANY BURIED STRUCTURES NOT INDICATED, SUCH AS CESSPOOLS, CISTERNS, FOUNDATIONS, ETC., ARE FOUND.
- THE CONTRACTOR IS SOLELY RESPONSIBLE FOR EXCAVATION PROCEDURES INCLUDING LAGGING, SHORING, UNDERPINNING AND PROTECTION OF EXISTING CONSTRUCTION.

METAL DECK

- STEEL DECK SHALL HAVE MINIMUM PROPERTIES AND BE THE TYPE AS INDICATED ON DRAWINGS.
- "S" AND "I" VALUES SHALL BE DETERMINED ACCORDING TO THE "LIGHT GAUGE STEEL INSTITUTE".
- ALL FLOOR AND ROOF DECK TO BE GALVANIZED IN ACCORDANCE WITH ASTM A653 COATING CLASS G90. REPAIR DAMAGED COATING.
- UNLESS NOTED OTHERWISE ON THE STRUCTURAL DRAWINGS, MINIMUM DECK ATTACHMENT IS AS FOLLOWS: 1/2" EFFECTIVE DIAMETER PUDDLE WELDS AT 12" O.C. AT TRANSVERSE AND PERIMETER SUPPORTS. 1/2" EFFECTIVE DIAMETER PUDDLE WELDS AT 16" O.C. AT LONGITUDINAL SUPPORTS. 3/16" BUTTON PUNCH OR 1-1/2" TOP SEAM WELD AT 36" O.C. AT SIDE LAP CONNECTIONS.
- ALL STEEL DECK SHALL BE CONTINUOUS OVER AT LEAST TWO SPANS EXCEPT ON SINGLE SPANS WHICH ARE SPECIFICALLY NOTED.
- ALL WELDING OF STEEL DECK SHALL BE PERFORMED BY LIGHT GAUGE WELDERS CERTIFIED BY CITY.
- STEEL DECK SHALL BE MANUFACTURED BY "VERCO" (ICC-ESR 1735P, LARR #23789)
- PROVIDE WELDS WASHERS IN ACCORDANCE WITH AWS D1.3 FOR ALL STEEL DECKS LIGHTER THAN 22 GAUGE.

STRUCTURAL OBSERVATION

PERIODIC STRUCTURAL OBSERVATION SHALL BE PROVIDED BY ORION STRUCTURAL GROUP INC. CONTRACTOR SHALL NOTIFY ENGINEER 72 HOURS BEFORE REQUIRED OBSERVATIONS. DELINQUENT NOTIFICATION MAY REQUIRE DEMOLITION OF COVERING MATERIAL TO FACILITATE OBSERVATION.

STRUCTURAL OBSERVATION PROGRAM AND DESIGNATION OF THE STRUCTURAL OBSERVER			
STRUCTURAL OBSERVATION (ONLY CHECKED ITEMS ARE REQUIRED)			
FIRM OR INDIVIDUAL TO BE RESPONSIBLE FOR THE STRUCTURAL OBSERVATION: NAME: ORION STRUCTURAL GROUP INC. PHONE: (805) 750-8136 CA. REGISTRATION: 5430 WILL LAMBERT			
FOUNDATION	WALL	FRAME	DIAPHRAGM
FOOTINGS, STEM WALLS, PIERS	<input checked="" type="checkbox"/> CONCRETE	<input type="checkbox"/> STEEL MOMENT FRAME	<input type="checkbox"/> CONCRETE
MAT FOUNDATION	<input type="checkbox"/> MASONRY	<input checked="" type="checkbox"/> BRACED FRAME	<input type="checkbox"/> STEEL DECK
CAISSONS, PILES, GRADE BEAMS	<input type="checkbox"/> WOOD	<input type="checkbox"/> CONCRETE MOMENT FRAME	<input type="checkbox"/> WOOD
STEPP'G/RET'G FOUND. - HILLSIDE SPECIAL ANCHORS	<input type="checkbox"/> HARDY FRAMES STRONG WALLS	<input type="checkbox"/> MASONRY FRAME	<input type="checkbox"/>
EMBEDDED ANCHORS	<input checked="" type="checkbox"/> OTHERS	<input type="checkbox"/> OTHERS	<input type="checkbox"/> OTHERS

GENERAL

- ALL DESIGN, CONSTRUCTION, AND WORKMANSHIP SHALL CONFORM TO THE 2019 EDITION OF THE CALIFORNIA BUILDING CODE (CBC), AND ALL LOCAL ORDINANCES AND REQUIREMENTS.
- STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION.
- THE GENERAL CONTRACTOR SHALL VERIFY ALL DIMENSIONS PRIOR TO STARTING SITE CONSTRUCTION. THE ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCIES OR INCONSISTENCIES.
- DIMENSIONS SHALL TAKE PRECEDENCE OVER SCALES ON DRAWINGS. NOTES AND DETAILS ON DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS.
- IT SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO ENSURE THAT ALL APPLICABLE SAFETY LAWS ARE STRICTLY ENFORCED AND TO MAINTAIN A SAFE CONSTRUCTION PROJECT.
- IT SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO PROVIDE SUPERVISION OF THE CONSTRUCTION WORK TO ENSURE THAT IT IS BUILT IN CONFORMANCE WITH THE APPROVED PLANS AND SPECIFICATIONS. THE ENGINEER WILL PROVIDE ONLY OBSERVATION OF THE WORK DURING CONSTRUCTION AS REQUIRED.
- THE APPROVED SET OF CONSTRUCTION DOCUMENTS, INCLUDING ALL APPROVED REVISIONS, SHALL BE PRESENT AT THE FABRICATION SITE AND JOB SITE AT ALL TIMES.
- CONSTRUCTION MATERIALS SHALL BE SPREAD OUT IF PLACED ON FRAMED FLOORS OR ROOFS. LOADS SHALL NOT EXCEED THE DESIGN LOADING FOR THE SUPPORTING MEMBERS.
- FIELD WORK (SITE BUILT) AND FIELD WELDING NOTED ON DRAWINGS IS A SUGGESTED CONSTRUCTION PROCEDURE ONLY. CONTRACTOR SHALL PROVIDE FIELD WORK AND FIELD WELDING AS REQUIRED FOR CONSTRUCTION.

CONCRETE NOTES

- CONCRETE MIXES SHALL BE DESIGNED BY A RECOGNIZED TESTING LABORATORY AND COPIES OF THE DESIGN SHALL BE SENT TO THE ARCHITECT AND THE ENGINEER. COMPRESSIVE STRENGTH TEST REPORTS SHALL BE SUBMITTED TO THE BUILDING DEPARTMENT AND THE ARCHITECT.
- PORTLAND CEMENT SHALL CONFORM TO ASTM C-150. TYPE II. AGGREGATE FOR STONE CONCRETE SHALL CONFORM TO ASTM C-33. WATER USED IN MIXING CONCRETE SHALL CONFORM TO ASTM C1602.
- ALL REINFORCING BARS, ANCHOR BOLTS, AND ALL OTHER CONCRETE INSERTS SHALL BE WELL SECURED IN POSITION PRIOR TO PLACING CONCRETE.
- THE MAXIMUM SLUMP SHALL NOT EXCEED 4" +/- 1" FOR FOOTINGS, SLABS ON EARTH, AND MASS CONCRETE, AND 5" +/- 1" FOR OTHER CONCRETE.
- MINIMUM ULTIMATE COMPRESSIVE STRENGTH AT 28 DAYS SHALL BE AS FOLLOWS: (MINIMUM 5 SACKS OF CEMENT PER CUBIC YARD) (MAXIMUM WATER/CEMENT RATIO TO BE 0.45).

A. CONCRETE FOUNDATIONS.....	3,000 P.S.I. (NORMAL WEIGHT)
B. CONCRETE FILL ON METAL DECK.....	3,000 P.S.I. (LIGHT WEIGHT)
- ALL STRUCTURAL CONCRETE IS TO BE REINFORCED.
- HIGH STRENGTH GROUT SHALL BE QUIKRETE NON-SHRINK PRECISION GROUT PER ASTM C109/C109M AND SHALL HAVE A MINIMUM ULTIMATE COMPRESSIVE STRENGTH OF 14,000 PSI AT 28 DAYS (DEPUTY INSP. REQ'D).

DESIGN CRITERIA

- NEW WORK SHALL COMPLY WITH THE 2019 CALIFORNIA BUILDING CODE.
- VERTICAL LIVE LOADS:

A. ROOF	20 PSF
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- LATERAL LOADS:

A. WIND:	
BASIC WIND SPEED:	95 MPH
WIND IMPORTANCE FACTOR, Iw:	1.0
EXPOSURE TYPE:	C
B. SEISMIC:	
SITE CLASS:	D-DEFAULT
SEISMIC DESIGN CATEGORY:	I
RISK CATEGORY:	I
SEISMIC IMPORTANCE FACTOR, I:	1.0
SS =	1.9910
S1 =	0.731
FA =	1.2
FV =	1.5
SDS =	1.5928
SD1 =	0.7310
SEISMIC FORCE RESISTING SYSTEM:	STEEL ORDINARY CANTILEVER COLUMN SYSTEM +SPECIAL REINFORCED MASONRY SHEAR WALL
- ANALYSIS PROCEDURE USED:

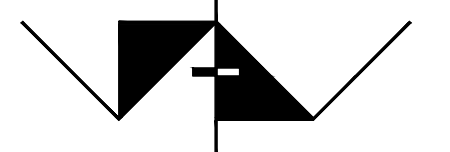
RESPONSE MODIFICATION FACTOR, R:	1.25
CS:	1.274 (LRFD)
	0.892 (ASD)

PROJECT TITLE
20-MPC-037

REPAIR BASEBALL DUGOUTS

7075 CAMPUS ROAD
MOORPARK, CALIFORNIA 93021

COMMISSIONED ARCHITECT



AMADOR WHITTLE ARCHITECTS, INC.

28328 AGOURA ROAD, SUITE 203
AGOURA HILLS, CA 91301
(805) 530-3838, (818) 874-0071

CONSULTANT

Orion Structural

Orion Structural Group, Inc.
239 East Thousand Oaks Boulevard, Suite 304
Thousand Oaks, California 91360 - 7734
Phone: 805.390.9242 Fax: 805.494.0148 O.S.G. # 21604

STAMPS/SEALS



SIGNED DATE: 05/11/2021

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SHEET TITLE:

GENERAL NOTES

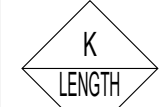
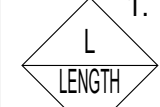
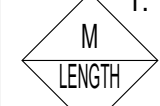
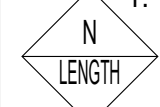
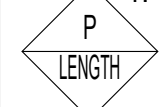
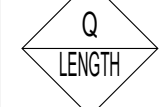
PROJECT NO:	020-MPC-037	PROJECT ARCH:	Designer
DRAWN:	MH	CHECKED:	WL

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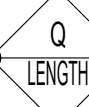
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DATE: 05/11/2021 SHEET: ___ OF ___

SHEAR WALL SCHEDULE
(2019 CBC - AF&PA SDPWS 2008, TABLE 4.3A)

MARK	ALLOWABLE SHEAR	PLYWOOD	EDGE NAILING	FASTENERS @ SOLE PLATE TO BLKG.	ANCHOR BOLTS @ FOOTING	'A35' @ BLKG. TO TOP PLATE
	280 #/	1/2" STR. 1	8d @ 6" O.C.	16d @ 6" O.C.	5/8" X 7" EMBED. @ 4'-0" O.C.	@ 1'-4" O.C.
	430 #/	1/2" STR. 1	8d @ 4" O.C.	16d @ 4" O.C.	5/8" X 7" EMBED. @ 3'-0" O.C.	@ 1'-4" O.C.
	550 #/	1/2" STR. 1	8d @ 3" O.C.	SIMPSON 'SDS' 1/4" X 6" SCREWS @ 12" O.C. STAGG.	5/8" X 7" EMBED. @ 2'-0" O.C.	@ 8" O.C.
	730 #/	1/2" STR. 1	8d @ 2" O.C.	SIMPSON 'SDS' 1/4" X 6" SCREWS @ 8" O.C. STAGG.	5/8" X 7" EMBED. @ 2'-0" O.C.	@ 8" O.C.
	870 #/	1/2" STR. 1	10d @ 2" O.C.	SIMPSON 'SDS' 1/4" X 6" SCREWS @ 6" O.C. STAGG.	5/8" X 7" EMBED. @ 1'-6" O.C.	@ 8" O.C.
	1042 #/	1/2" STR. 1 BOTH SIDES	8d @ 3" O.C. BOTH SIDES	SIMPSON 'SDS' 1/4" X 6" SCREWS @ 6" O.C. STAGG.	5/8" X 7" EMBED. @ 1'-0" O.C.	@ 8" O.C.

NOTE:

-  DENOTES PERFORATED SHEAR WALLS PER PLAN &
1. WHERE ALLOWABLE SHEAR VALUES EXCEED 300 POUNDS PER FOOT (5.11 N/mm), FOUNDATION SILL PLATES AND ALL FRAMING MEMBERS RECEIVING EDGE NAILING FROM ABUTTING PANELS SHALL NOT BE LESS THAN A SINGLE 3-INCH (76mm) NOMINAL MEMBER. NAILS SHALL BE STAGGERED.
- REQUIRE MINIMUM 1/2" EDGE DISTANCE FOR NAILING AT THE 3X BOUNDARY AND PANEL EDGE MEMBERS OF THESE SHEAR WALLS.
- FIELD NAIL SHALL BE AT 12" O.C. AND NAIL SIZE SHALL BE BASED ON EDGE NAILING (E.N.) SIZE (8d E.N. = 8d F.N. AND 10d E.N. = 10d F.N.)
- 1/2" STRUCTURAL 1 PLYWOOD SHALL HAVE 4 PLY LAMINATIONS.
- ALL BOLTS HOLES SHALL BE 1/16" (MAX.) OVER SIZED AT THE CONNECTION OF THE HOLD DOWN POSTS. (INSPECTOR TO VERIFY)
- HOLD DOWN CONNECTION BOLTS AND NUTS SHALL BE TORQUED 1/2 TURN BEYOND FINGER TIGHT OR AS REQ'D. BY THE MANUF. (INSPECTOR SHALL VERIFY BY RANDOM INSPECTION PRIOR TO COVERING THE WALLS).
- APPROVED PLATE WASHERS, IN-LIEU OF CUT WASHERS, SHALL BE PROVIDED FOR ALL PLYWOOD SHEAR WALL SILL PLATE ANCHOR BOLTS, PER TABLE 'A' BELOW.
- APPROVED PLATE WASHERS, IN-LIEU OF CUT WASHERS, SHALL BE PROVIDED FOR HOLD DOWN CONNECTORS BOLTS AT SHEAR WALL WOOD FLANGES, PER TABLE 'A' BELOW.
- SIMPSON LTP4 MAY BE USED IN-LIEU OF SIMPSON A35 AND A35F.
- A.B. EMBED. IS THE MINIMUM EMBEDMENT INTO FOOTING.
- WHERE PANELS ARE APPLIED ON BOTH FACES OF A WALL AND NAIL SPACING IS LESS THAN 6 INCHES (152mm) ON CENTER ON EITHER SIDE. PANEL JOINTS SHALL BE OFFSET TO FALL ON DIFFERENT FRAMING MEMBERS OR FRAMING SHALL BE 3-INCH (76mm) NOMINAL OR THICKER AND NAILS ON EACH SIDE SHALL BE STAGGERED.
- WHERE SIMPSON 'SDS' SCREWS ARE USED AS SOLE PLATE FASTENERS PROVIDE 3X RIM OR 3X BLKG. BELOW MIN.

BOLT SIZE	PLATE WASHER SIZE
5/8"	1 1/4" X 3" X 3"
3/4"	5/16" X 3" X 3"
7/8"	5/16" X 3" X 3"
1"	3/8" X 3-1/2" X 3-1/2"

FASTENING SCHEDULE
2019 CBC, TABLE 2304.10.1

CONNECTION	FASTENING (a,m)	LOCATION
1. JOIST TO SILL OR GIRDER	(3)-8d COMMON (2)3"x0.131" (3)-3"x0.131 NAILS	TOENAIL
2. BRIDGING TO JOIST	(2)-8d COMMON (2)3"x0.131" (2)-3"x0.131 NAILS	TOENAIL EACH END
3. 1"X6" SUBFLOOR OR LESS TO EACH JOIST	(2)-8d COMMON (2)3"x0.131"	FACE NAIL
4. WIDER THAN 1"X6" SUBFLOOR TO EACH JOIST	(3)-8d COMMON (2)3"x0.131"	FACE NAIL
5. 2" SUBFLOOR TO JOIST OR GIRDER	(2)-16d COMMON (3)3"x0.162"	BLIND AND FACE NAIL
6. SOLE PLATE TO JOIST OR BLOCKING	16d (3)3"x0.135" AT 16" O.C. 3"x0.131" NAIL AT 8" O.C.	TYPICAL FACE NAIL
SOLE PLATE TO JOIST OR BLOCKING AT BRACED WALL PANEL	(3)-16d (3)3"x0.135" AT 16" (4)-3"x0.131" NAILS AT 16" O.C.	BRACED WALL PANELS
7. TOP PLATE TO STUD	(2)-16d COMMON (3)3"x0.162" (3)-3"x0.131" NAILS	END NAIL
8. STUD TO SOLE PLATE	(4)-8d COMMON (2)3"x0.131" (4)-3"x0.131 NAILS	TOENAIL
	(2)-16d COMMON (3)3"x0.162" (3)-3"x0.131" NAILS	END NAIL
9. DOUBLE STUDS	16d (3)3"x0.135" AT 24" O.C. (3)-3"x0.131" NAILS AT 8" O.C.	FACE NAIL
10. DOUBLE TOP PLATES	16d (3)3"x0.135" AT 16" O.C. (3)-3"x0.131" NAILS AT 12" O.C.	TYPICAL FACE NAIL
DOUBLE TOP PLATES	(8)-16d COMMON (3)3"x0.162" (12)-3"x0.131" NAILS	LAP SPLICE
11. BLOCKING BETWEEN JOIST OR RAFTERS TO TOP PLATE	(3)-8d COMMON (2)3"x0.131" (3)-3"x0.131" NAILS	TOENAIL
12. RIM JOIST TO TOP PLATE	8d COMMON (2)3"x0.131" AT 6" O.C. (3)-3"x0.131" NAIL AT 6" O.C.	TOENAIL
13. TOP PLATES, LAPS AND INTERSECTIONS	(2)-16d COMMON (3)3"x0.162" (3)-3"x0.131" NAILS	FACE NAIL
14. CONTINUOUS HEADER, TWO PIECES	16d COMMON (3)3"x0.162"	16" O.C. ALONG EDGE
15. CEILING JOIST TO PLATE	(3)-8d COMMON (2)3"x0.131" (5)-3"x0.131" NAIL	TOENAIL
16. CONTINUOUS HEADER TO STUD	(4)-8d COMMON (2)3"x0.131"	TOENAIL
17. CEILING JOISTS, LAPS OVER PARTITIONS (SEE SECTION 2308.10.4.1, TABLE 2308.10.4.1)	(3)-16d COMMON (3)3"x0.162" MINIMUM, TABLE 2308.10.4.1 (4)-3"x0.131" NAILS	FACE NAIL
18. CEILING JOISTS TO PARALLEL RAFTER (SEE SECTION 2308.10.4.1, TABLE 2308.10.4.1)	(3)-16d COMMON (3)3"x0.162" MINIMUM, TABLE 2308.10.4.1 (4)-3"x0.131" NAILS	FACE NAIL
19. RAFTER TO PLATE (SEE SECTION 2308.10.1, TABLE 2308.10.1)	(3)-8d COMMON (2)3"x0.131" (3)-3"x0.131" NAILS	TOENAIL
20. 1" DIAGONAL BRACE TO EACH STUD AND PLATE.	(2)-8d COMMON (2)3"x0.131" (2)-3"x0.131" NAILS	FACE NAIL
21. 1" X 8" SHEATHING TO EACH BEARING	(3)-8d COMMON (2)3"x0.131"	FACE NAIL
22. WIDER THAN 1" X 8" SHEATHING TO EACH BEARING	(3)-8d COMMON (2)3"x0.131"	FACE NAIL
23. BUILT UP CORNER STUDS	16d COMMON (3)3"x0.162" 3"x0.131" NAILS	24" O.C. 16" O.C.
24. BUILT UP GIRDER AND BEAMS	20d COMMON (4"x0.192") 32" O.C. 3"x0.131 NAILS AT 24" O.C.	FACE NAIL AT TOP AND BOTTOM STAGGERED ON OPPOSITE SIDES
	(2)-20d COMMON (4"x0.192") (3)-3"x0.131 NAILS	FACE NAIL AT ENDS AND AT EACH SPLICE.
25. 2" PLANKS	16d COMMON (3)3"x0.162"	AT EACH BEARING
26. COLLAR TIE TO RAFTER	(3)-10d COMMON (3"x0.148") (4)-3"x0.131" NAILS	FACE NAILING
27. JACK RAFTER TO HIP	(3)-10d COMMON (3"x0.148") (4)-3"x0.131" NAILS	TOE NAIL
	(2)-16d COMMON (3)3"x0.162" (3)-3"x0.131 NAILS	FACE NAIL
28. ROOF RAFTER TO 2X RIDGE BEAM	2-16"d COMMON (3"x0.148") (4)-3"x0.131"	TOE NAIL
	(2)-16" COMMON (3)3"x0.162" (3)-3"x0.131" NAILS	FACE NAIL
29. JOIST TO BAND JOIST	16d COMMON (3)3"x0.162" 4-3"x0.131" NAILS	FACE NAIL
30. LEDGER STRIP	(3)-16d COMMON (3)3"x0.162" (4)-3"x0.131 NAILS	FACE NAIL
31. WOOD STRUCTURE PANELS AND PARTICLE-BOARD (b) SUBFLOOR, ROOF AND WALL SHEATHING (TO FRAMING)	1/2" AND LESS 2-3/8" X 0.13" NAIL (n) 3/8" TO 1/2" 8d (d) OR 6d (e) 2-3/8" X 0.113" NAIL (p) 1/2" TO 1" 8d (c) 1-1/8" TO 1-1/4" 10d (d) OR 8d (d) 1/2" AND LESS 6d (e) 1/2" TO 1" 8d (e) 1-1/8" TO 1-1/4" 10d (d) OR 8d (e)	
32. PANEL SIDING (TO FRAMING)	1/2" AND LESS 5/8"	6d (f) 8d (f)
33. FIBERBOARD SHEATHING (g)	1/2" 25/32"	NO. 11 GAGE ROOFING NAIL (h) 6d COMMON NAIL (2" X 0.113") NO. 11 GAGE ROOFING NAIL (h) 8d COMMON NAIL (2-1/2" X 0.131")

FASTENING SCHEDULE NOTES

FOR SI: 1 INCH = 25.4 mm

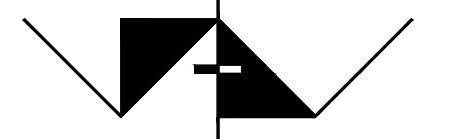
- COMMON OR BOX NAILS ARE PERMITTED TO BE USED EXCEPT WHERE OTHERWISE STATED.
- NAILS SPACED AT 6 INCHES ON CENTER AT EDGES, 12" INCHES AT INTERMEDIATE SUPPORTS EXCEPT 6 INCHES AT SUPPORTS WHERE SPANS ARE 48 INCHES OR MORE. FOR NAILING OF WOOD STRUCTURAL PANELS AND PARTICLE BOARD DIAGRAMMS AND SHEAR WALLS REFER TO SECTION 2305. NAILS FOR WALL SHEATHING ARE PERMITTED TO BE COMMON, BOX OR CASING.
- COMMON OR DEFORMED SHANK (6d - 2"x0.113"; 8d - 2"x0.131"; 10d - 3"x0.148")
- COMMON (6d - 2"x0.113"; 8d - 2 1/2"x0.131"; 10d - 3"x0.148")
- DEFORMED SHANK (6d - 2"x0.113"; 8d-2)3"x0.131"; 10d - 3"x0.148")
- CORROSION-RESISTANT SIDING (6d - 1 1/2"x0.106"; 8d - 2)3"x0.128") OR CASING (6d - 2" X 0.099"; 8d - 2)3"x0.113") NAIL.
- FASTENERS SPACED 3 INCHES ON CENTER AT EXTERIOR EDGES AND 6 INCHES ON CENTER AT INTERMEDIATE SUPPORTS, WHEN USED AS STRUCTURAL SHEATHING. SPACING SHALL BE 6 INCHES ON CENTER ON THE EDGES AND 12 INCHES ON CENTER AT INTERMEDIATE SUPPORTS FOR NON-STRUCTURAL APPLICATIONS.
- CORROSION-RESISTANT ROOFING NAILS WITH 1/4" INCH DIAMETER HEAD AND 1-1/2 INCH LENGTH FOR 3/8" SHEATHING AND 1-3/4 INCH LENGTH FOR 1/2" INCH SHEATHING.
- CORROSION RESISTANT STAPLES WITH NORMAL 1/8" INCH CROWN AND 1 -1/8 INCH LENGTH FOR 1/2 INCH SHEATHING AND 1-1/2 INCH LENGTH FOR 3/8" INCH SHEATHING. PANEL SUPPORTS AT 16 INCHES (20 INCHES IF STRENGTH AXIS IN THE LONG DIRECTION OF THE PANEL, UNLESS OTHERWISE MARKED).
- CASING (1-1/2" X 0.080") OR FINISH (1-1/2" X 0.072") NAILS SPACED 6 INCHES ON PANEL EDGES, 12 INCHES AT INTERMEDIATE SUPPORTS.
- PANEL SUPPORTS AT 24 INCHES. CASING OF FINISH NAILS SPACED 6 INCHES ON PANEL EDGES, 12 INCHES AT INTERMEDIATE SUPPORTS.
- FOR ROOF SHEATHING APPLICATIONS, 8d NAILS (2-1/2" X 0.113") ARE THE MINIMUM REQUIRED FOR WOOD STRUCTURAL PANELS.
- STAPLES SHALL HAVE A MINIMUM CROWN WIDTH OF 7/16 INCH.
- FOR ROOF SHEATHING APPLICATIONS, FASTENERS SPACED 4 INCHES ON CENTER AT EDGES, 8 INCHES AT INTERMEDIATE SUPPORTS.
- FASTENERS SPACED 4 INCHES ON CENTER AT EDGES, 8 INCHES AT INTERMEDIATE SUPPORTS FOR SUBFLOOR AND WALL SHEATHING AND 3 INCHES ON CENTER AT EDGES, 6 INCHES AT INTERMEDIATE SUPPORTS FOR ROOF SHEATHING.
- FASTENERS SPACED 4 INCHES ON CENTER AT EDGES, 8 INCHES AT INTERMEDIATE SUPPORTS.

PROJECT TITLE
20-MPC-037

REPAIR BASEBALL DUGOUTS

7075 CAMPUS ROAD
MOORPARK, CALIFORNIA 93021

COMMISSIONED ARCHITECT



AMADOR WHITTLE ARCHITECTS, INC.

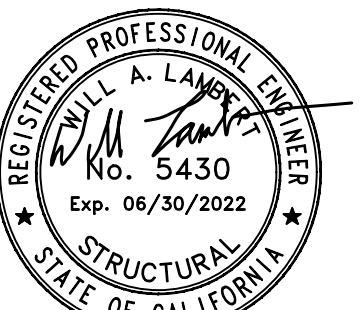
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CONSULTANT

Orion Structural

Orion Structural Group, Inc.
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STAMPS/SEALS



SIGNED DATE: 05/11/2021

SHEET TITLE:

GENERAL NOTES

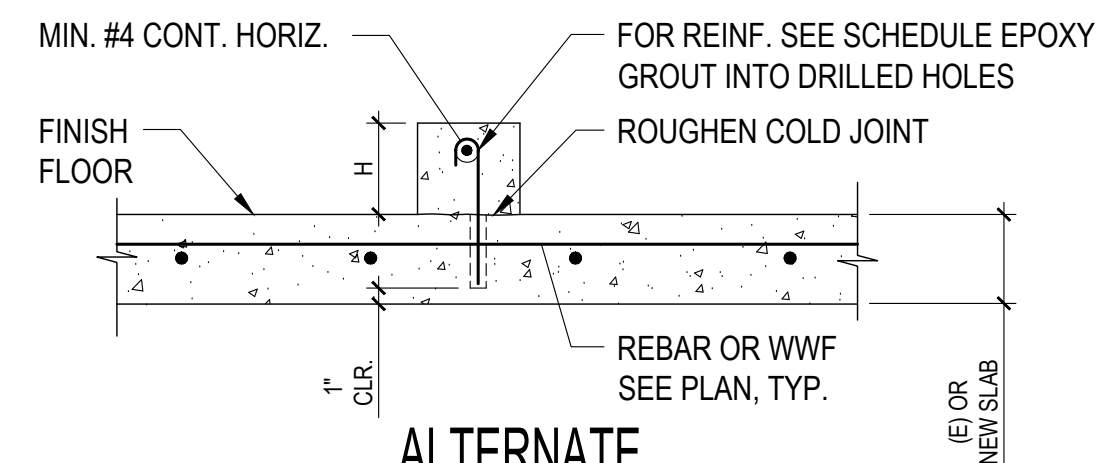
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DRAWN: MH CHECKED: WL

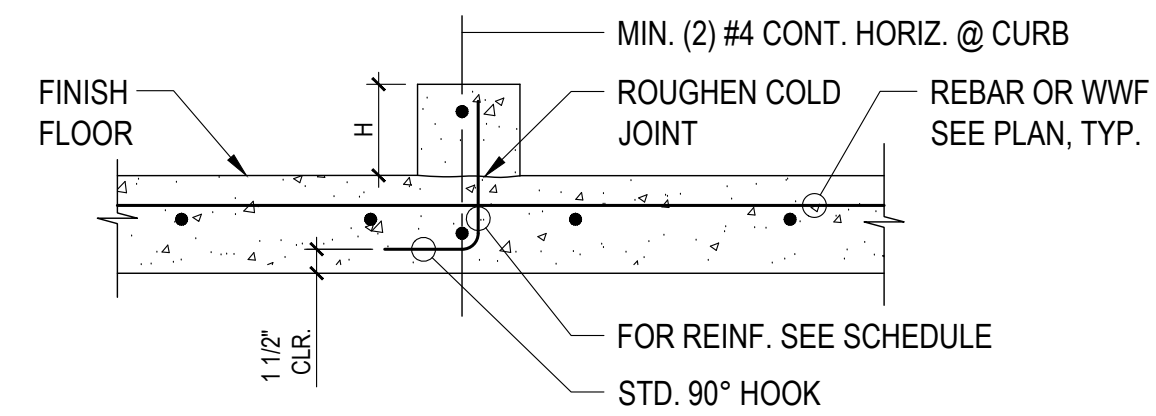
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S001

DATE: 05/11/2021 SHEET: ___ OF ___



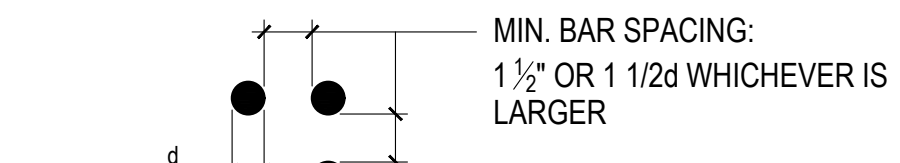
ALTERNATE



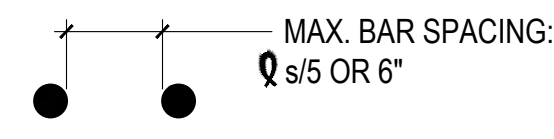
MAX. "H"	VERTICAL	HORIZ.
8"	#3 @ 18"	
18"	#4 @ 18"	
4'-0"	#4 @ 12"	#4 @ 12"

NOTE:
1. SEE ARCH'L DWGS FOR CURB LOCATION & SIZE

REIN. TO BE PLACED IN CURB WALL ϵ



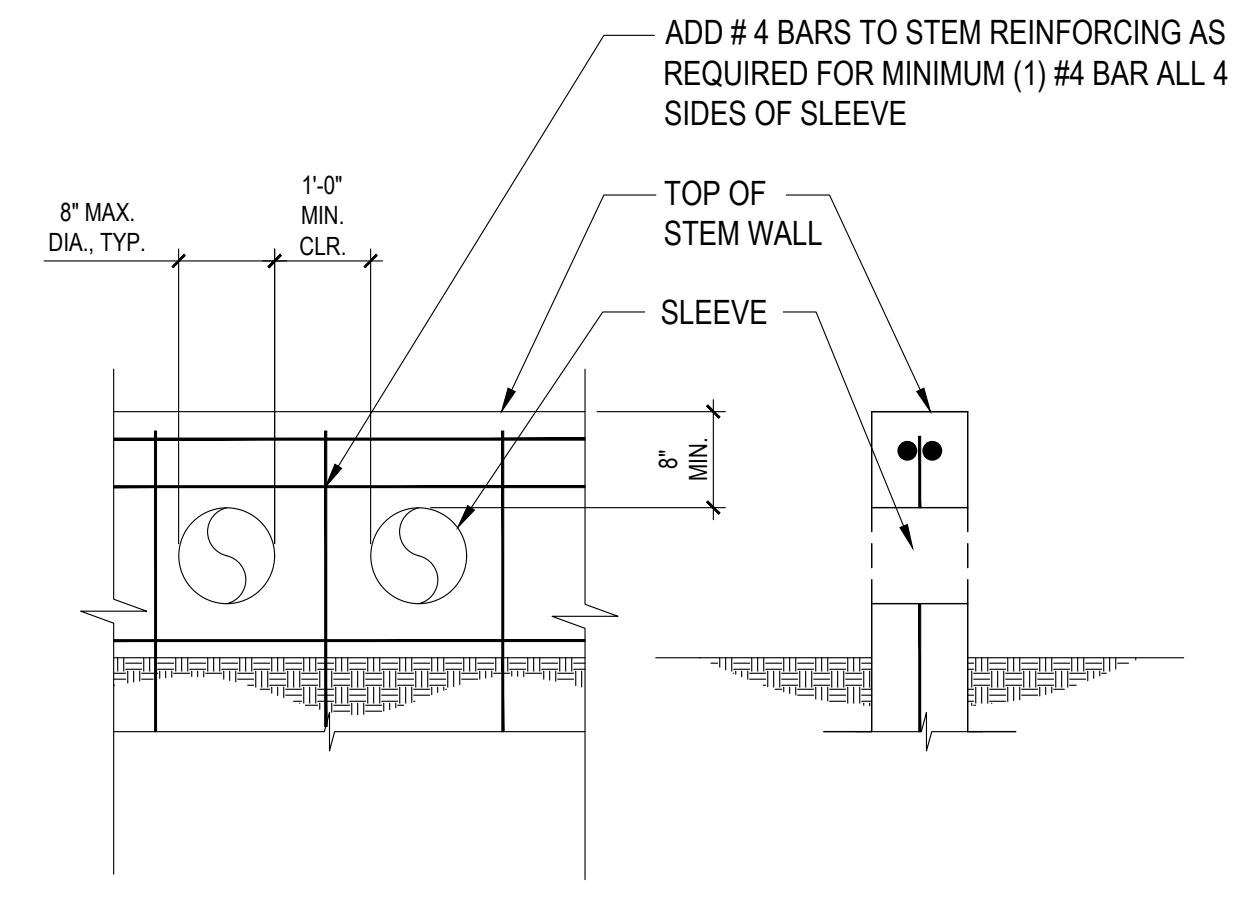
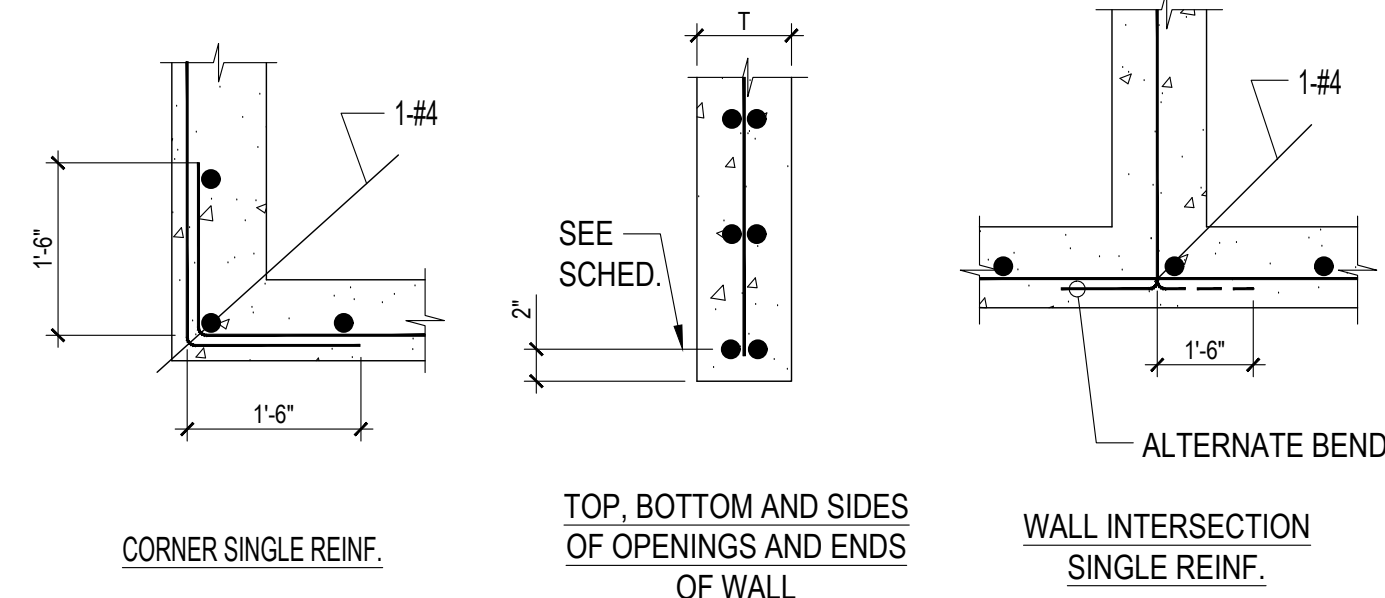
BAR SPACING FOR NON-SPLICED BARS



BAR SPACING FOR BARS SPLICED W/ NON-CONTACT LAP

NOTES:
WHERE REINFORCING IS NOT SHOWN OTHERWISE ON SECTIONS, DETAILS OR WALL ELEVATIONS.

MINIMUM REINFORCEMENT IN CONCRETE WALLS		
WALL THICKNESS T	REINFORCING EACH WAY	OPENING REINF.
8"	#4 @ 12" @ C.L.	2-#4



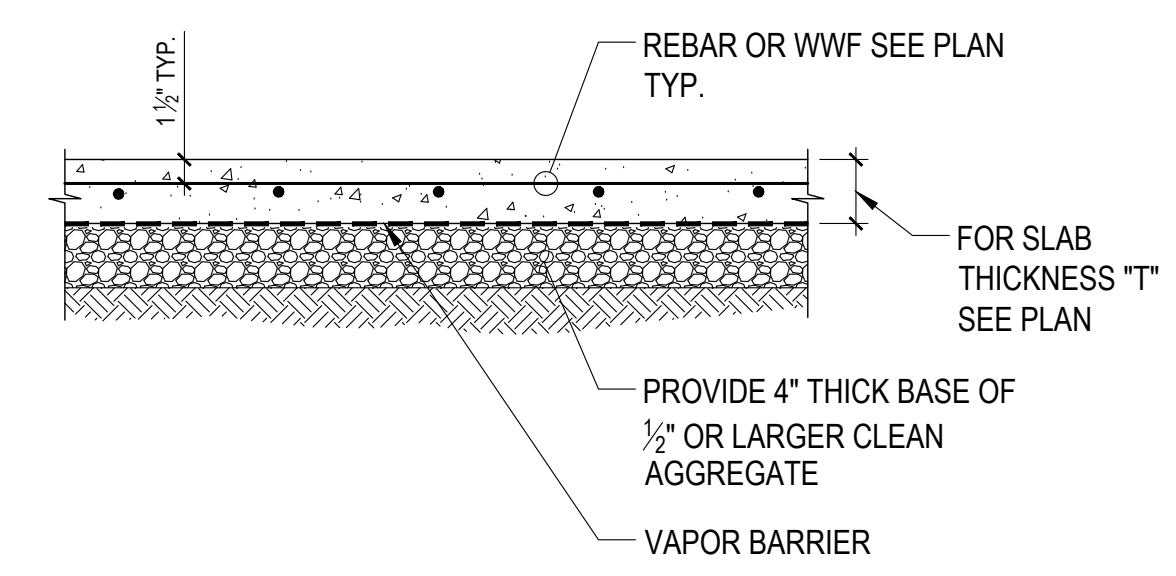
SLEEVE AT STEM WALLS

CONCRETE CURB DETAIL NOT TO SCALE 10

BAR SPACING NOT TO SCALE 7

TYPICAL CONCRETE WALL DETAILS NOT TO SCALE 4

SLEEVE AT STEM WALLS NOT TO SCALE 1

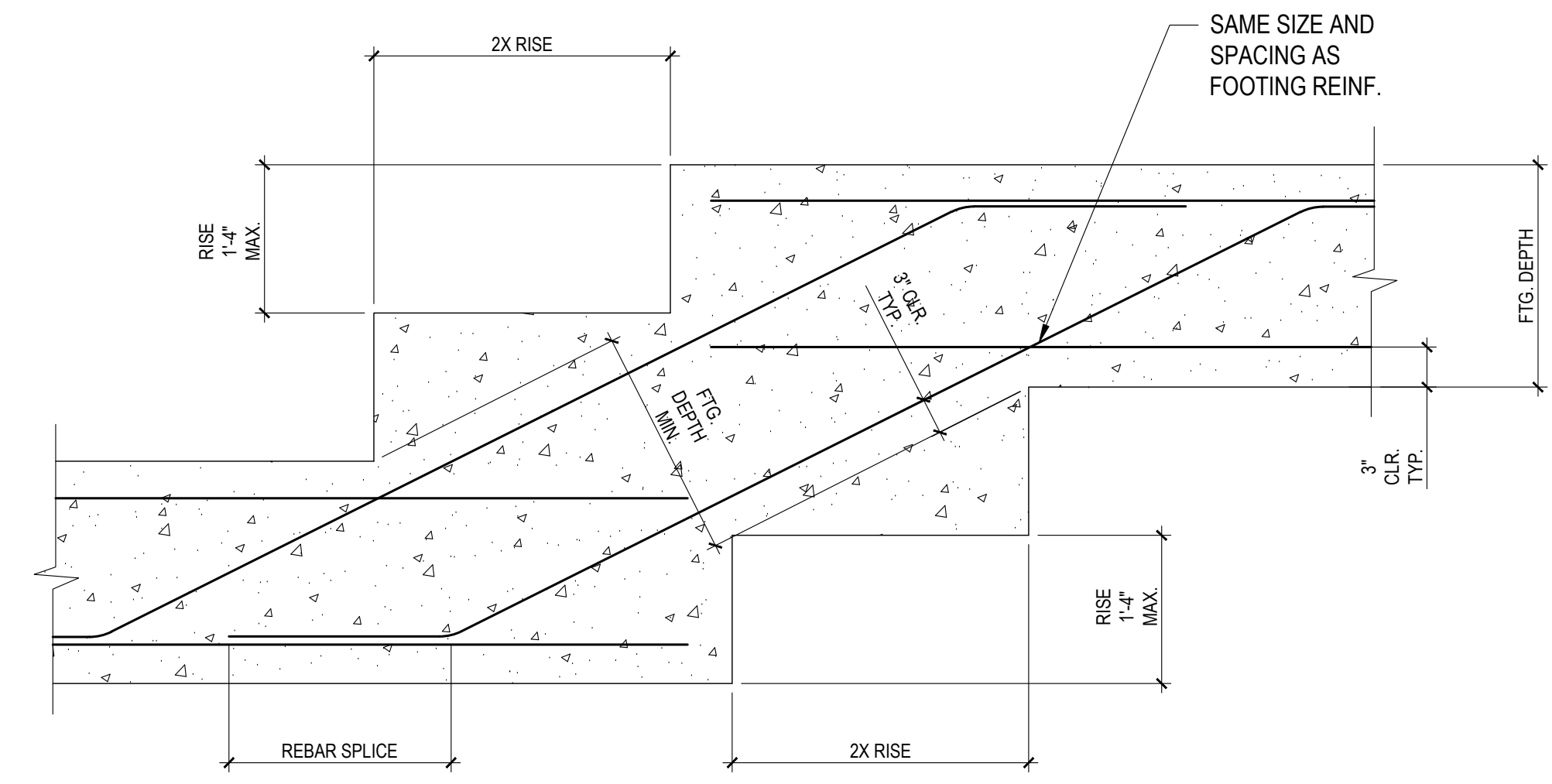


IF APPLICABLE, SEE SOILS REPORT FOR ADDITIONAL SUBGRADE PREPARATION

BAR SIZE	TENSION LAP SPLICE LENGTH (CLASS B)						DEVELOPMENT LENGTH (1d) (CLASS A)					
	3000 PSI CONC f _c		4000 PSI CONC f _c		5000 PSI CONC f _c		3000 PSI CONC f _c		4000 PSI CONC f _c		5000 PSI CONC f _c	
	TOP	OTHER	TOP	OTHER	TOP	OTHER	TOP	OTHER	TOP	OTHER	TOP	OTHER
#3	29	22	28	20	28	20	23	17	21	15	21	15
#4	39	29	34	25	30	23	30	22	26	19	23	17
#5	48	36	42	31	38	28	37	28	32	24	29	22
#6	58	43	50	37	45	34	45	33	39	29	35	26
#7	81	63	71	54	63	49	63	48	54	42	49	38
#8	93	72	81	62	72	56	72	55	62	48	56	43
#9	105	81	91	70	81	63	81	62	70	54	63	48
#10	118	91	102	79	92	70	91	70	79	61	70	54
#11	131	101	113	87	102	78	101	78	87	67	78	60

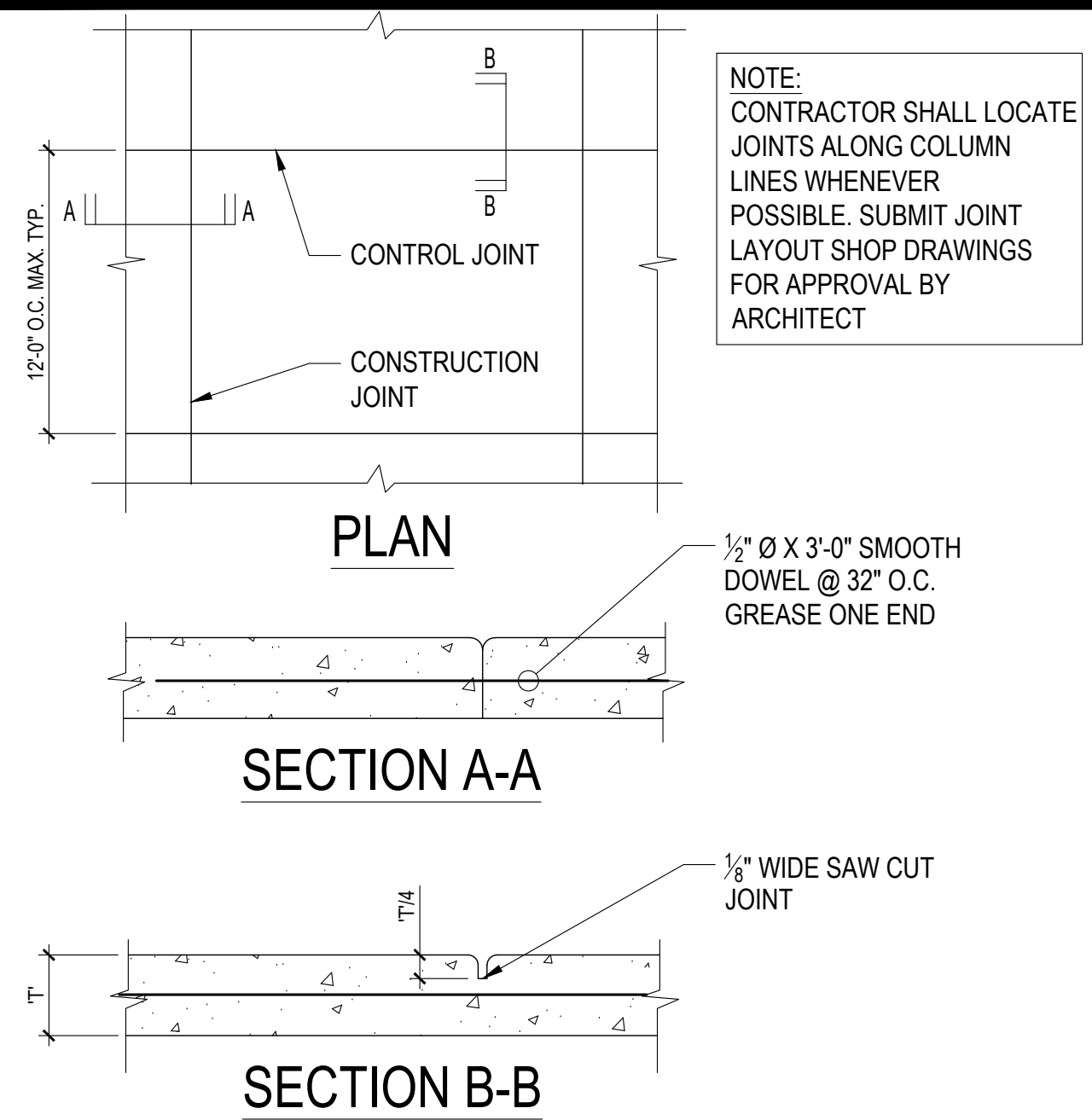
NOTES:
1. ALL SPLICES SHALL BE TENSION LAP SPLICES U.O.
2. LENGTHS SHOWN ARE FOR GRADE 60 UNCOATED BARS.
3. LENGTHS SHOWN ARE IN INCHES.
4. INCREASE LENGTHS 30% FOR LIGHT WEIGHT CONCRETE AND AT FOUR BAR BUNDLES (WHERE 2 BARS LAP WITH 2 OTHER BARS) INDIVIDUAL BARS WITHIN A BUNDLE SHALL NOT OVERLAP.
5. TOP BARS - HORIZONTAL BARS PLACED WITH MORE THAN 12" OF FRESH CONCRETE CAST BELOW THEM.
6. INCREASE LENGTHS 50% WHERE a < db OR WHERE b < db FOR BEAMS AND COLUMNS OR WHERE b < 2db FOR OTHER ELEMENTS.

WHERE:
"a" IS THE CLEAR COVER "b" IS THE CLEAR ϵ INCREASE LENGTHS 50% WHERE a < db OR WHERE b < db FOR BEAMS AND COLUMNS OR WHERE b < 2db FOR OTHER ELEMENTS.
SPACING "db" IS THE BAR DIA



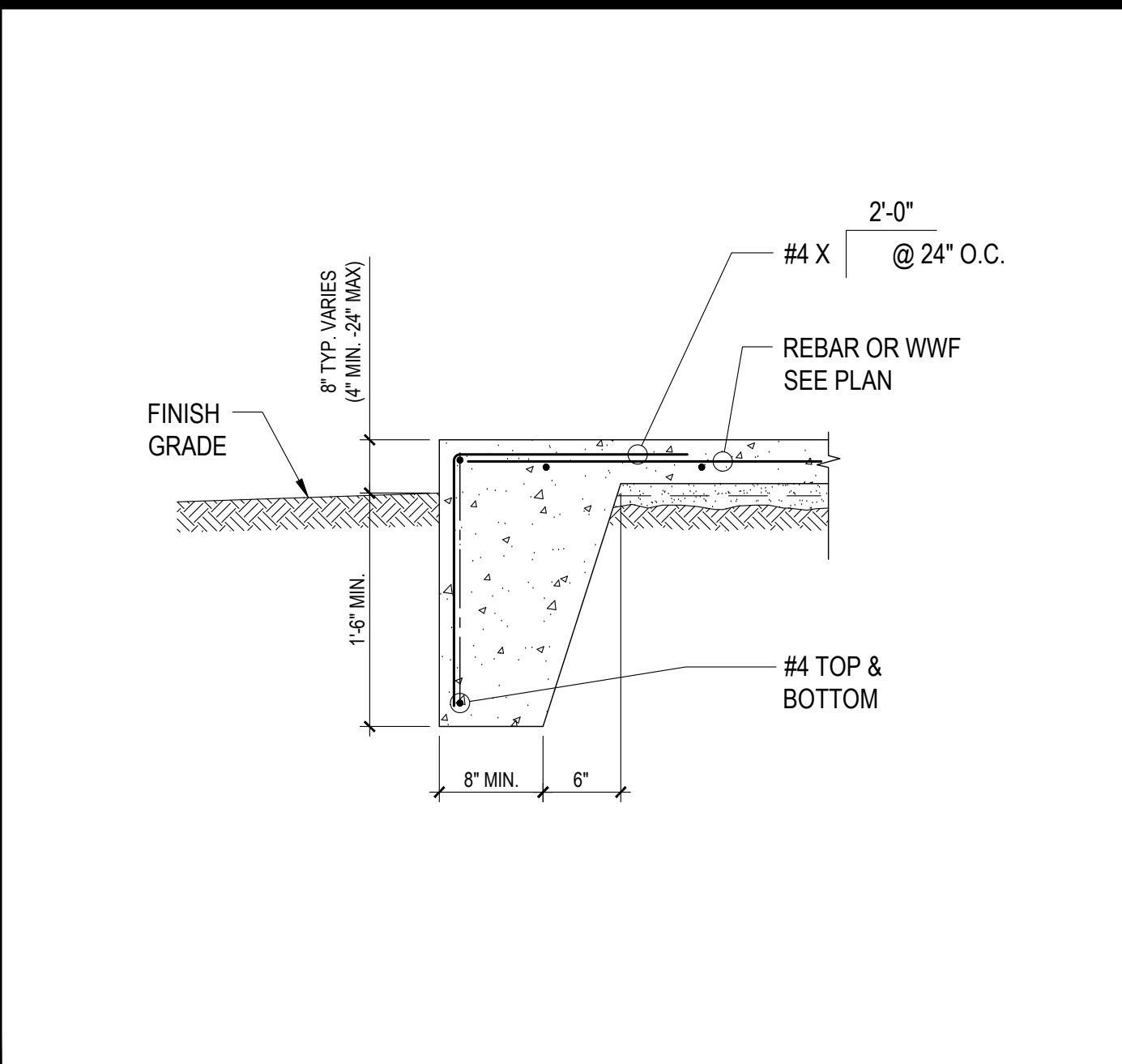
TYP. REINF. DEVEL. LENGTH / LAP SPLICE SCHED. NOT TO SCALE 8

TYPICAL STEPPED FOOTING NOT TO SCALE 2

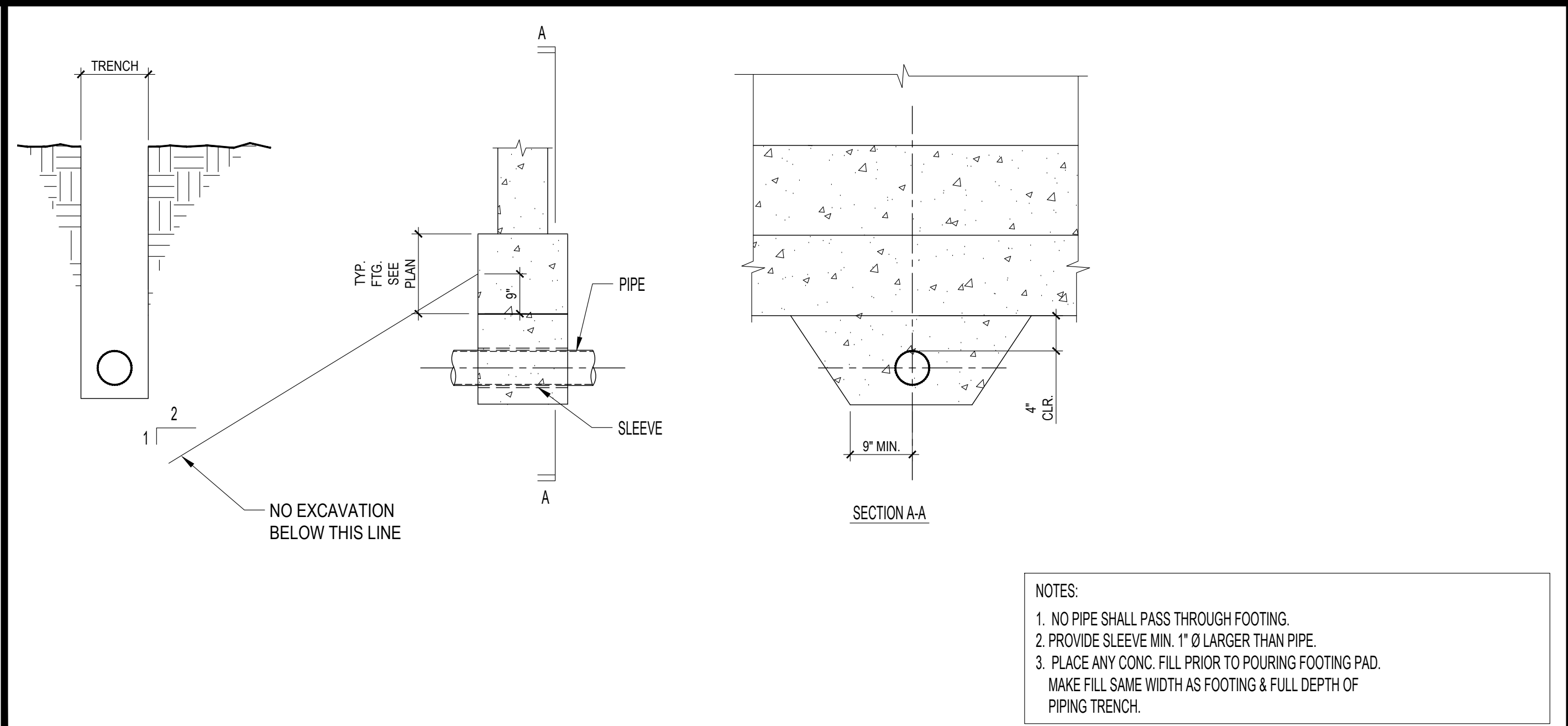


NOTE:
CONTRACTOR SHALL LOCATE JOINTS ALONG COLUMN LINES WHENEVER POSSIBLE. SUBMIT JOINT LAYOUT SHOP DRAWINGS FOR APPROVAL BY ARCHITECT

TYPICAL SLAB ON GRADE JOINT DETAIL NOT TO SCALE 12



SLAB ON GRADE EDGE DETAIL NOT TO SCALE 9



NOTES:
1. NO PIPE SHALL PASS THROUGH FOOTING.
2. PROVIDE SLEEVE MIN. 1" ϕ LARGER THAN PIPE.
3. PLACE ANY CONC. FILL PRIOR TO POURING FOOTING PAD. MAKE FILL SAME WIDTH AS FOOTING & FULL DEPTH OF PIPING TRENCH.

TYP PIPE BELOW FOOTING NOT TO SCALE 3

PROJECT TITLE
20-MPC-037
REPAIR BASEBALL DUGOUTS

7075 CAMPUS ROAD
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COMMISSIONED ARCHITECT
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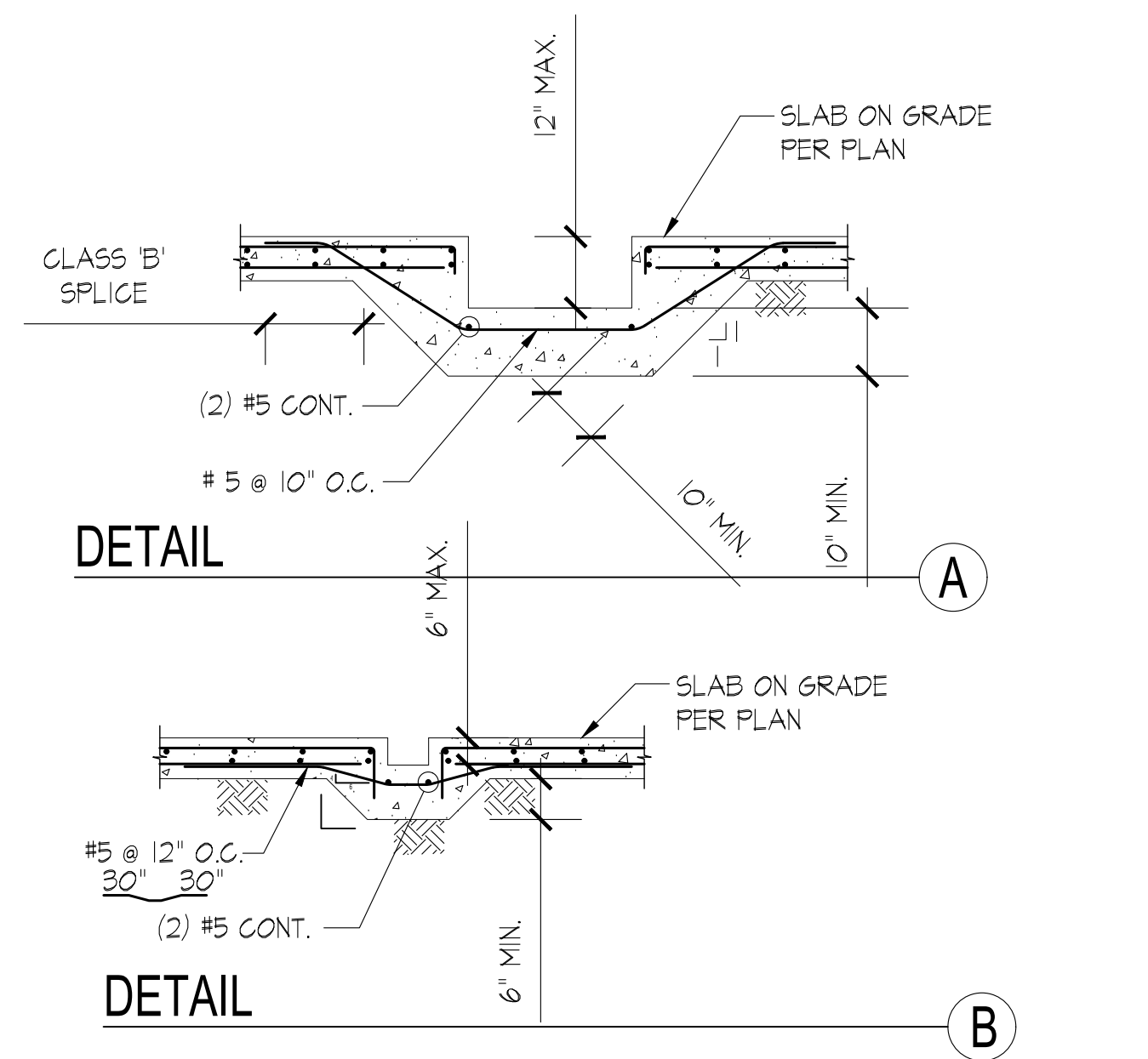
STAMPS/SEALS
LICENSED ARCHITECT
WILLIAM JORGE AMADOR
C-20348
JANUARY 31, 2023
RENEWAL DATE
REGISTERED PROFESSIONAL ENGINEER
WILLIAM A. LAVERGNE
No. 5430
Exp. 06/30/2022
STRUCTURAL
STATE OF CALIFORNIA
SIGNED DATE: 05/11/2021

SHEET TITLE:
TYPICAL DETAILS

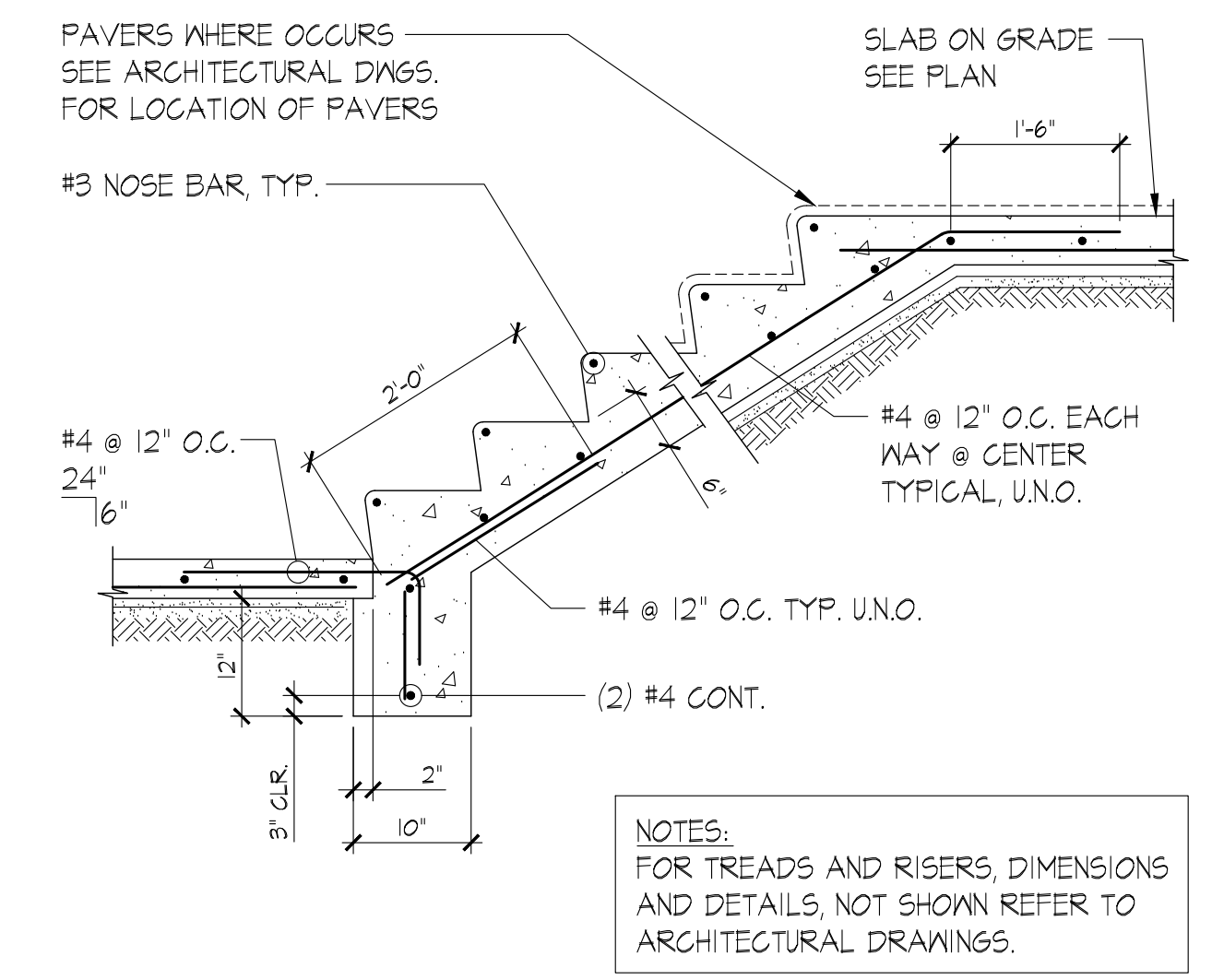
PROJECT NO.: 020-MPC-037 PROJECT ARCH: Designer
DRAWN: MH CHECKED: WL

SHEET NUMBER:
S010

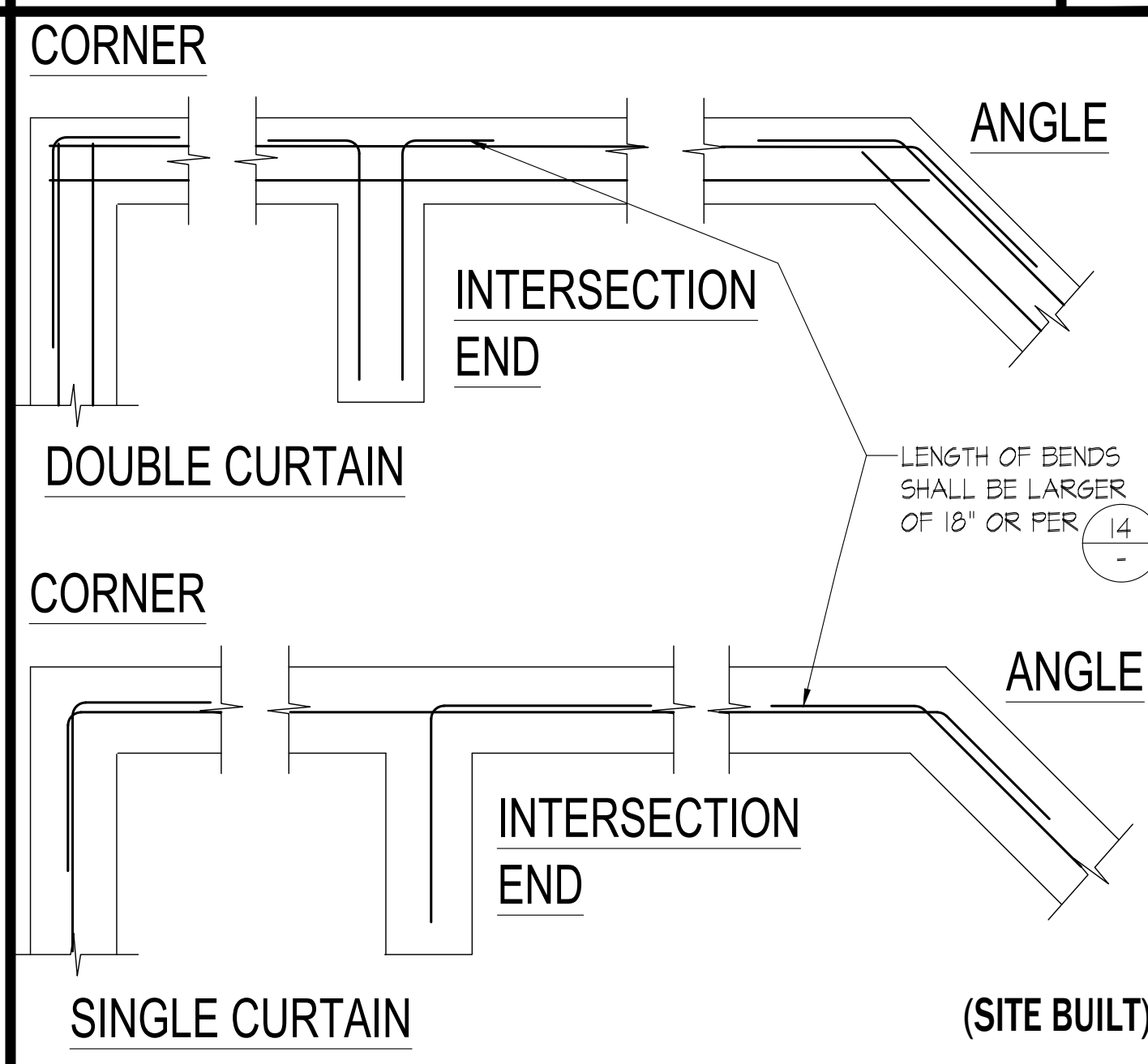
DATE: 05/11/2021 SHEET: OF



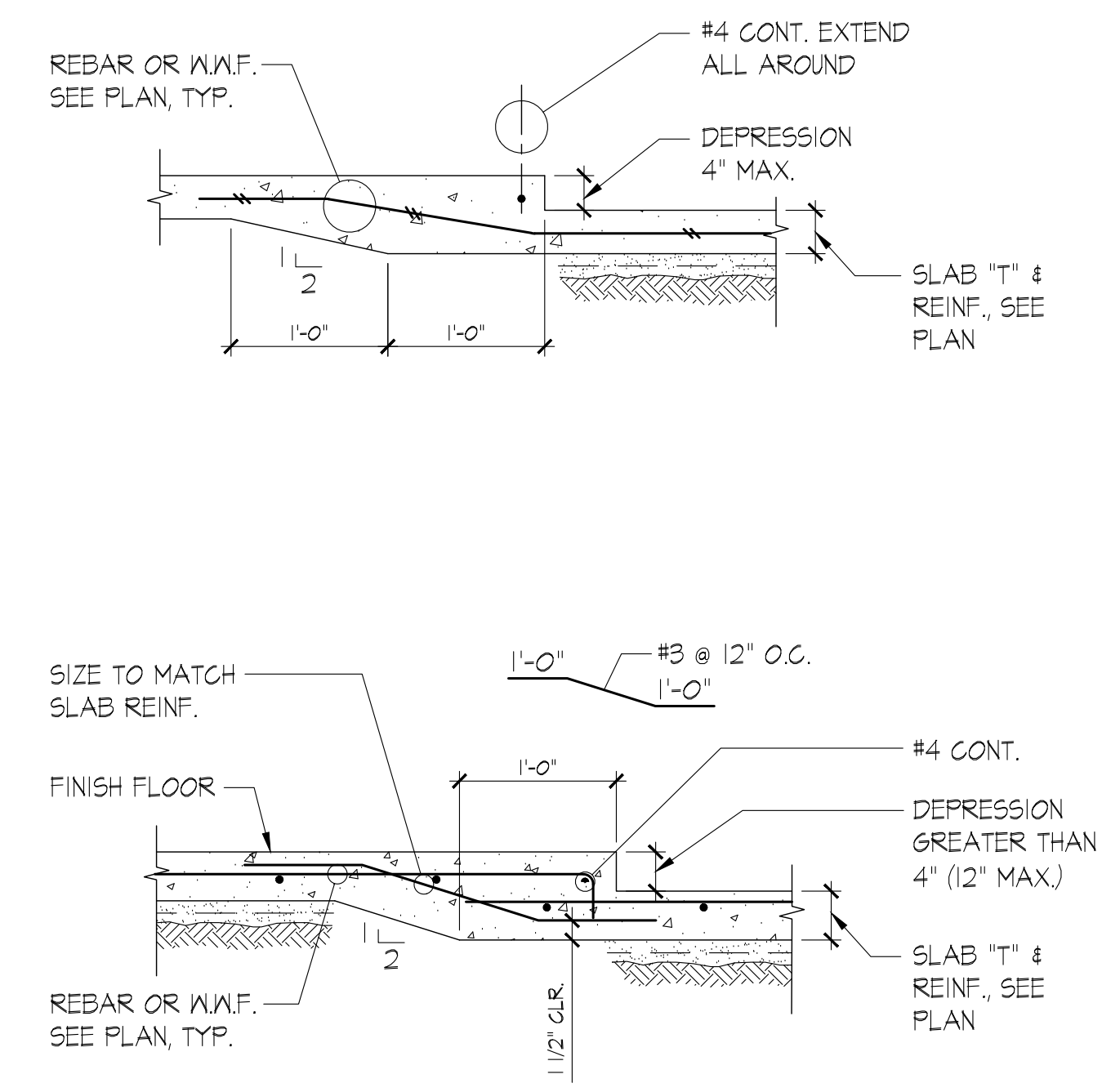
SCALE: 1"=1'-0" 22 TYPICAL TRENCH DRAIN DETAIL NTS 19



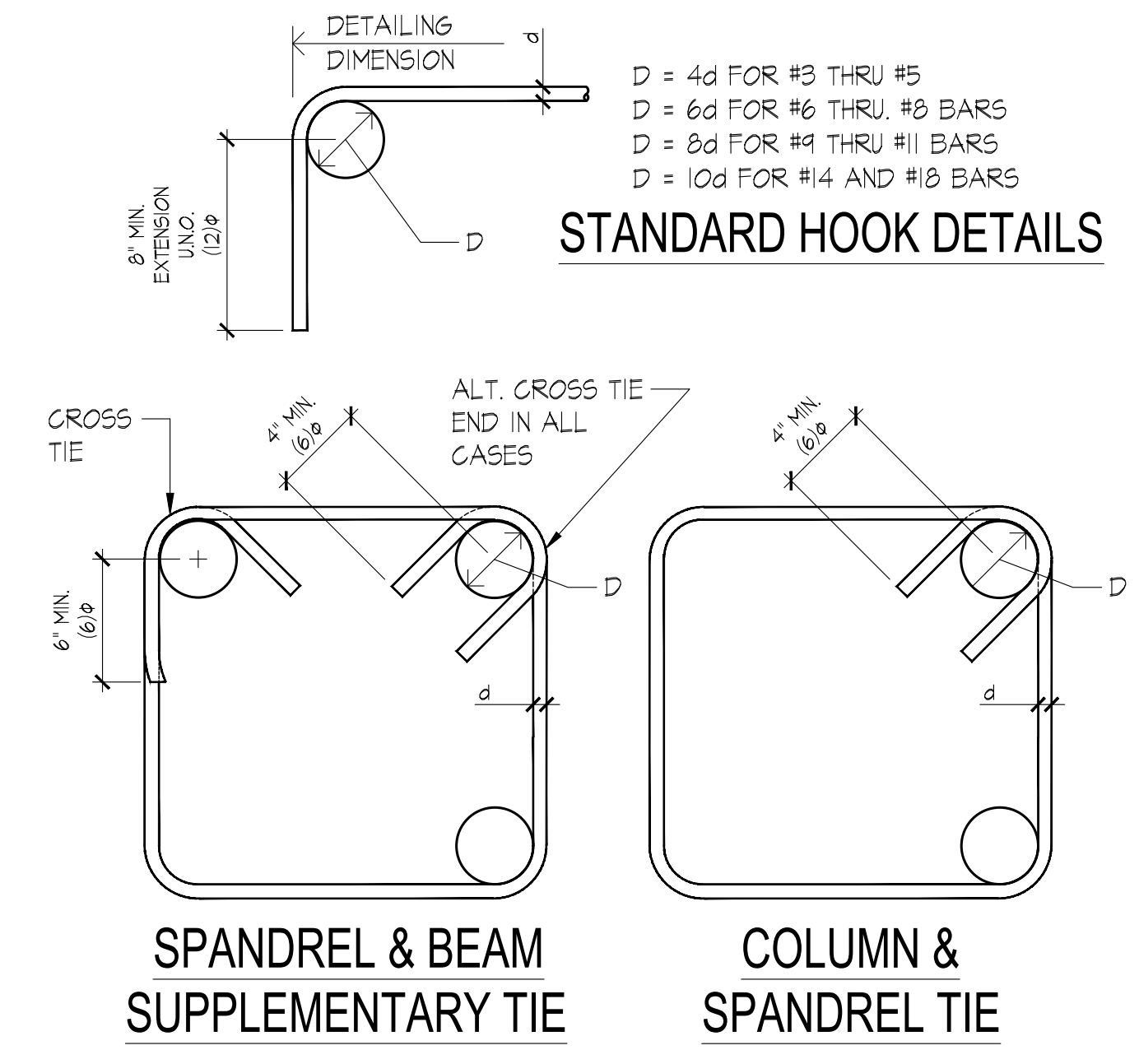
23 STAIR ON GRADE DETAIL NOT TO SCALE



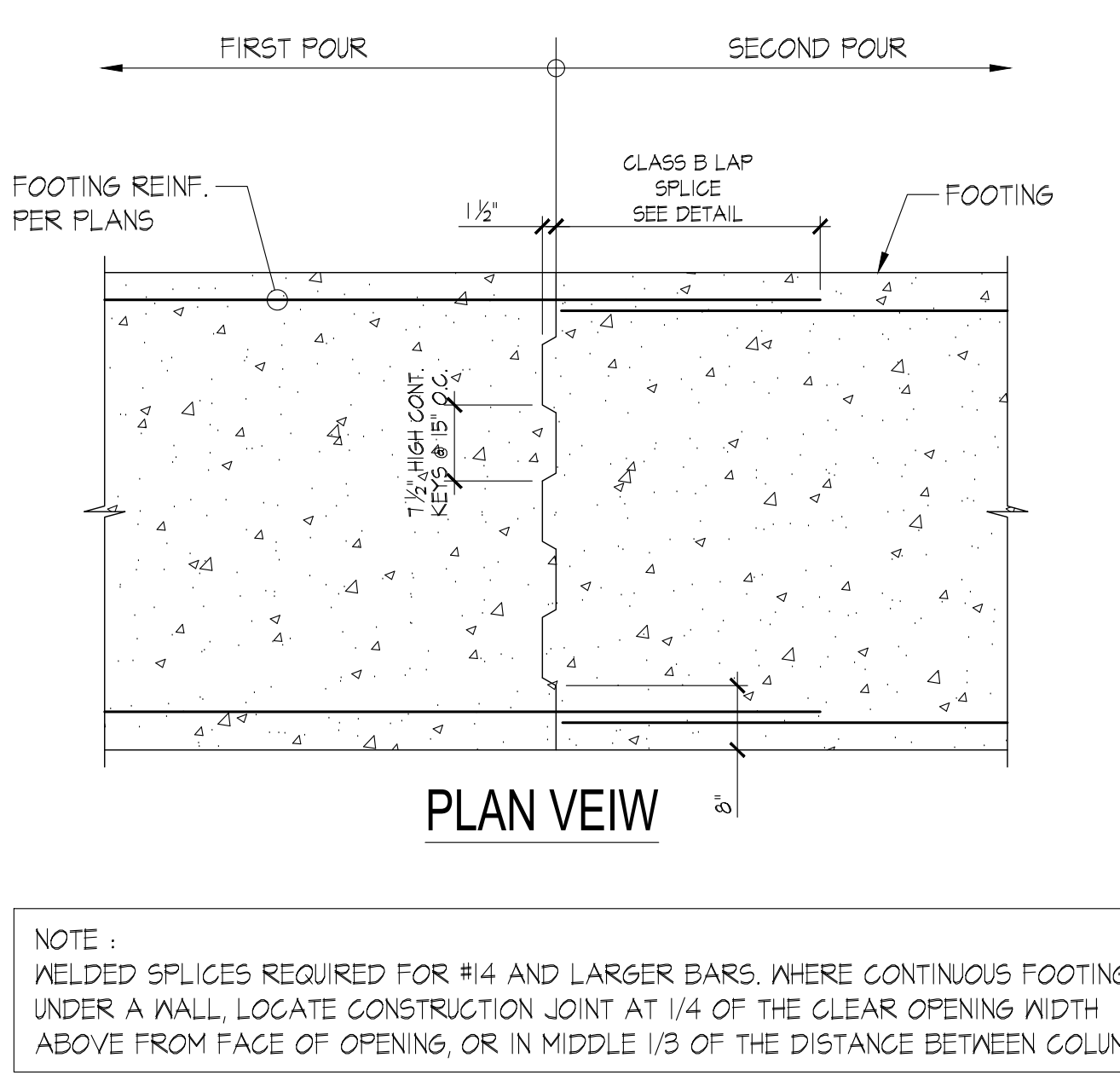
20 REBAR AT INTERSECTION NTS



17 SLAB ON GRADE DEPRESSION NOT TO SCALE



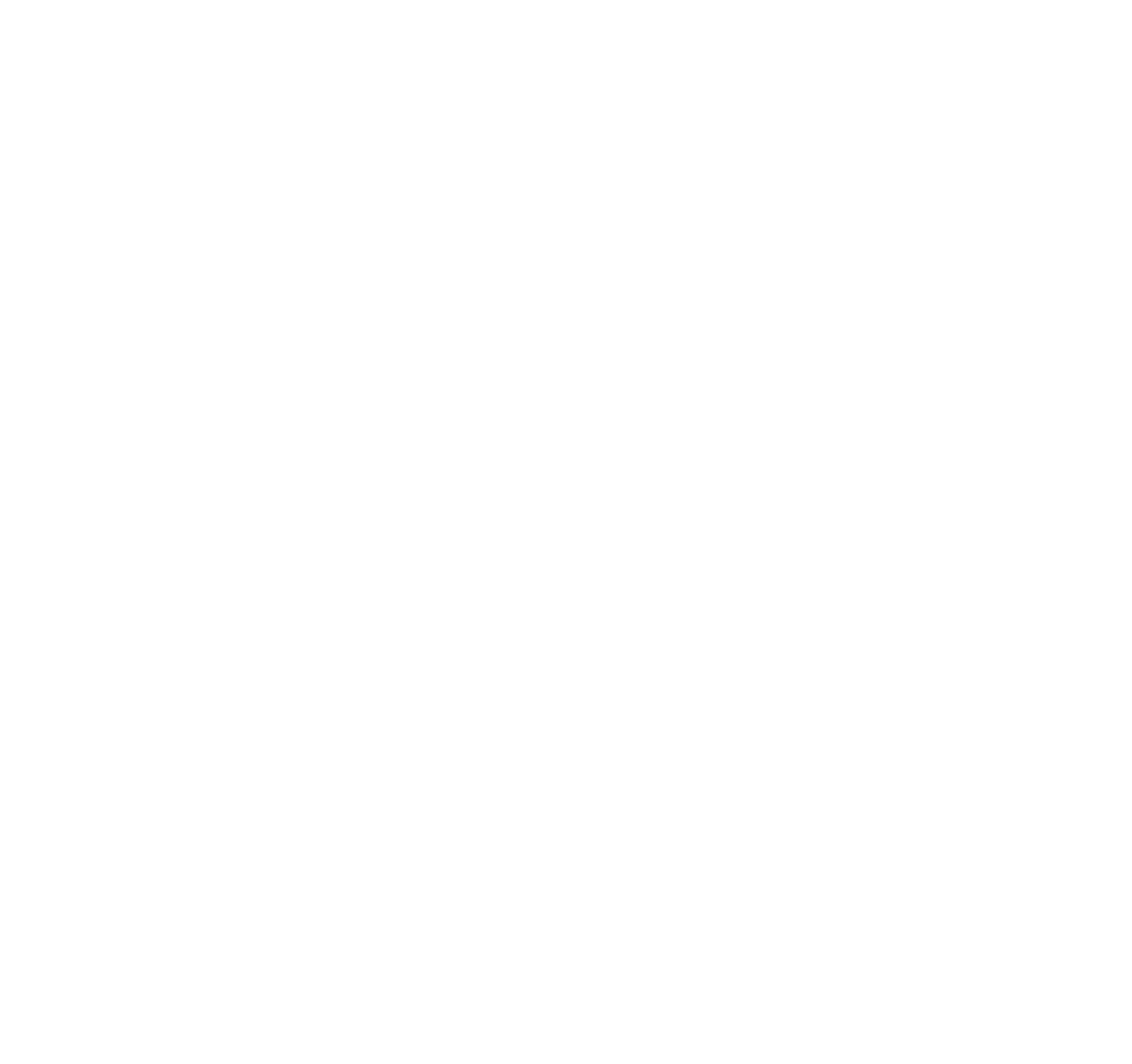
14 TYPICAL STANDARD HOOK DETAIL NOT TO SCALE



24 CONT. FTG. CONSTRUCTION JOINT NOT TO SCALE



21 NOT TO SCALE



18 NOT TO SCALE



15 NTS

PROJECT TITLE
20-MPC-037
**REPAIR BASEBALL
DUGOUTS**

7075 CAMPUS ROAD
MOORPARK, CALIFORNIA 93021

COMMISSIONED ARCHITECT

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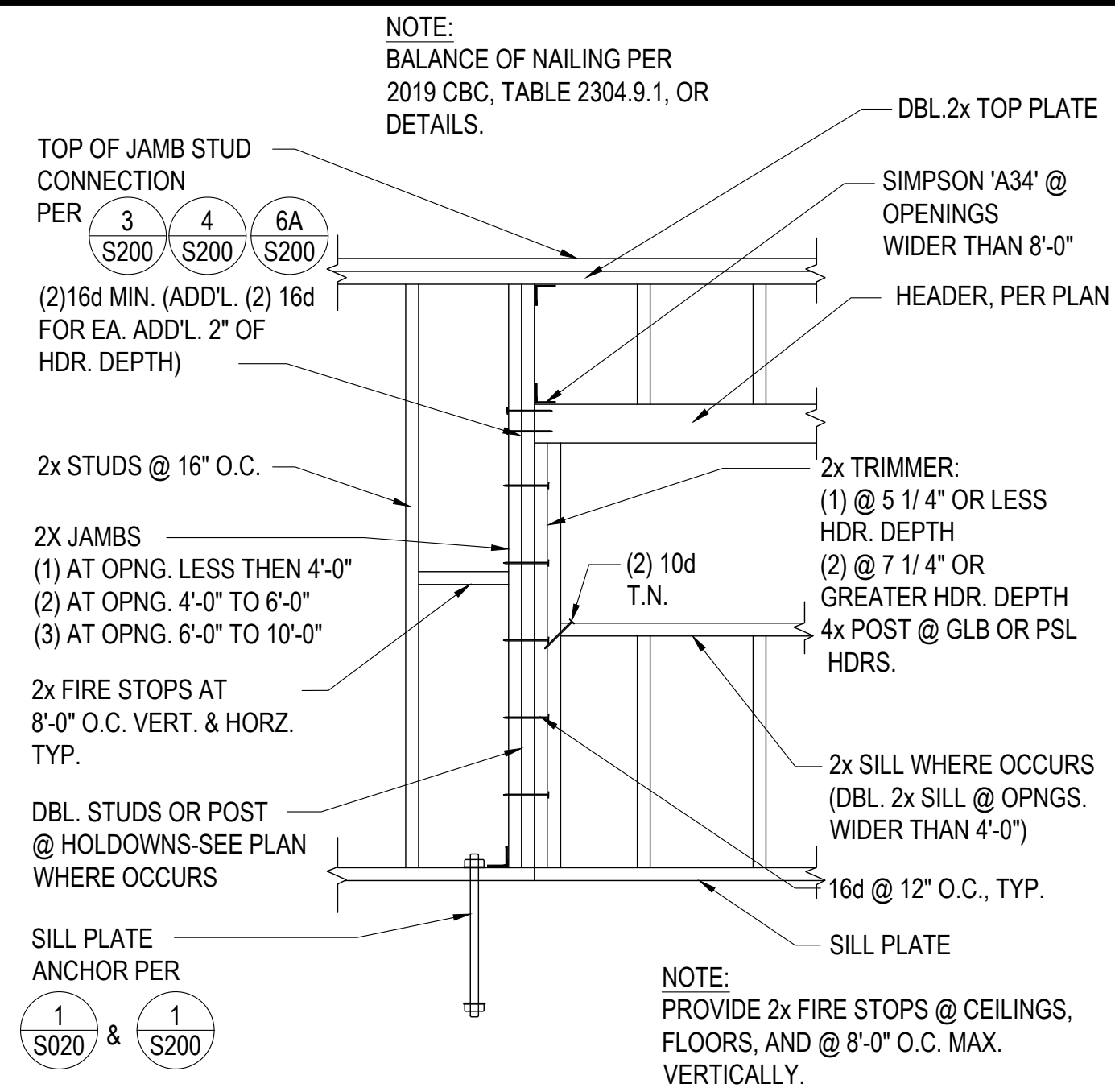
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STAMPS/SEALS

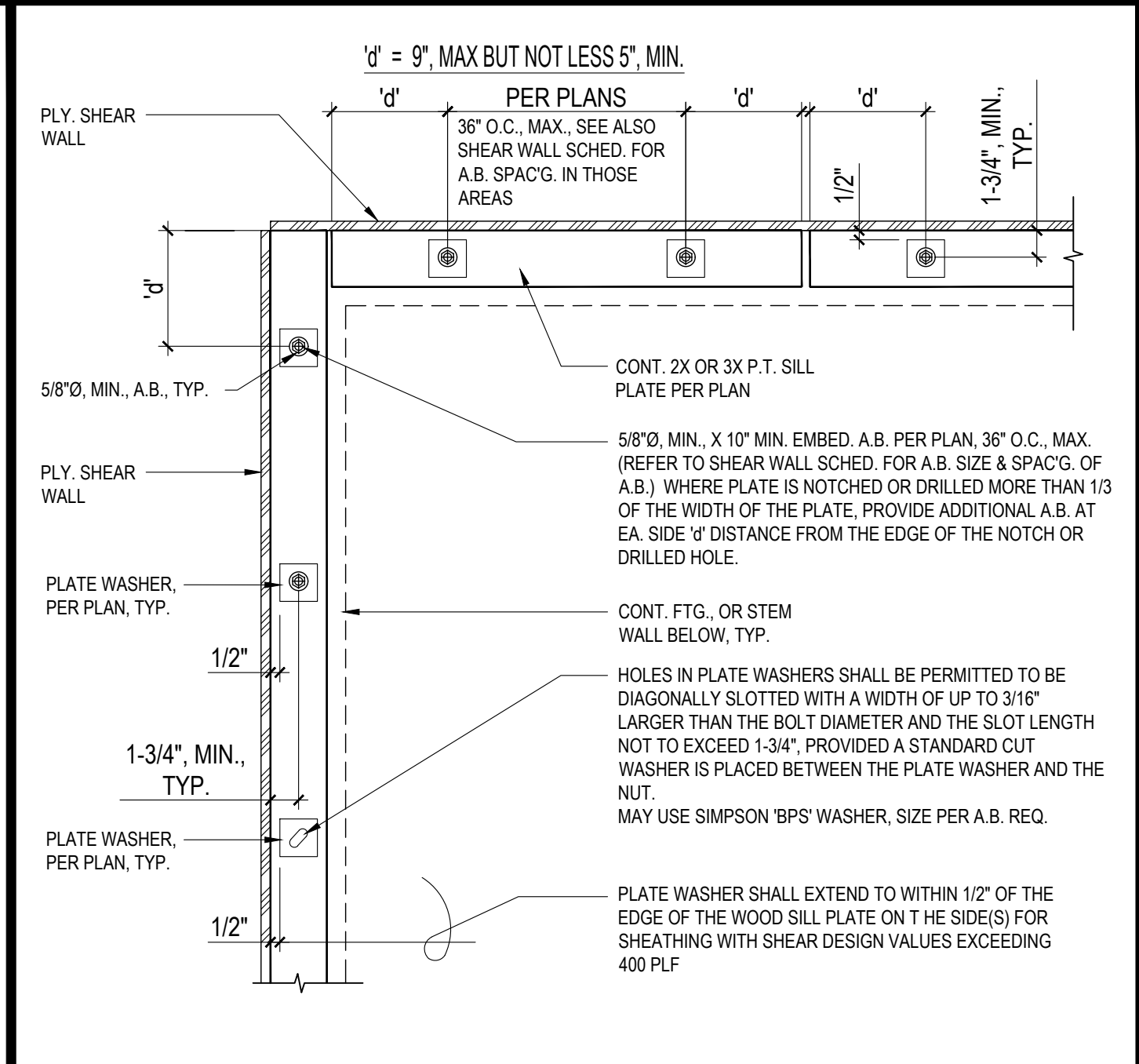
SIGNED DATE: 05/11/2021

SHEET TITLE:
**TYPICAL
DETAILS**

PROJECT NO.:	020-MPC-037	PROJECT ARCHITECT:	Designer
DRAWN:	MH	CHECKED:	WL
SHEET NUMBER:	S011		
DATE:	05/11/2021	SHEET:	___ OF ___



10 TYPICAL WALL FRAMING DETAIL

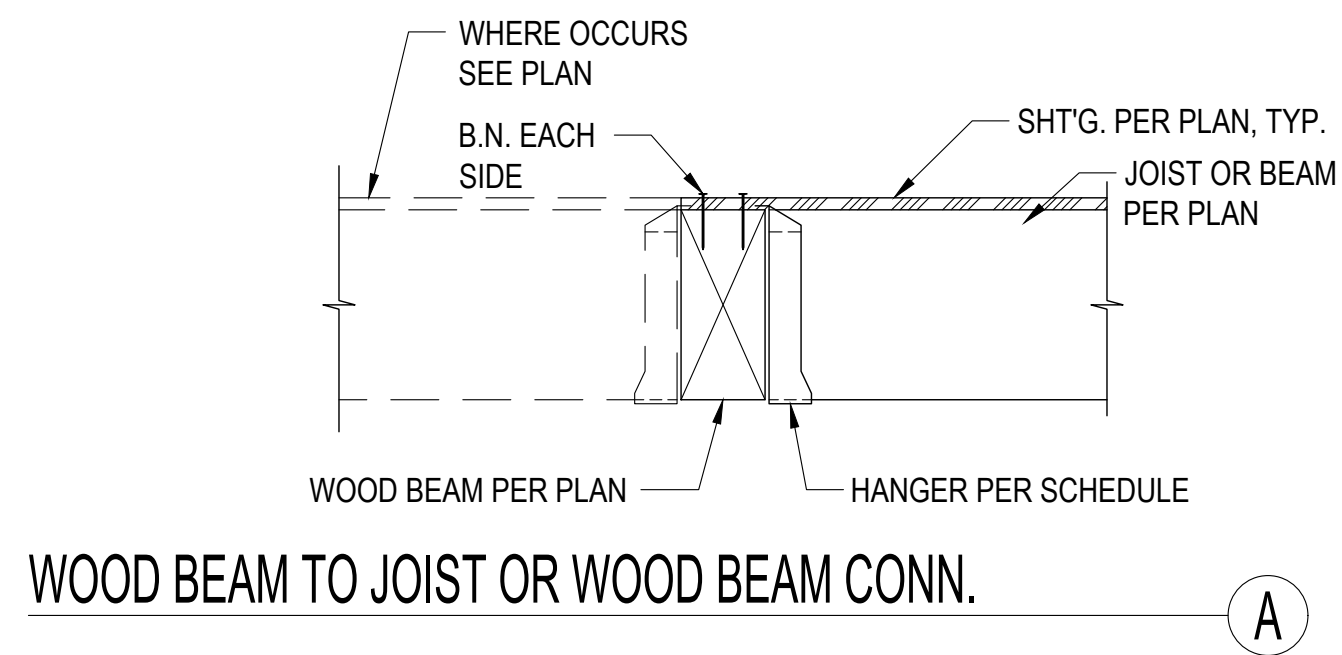


4 TYPICAL ANCHOR BOLTS AT SILL PLATE SCALE: 1"=1'-0" 1

HANGER SCHEDULE - MANUFACTURER: SIMPSON STRONGTIE			
TOP MOUNT HANGER (SEE NOTE 1)		FACE MOUNT HANGER (SEE NOTE 1)	
JOIST/BEAM SIZE	HANGER TYPE	JOIST/BEAM SIZE	HANGER TYPE
ALL SAWN LUMBER U.N.O.	SIMPSON 'HUT'	ALL SAWN LUMBER U.N.O.	SIMPSON 'HU'
2X6 THRU 2X16	SIMPSON 'LB'	2X6 THRU 2X16	SIMPSON 'LUS'
(2) 2X6 THRU (2) 2X14	SIMPSON 'HUST'	(2) 2X6 THRU (2) 2X14	SIMPSON 'LUS'
4X6 THRU 4X14	SIMPSON 'HUST'	4X6 THRU 4X14	SIMPSON 'HHUS'
ALL I-JOIST U.N.O.	SIMPSON 'LBV'	ALL I-JOIST U.N.O.	SIMPSON 'MIU'
SINGLE I-JOIST TO WOOD BEAM 9 1/2 THRU 16 DEEP	SIMPSON 'ITS'	SINGLE I-JOIST TO WOOD BEAM 9 1/2 THRU 16 DEEP	SIMPSON 'IUS'
ALL GLULAM BEAMS U.N.O.	SIMPSON 'EG'	ALL GLULAM BEAMS U.N.O.	SIMPSON 'HHGU'

NOTES:
 1. PROVIDE TOP MOUNT HANGERS. FACE MOUNT HANGERS SHALL BE ALLOWED ONLY WHERE SPECIFICALLY INDICATED ON THE PLANS OR DETAILS
 2. PROVIDE SKEWED, SLOPED HANGERS AS REQ'D.

HANGER SCHEDULE



TYPICAL BEAM / JOIST CONNECTIONS

SCALE: 1"=1'-0" 9

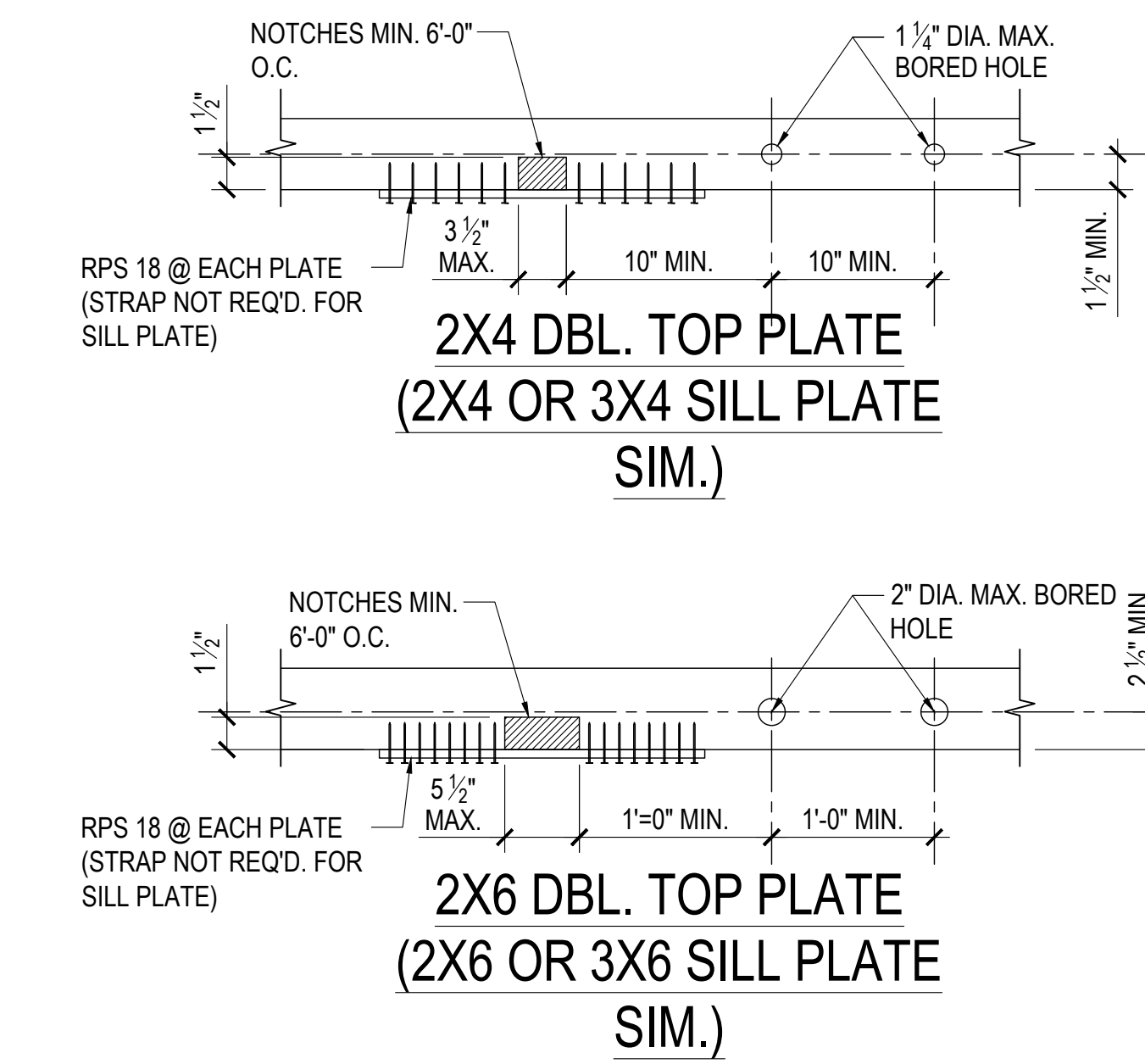
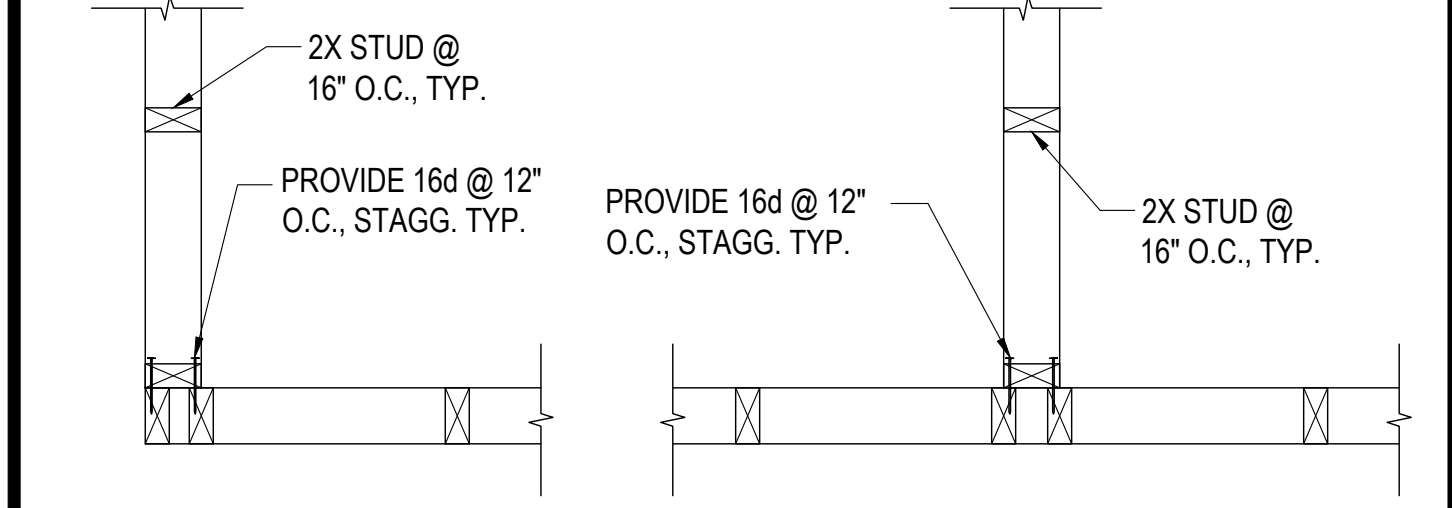


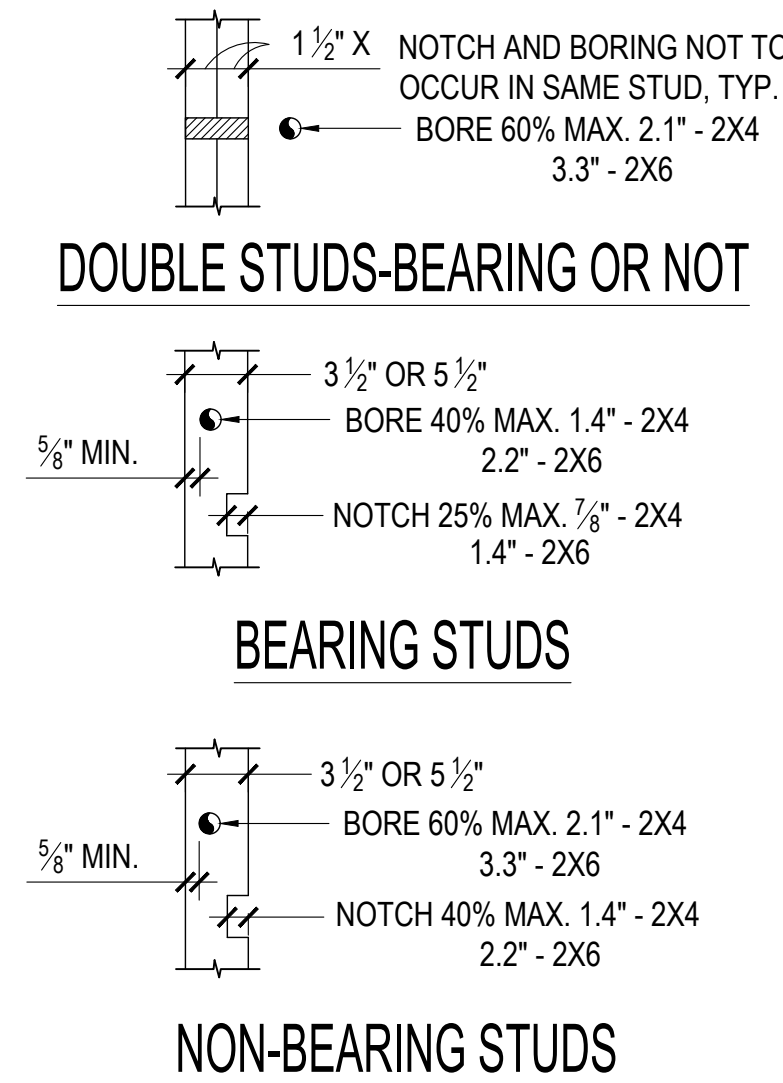
PLATE NOTCHING & BORING

SCALE: 1"=1'-0" 5



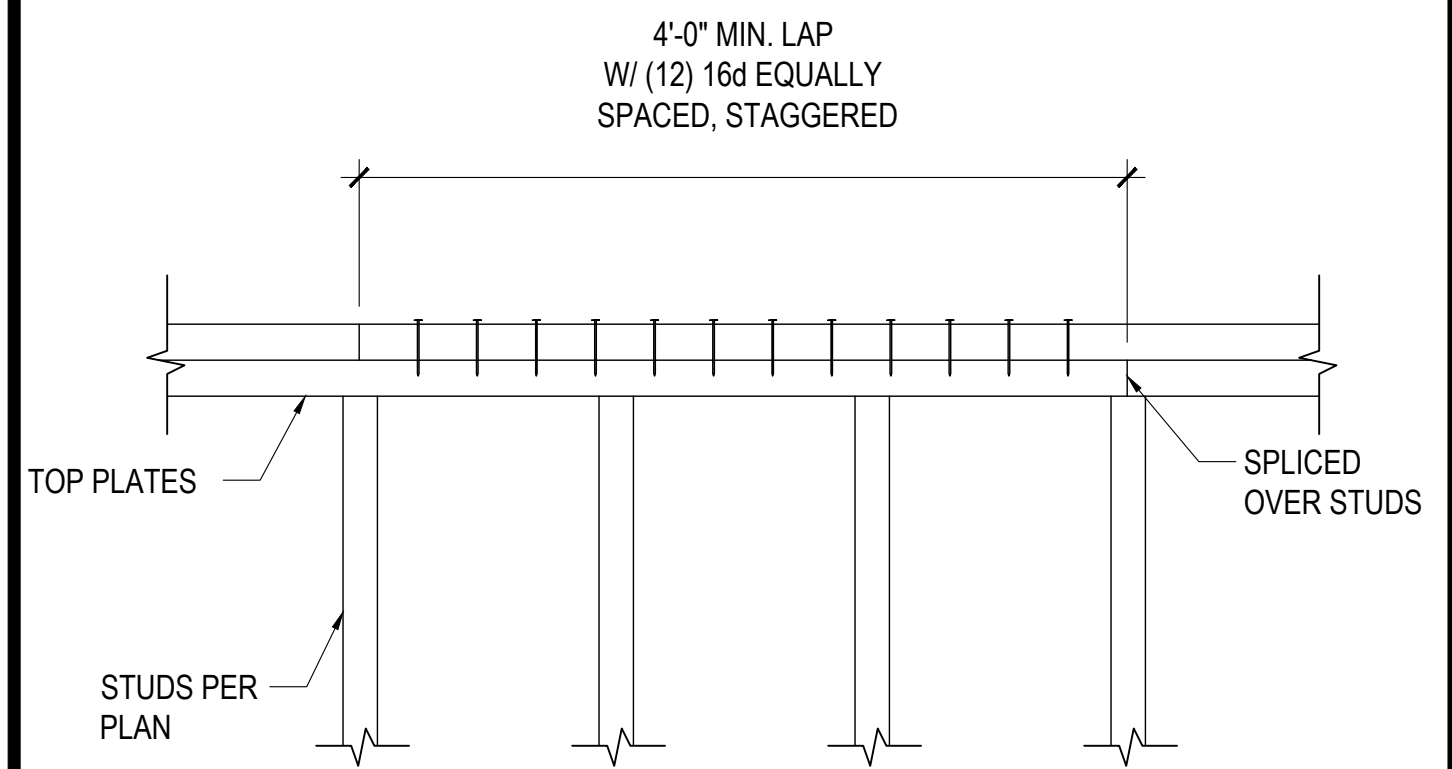
STUD WALL AT CORNERS

SCALE: 1"=1'-0" 2



TYPICAL STUD NOTCHING

NOT TO SCALE 6



TYPICAL TOP PLATES SPLICE DETAIL

SCALE: 1"=1'-0" 3

PROJECT TITLE
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STAMPS/SEALS

 SIGNED DATE: 05/11/2021

SHEET TITLE:
TYPICAL WOOD DETAILS

PROJECT NO.: 020-MPC-037 PROJECT ARCH: Designer
DRAWN: MH CHECKED: WL
SHEET NUMBER:

S020

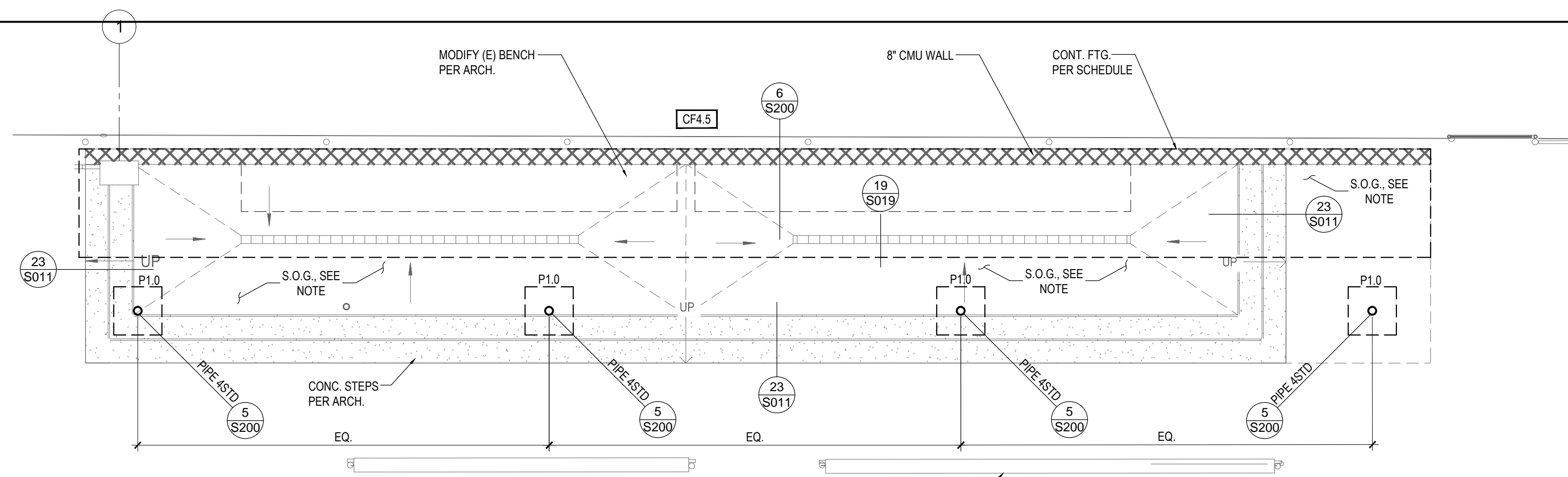
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FRAMING NOTES:

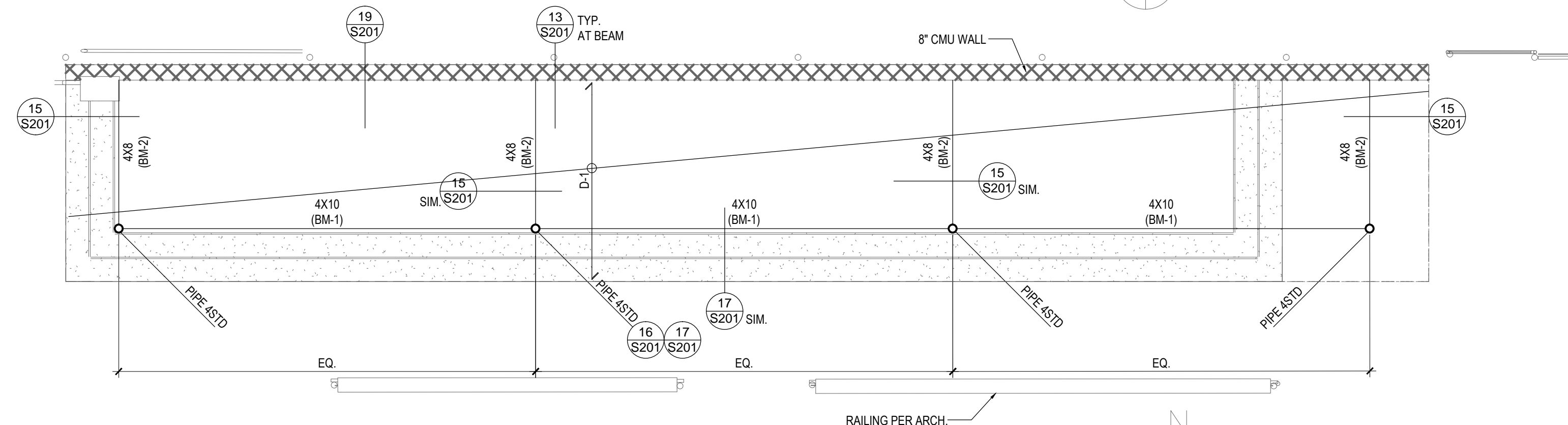
- PROVIDE SINGLE TRIMMER EACH END OF 4X6 OR LESS HEADER OR BEAM & WOOD POST EACH OF 4X8 OR GREATER HEADERS OR BEAM. TYP. UNLESS NOTED OTHERWISE.
- ALL EXTERIOR WALL SHALL BE 2X4 MIN. STUDS @ 16" O.C., UNLESS NOTES OTHERWISE.
- FOR TYP. STUD WALL FRAMING SEE DETAIL 4/S020.
- PROVIDE MULTIPLE STUDS UNDER MULTIPLE JOISTS, RAFTERS OR TRUSSES.
- PROVIDE BOUNDARY NAILING ALONG ALL DRAG MEMBERS, UNLESS NOTED OTHERWISE.
- SEE ARCHITECTURAL PLAN FOR ALL DIMENSIONS AND CONDITIONS NOT SHOWN.
- FASTENERS FOR PRESERVATIVE TREATED AND FIRE-RESISTANT-TREATED WOOD SHALL BE OF HOT DIPPED ZINC-COATED GALVANIZED STL., STAINLESS STL., SILICON BRONZE OR COPPER. THE COATING WEIGHTS FOR ZINC-COATED FASTENERS SHALL BE IN ACCORDANCE WITH ASTM A 153. FASTENERS OTHER THAN NAILS, TIMBER RIVETS WOOD SCREWS AND LAG SCREWS SHALL BE PERMITTED TO BE OF MECHANICALLY DEPOSITED ZINC COATED STL. WITH COATING WEIGHTS IN ACCORDANCE WITH ASTM B 695, CLASS 55 MIN.

FRAMING LEGEND

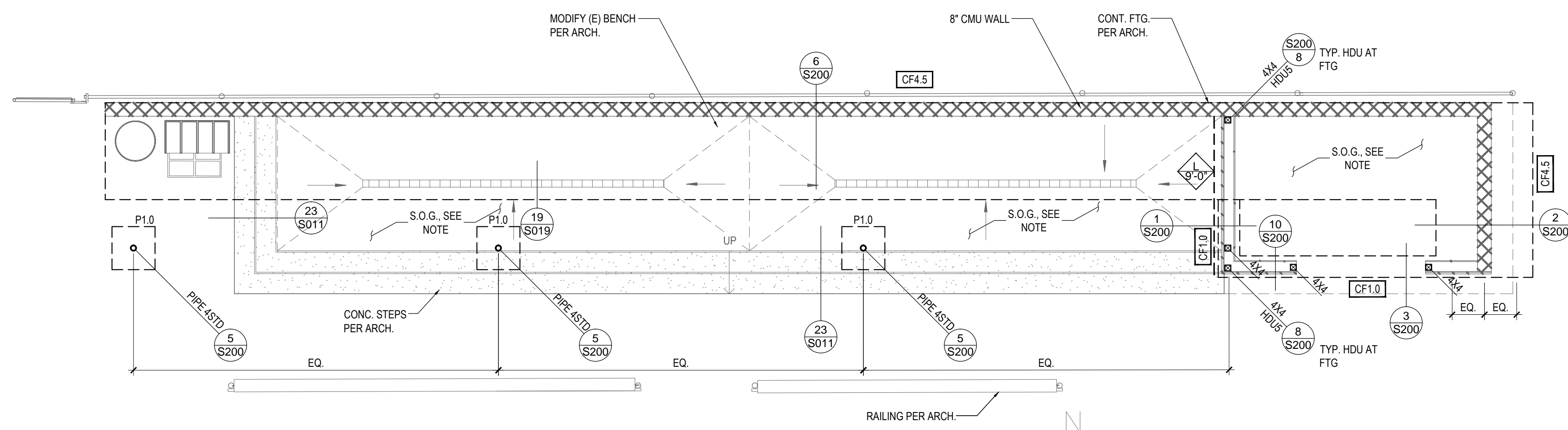
- INDICATES NEW 2 X 4 MINIMUM STUDS @ 16" O.C.
- HDR. INDICATES HEADER PER 4/S202
- SHEAR WALL NOTATION PER SHEAR WALL SCHEDULE AT SHEET S001
- INDICATES COLUMN
- D-1 INDICATES 20 GA "N" DECK (3" DEEP) W/ MANUFACTURER'S CRIMPED SIDE SEAM CONNECTION @ 12" O.C. ATTACH TO WOOD BEAM/NAILER W/ SDS SCREW AND EPDM WASHER (ASC DGN-32 IAPMO #161, VERCO PLN-24 OR N-24 IAPMO #217)



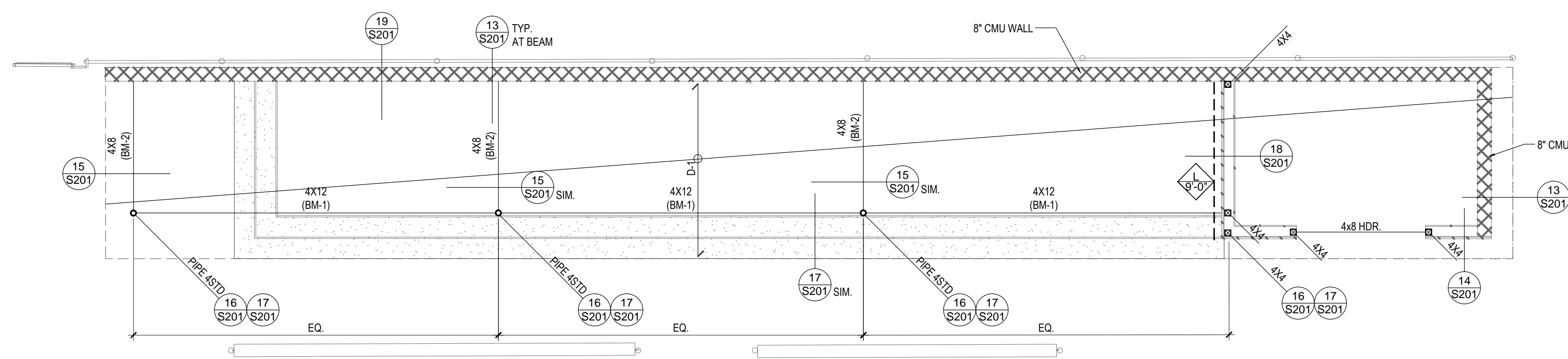
1A 1ST BASE DUGOUT FOUNDATION PLAN
SCALE: 1/4" = 1'-0"



1B 1ST BASE DUGOUT ROOF FRAMING PLAN
SCALE: 1/4" = 1'-0"



2A 3RD BASE DUGOUT FOUNDATION PLAN
SCALE: 1/4" = 1'-0"



2B 3RD BASE DUGOUT ROOF FRAMING PLAN
SCALE: 1/4" = 1'-0"

FOUNDATION NOTES:

- SEE FOUNDATION NOTES ON SHEET S000 FOR DESIGN SOIL BEARING PRESSURE, ETC.
- SEE ARCHITECTURAL PLANS FOR DIMENSIONS AND CONDITIONS NOT SHOWN.
- ALL HOLDDOWNS AND ANCHOR BOLTS AT SHALL BE SET IN PLACE BY TEMPLATE PRIOR TO FOUNDATION INSPECTION.
- WOOD PATIO COVER COLUMN BASES SHALL BE A MINIMUM OF 1 INCH STAND-OFF OF FINISH SURFACE FOR MOISTURE PROTECTION.
- ALL WORK SHALL CONFORM TO THE 2019 CALIFORNIA BUILDING CODE (2019 CBC).
- ALL HOLDDOWN BOLTS SHALL BE RE-TIGHTENED JUST PRIOR TO COVERING THE WALL FRAMING.
- ALL CONNECTORS AND / OR FASTENERS IN CONTACT WITH PRESSURE TREATED LUMBER SHALL BE HOT DIPPED GALVANIZED (ASTM 123 & 153) G85 MINIMUM AND FOR MECHANICAL GALVANIZING (ASTM B695)

CONTINUOUS FOOTING SCHEDULE

MARK	WIDTH	FTG. THK.	DEPTH	REINFORCEMENT	TIES
CF-1.0	12"	12"	21" MIN.	(2) #5 BARS @ TOP AND BOTTOM	NONE
CF-4.5	54"	15"	21" MIN.	(5) #5 BARS @ TOP AND BOTTOM	NONE

- NOTE:
- ALL FTGS. TO BE MIN. OF 2" BELOW LOWEST ADJACENT GRADE
 - ALL CONC. TO HAVE $F_c=3,000$ PSI, MIN.

PIER FOOTING TABLE

MARK	DEPTH (D)	DIAMETER (D)	VERT. REINF. BARS
P-1	4'-6"	24"	(6) #5

SILL PLATE NOTE: ALL FASTENERS IN CONTACT W/ PRESSURE TREATED SILL SHALL BE GALVANIZED (ANCHOR BOLTS, NAILS, ETC.)

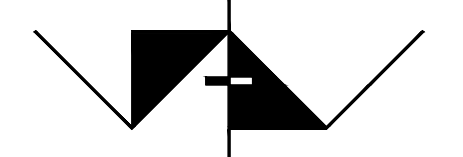
TYPICAL CONCRETE SLAB ON GRADE SHALL BE 4" THICK WITH #4 @ 16" O.C. EA. MAY MID HT. OVER 2" SAND OVER AN 10 MIL. VISQUEEN VAPOR BARRIER OVER 4" OF SAND, OVER COMPACTED GRADE.

PROJECT TITLE
20-MPC-037

REPAIR BASEBALL DUGOUTS

7075 CAMPUS ROAD
MOORPARK, CALIFORNIA 93021

COMMISSIONED ARCHITECT



AMADOR WHITTLE ARCHITECTS, INC.

28328 AGOURA ROAD, SUITE 203
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STAMPS/SEALS



SIGNED DATE: 05/11/2021

SHEET TITLE:

STRUCTURAL PLANS

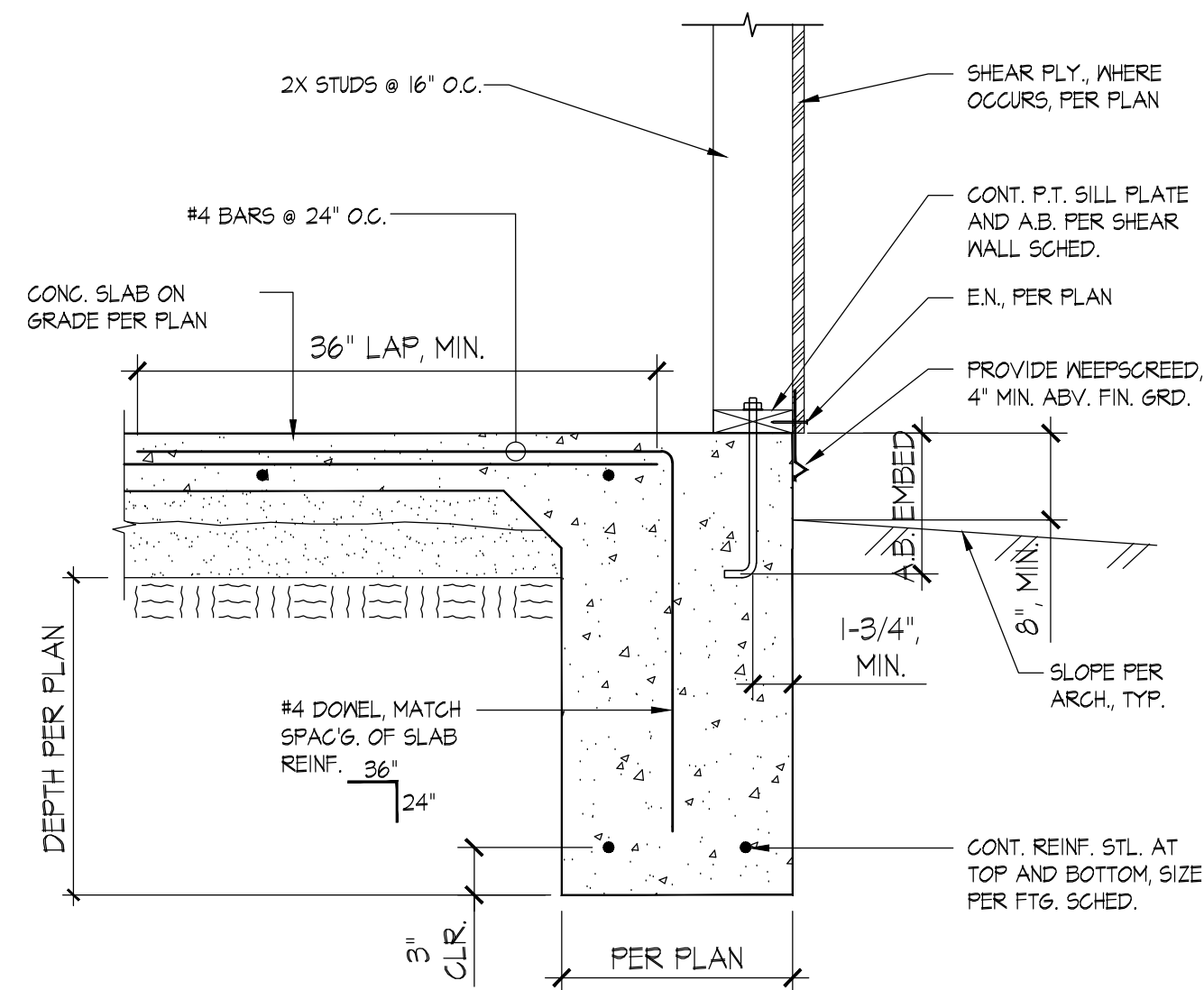
PROJECT NO.: 020-MPC-037 PROJECT ARCH: Designer

DRAWN: MH CHECKED: WL

SHEET NUMBER:

S100

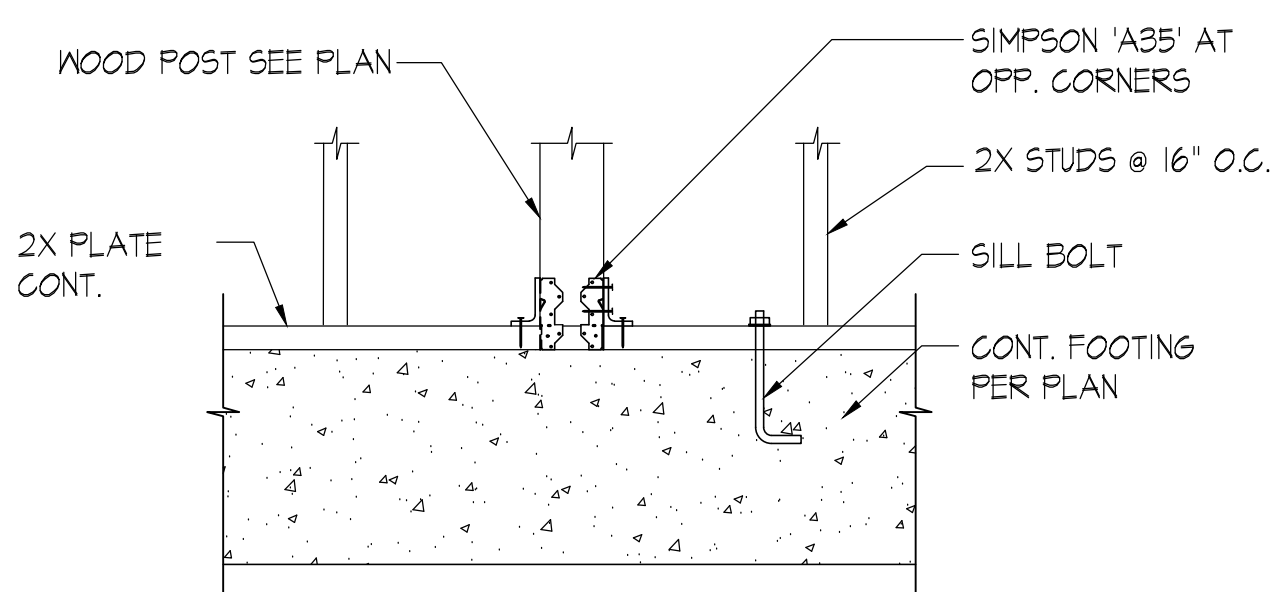
DATE: 05/11/2021 SHEET: OF



SECTION AT EXTERIOR FOOTING

SCALE: 1"=1'-0"

10



POST BASE SECTION (PART OF WALL)

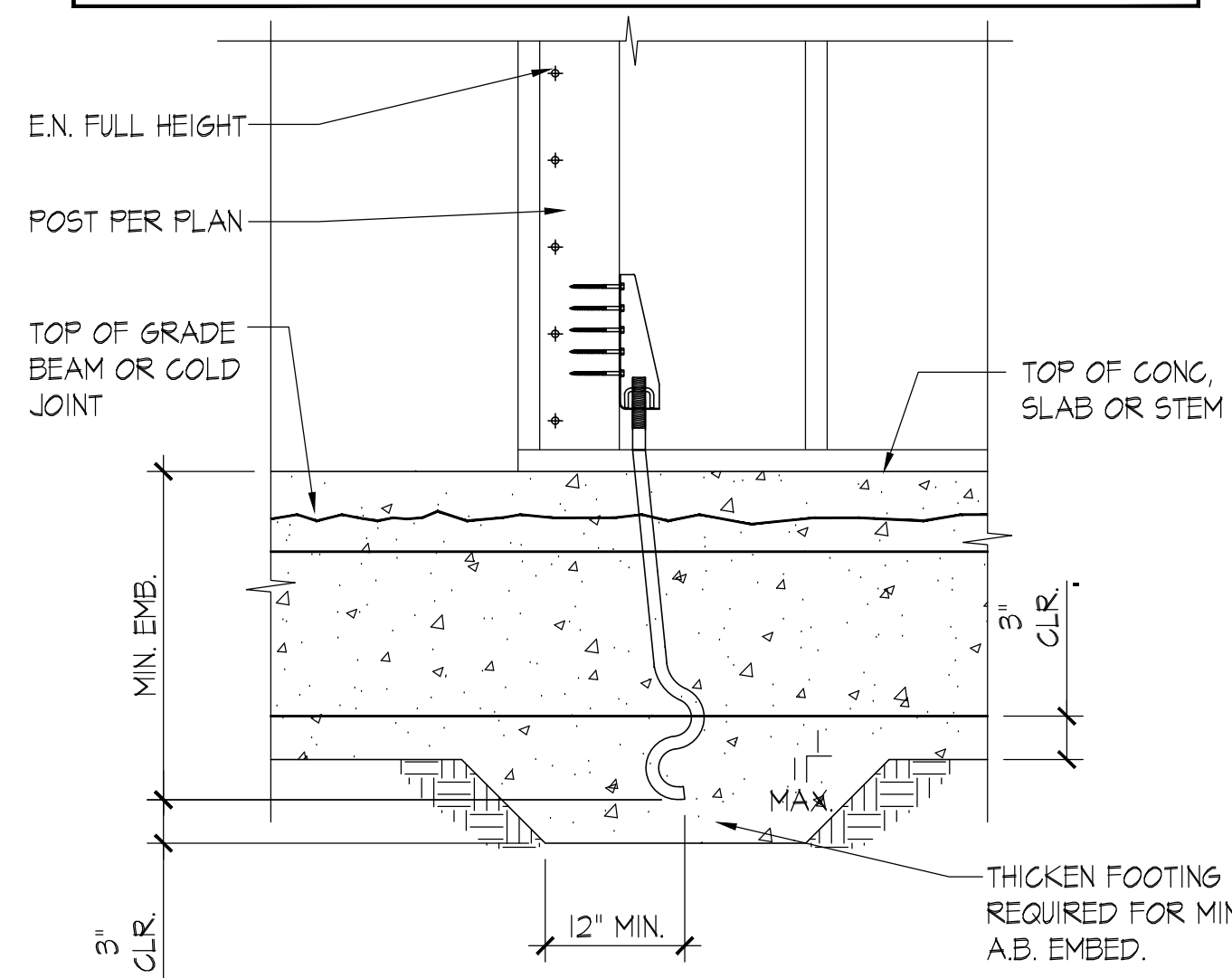
SCALE: 1"=1'-0"

11

HOLDOWN SCHEDULE - 'SSTB' ICC ES ESR 2611

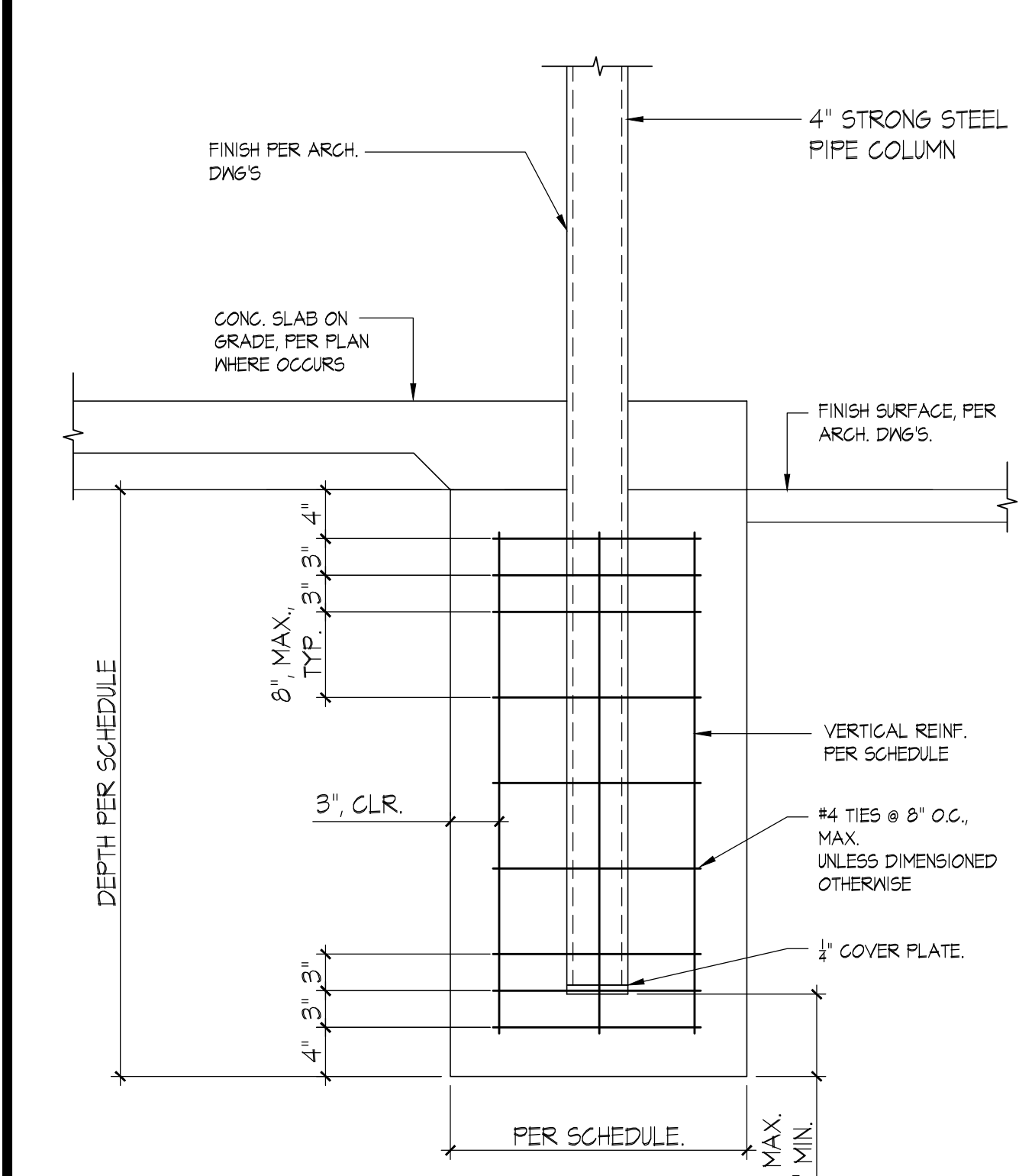
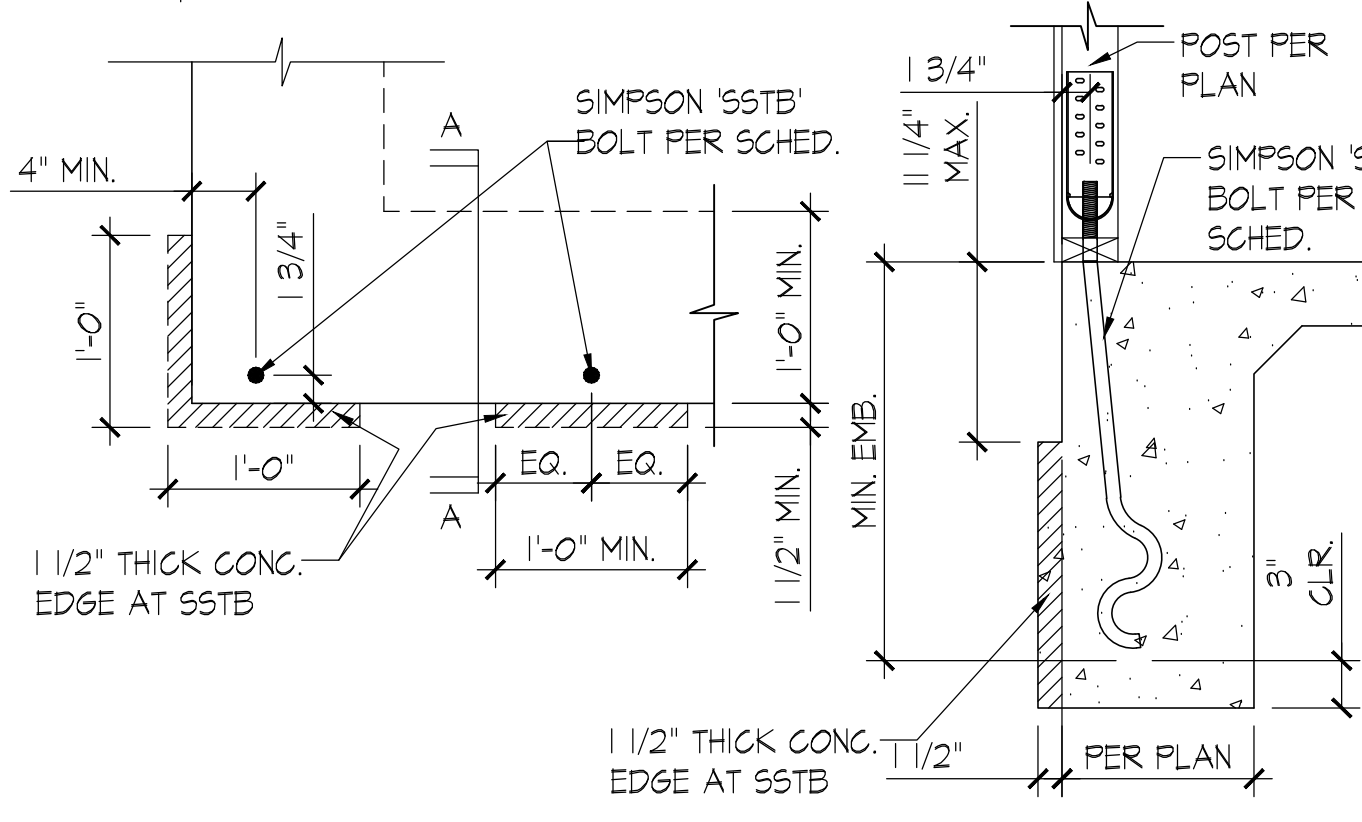
HOLDOWN TYPE	SDS 1/2 X 3 SCREWS	FOUNDATION ANCHOR (ICC 2611)	FOUNDATION ANCHOR MIN. EMBEDMENT	MAX. ALLOW. TENSION LOAD 'ASD' 100% VALUES
HDU2	(6)	S5TB24	20 5/8"	HDU2 = 3075 LB
HDU4	(10)	S5TB24	20 5/8"	S5TB = 3325 LB
HDU5	(14)	S5TB24	20 5/8"	S5TB = 3325 LB
HDU8	(20)	S5TB28	24 7/8"	S5TB = 6395 LB

NOTE:
1. INSTALL PER MANUF. RECOMMENDATIONS.
2. USE S5TBL AT 9X SILL PLATES.

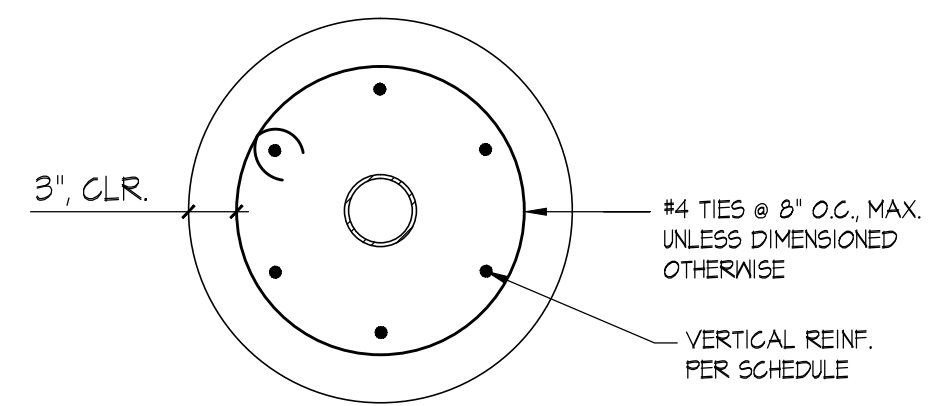


PLAN VIEW

SECTION A-A



SECTION VIEW

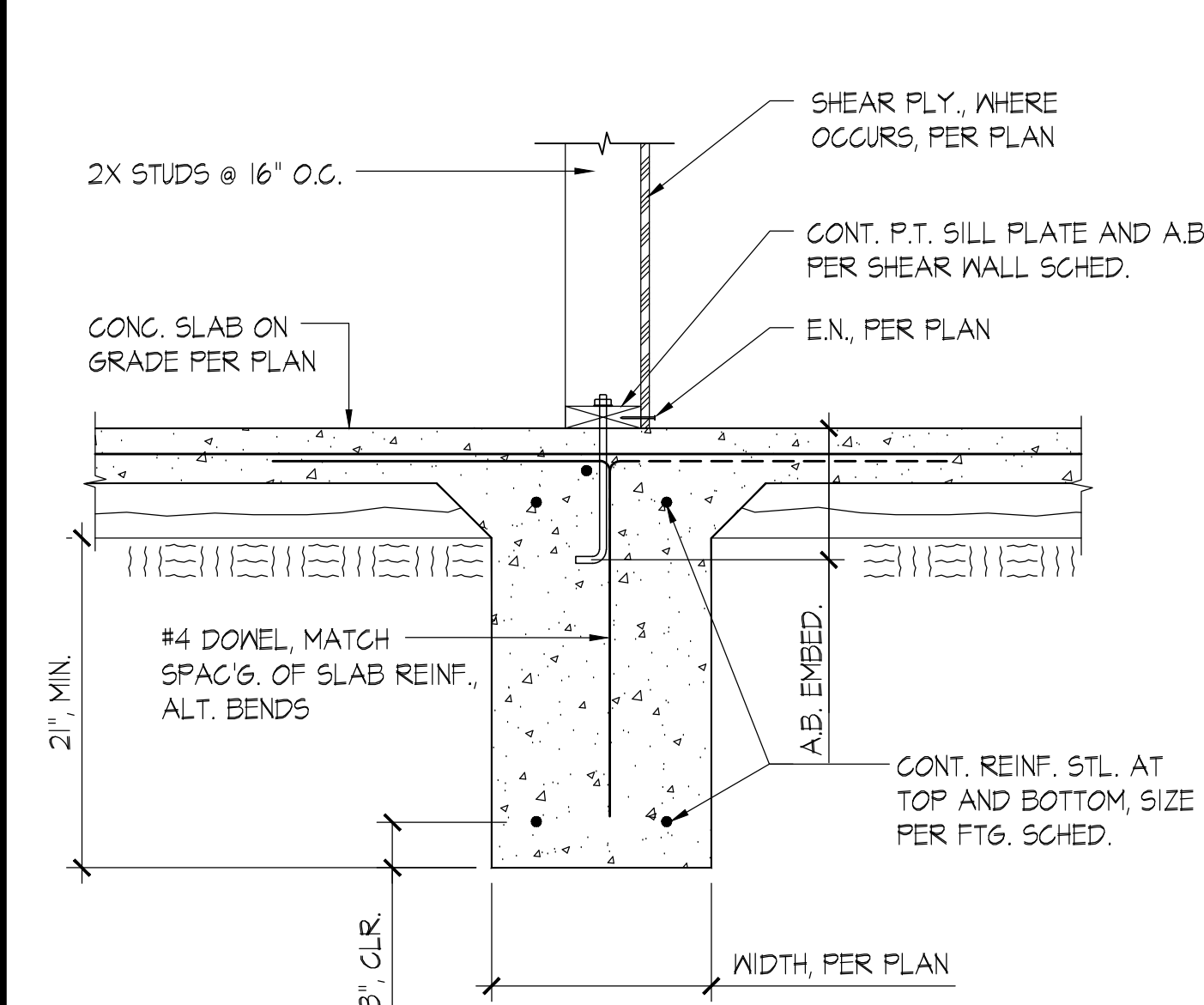


PLAN VIEW

STEEL POST POLE FOOTING

SCALE: 1"=1'-0"

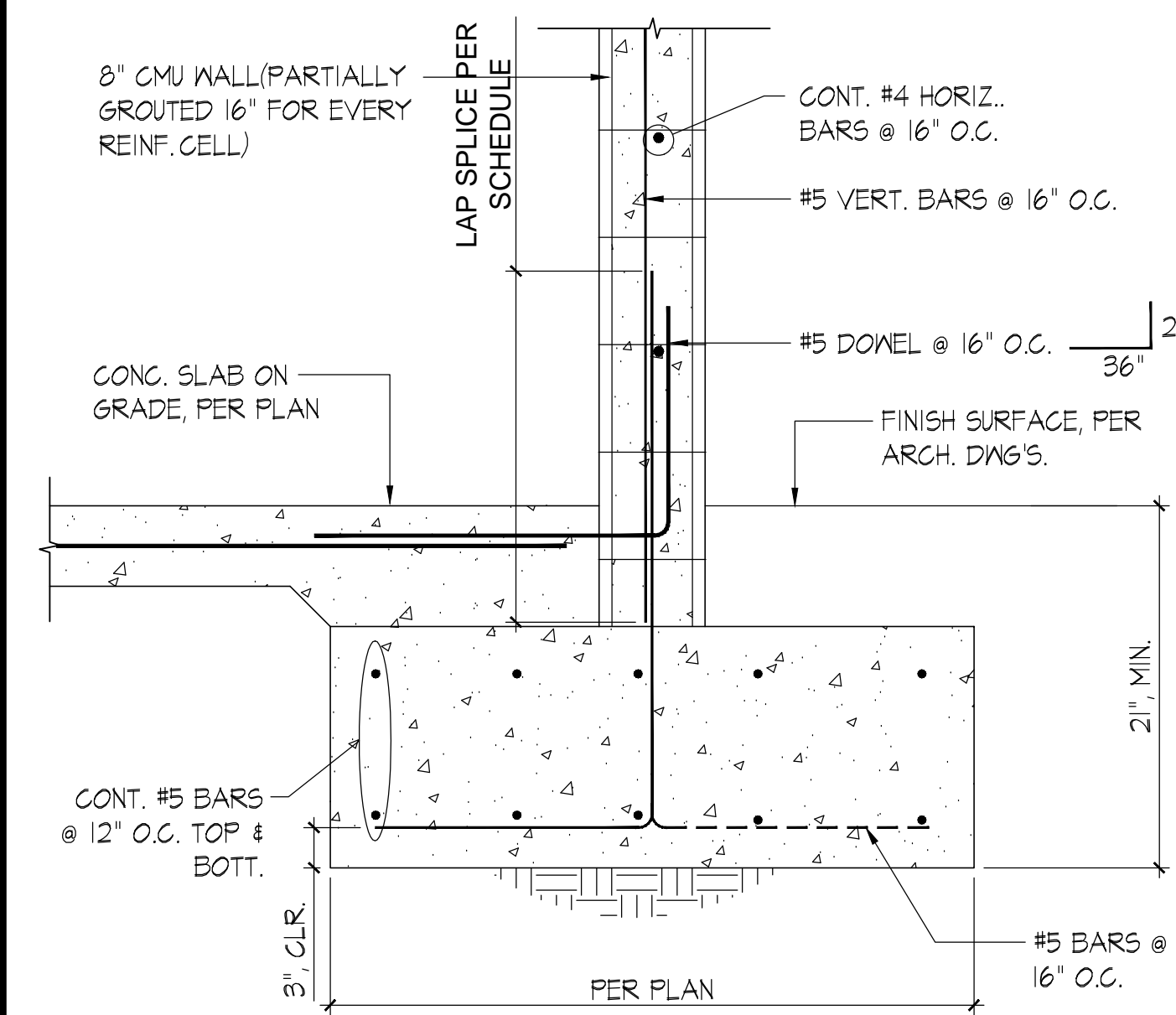
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SHEAR TRANSFER AT INTERIOR FOOTING

SCALE: 1"=1'-0"

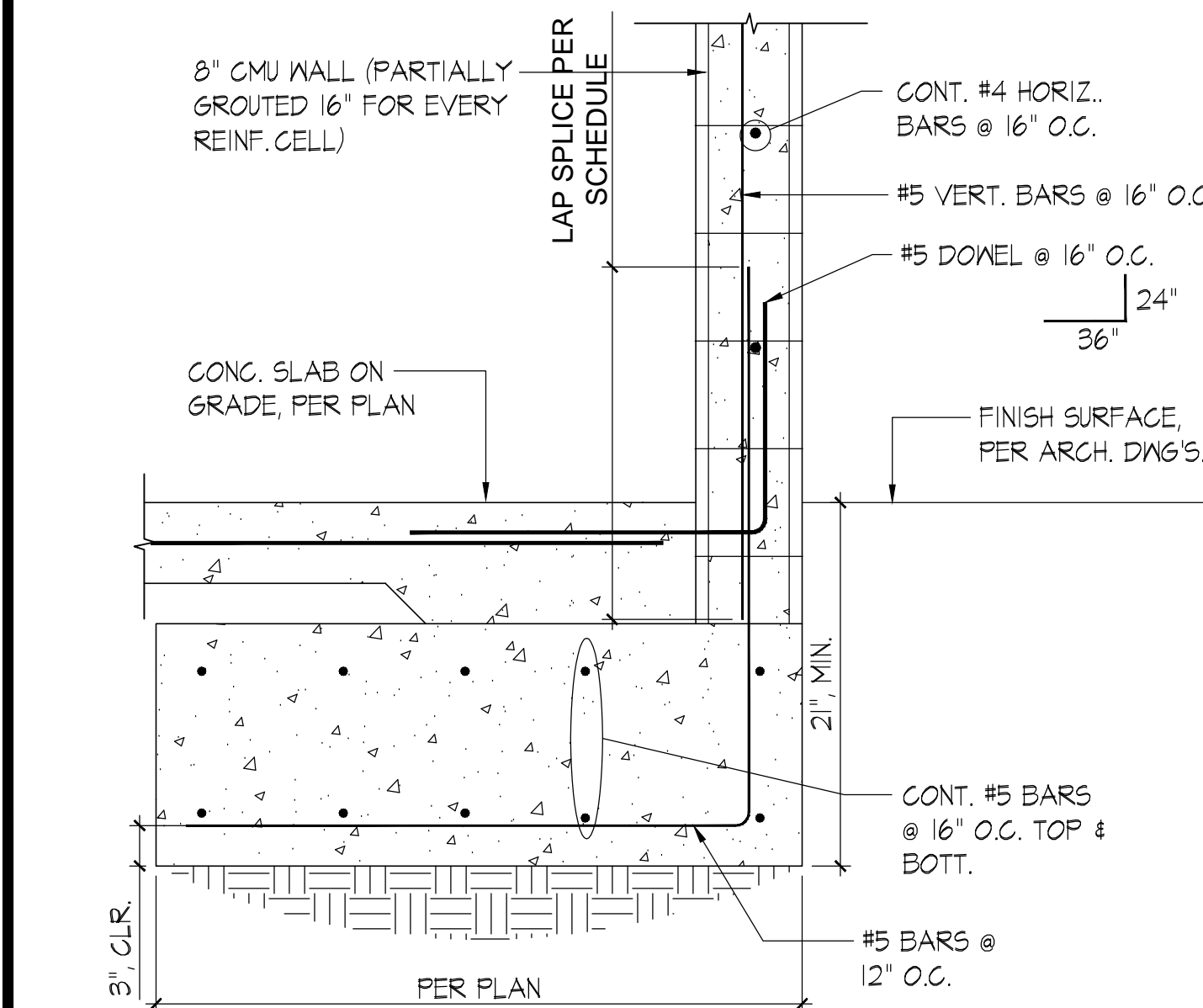
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FTG SECTION AT MASONRY WALL

SCALE: 1"=1'-0"

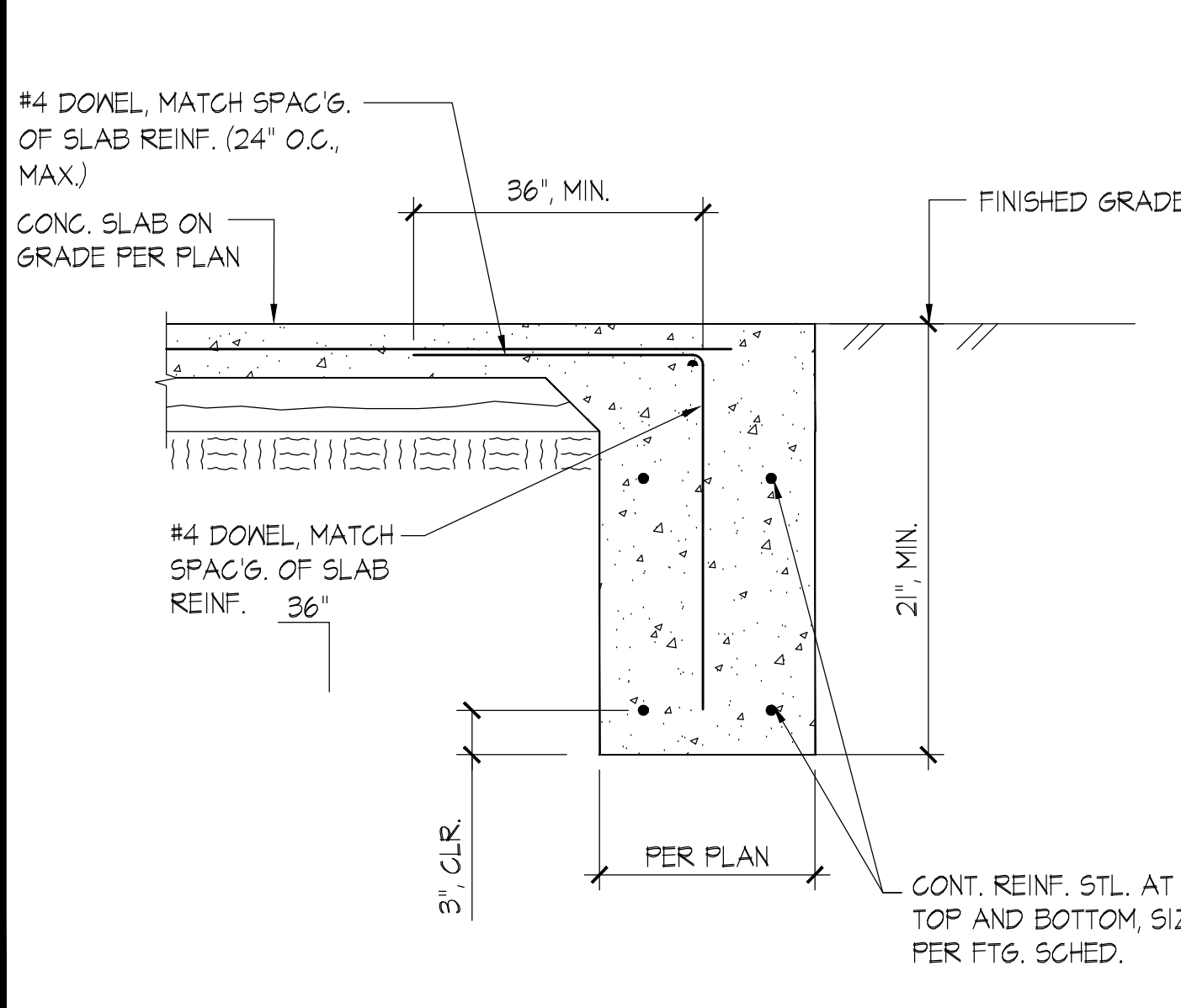
2



FTG SECTION AT MASONRY WALL

SCALE: 1"=1'-0"

6



TIE FOOTING

SCALE: 1"=1'-0"

2



SCALE: 1"=1'-0"

12

HOLDOWN AT NEW FOOTING

SCALE: 1"=1'-0"

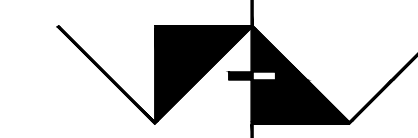
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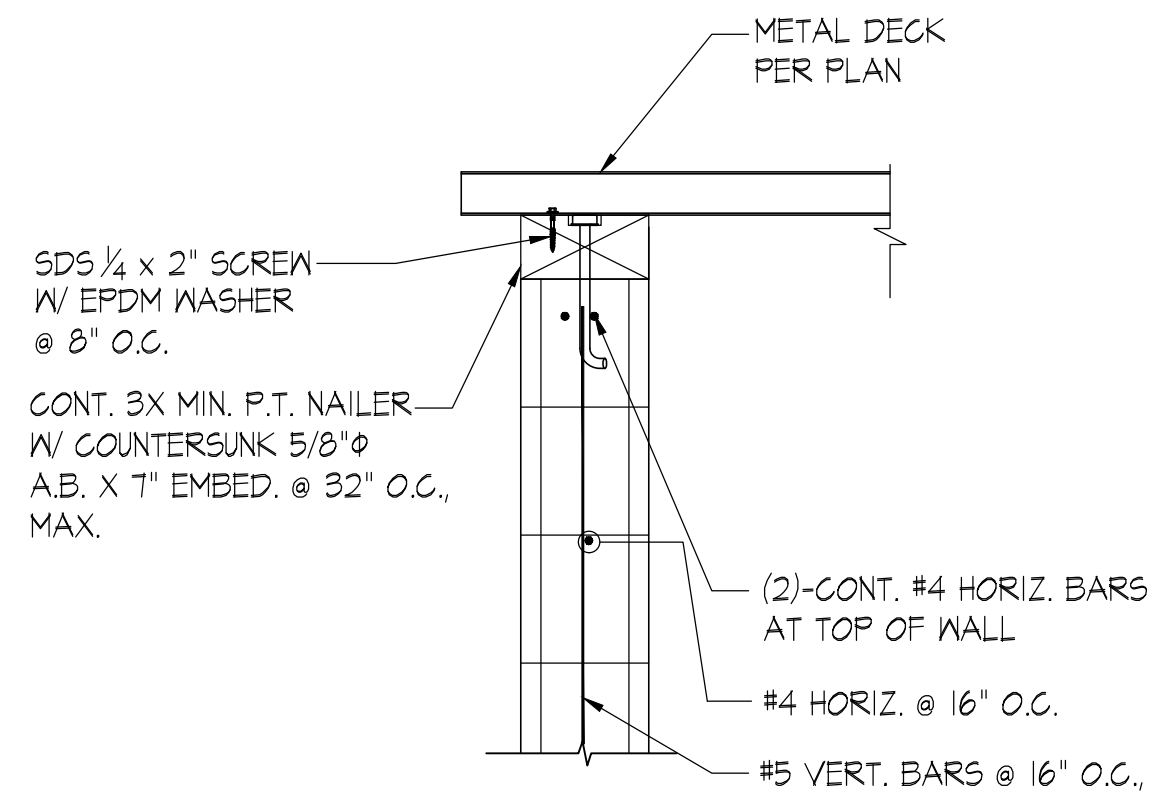
STRUCTURAL
DETAILS

PROJECT NO.: 020-MPC-037 PROJECT ARCH: Designer
DRAWN: MH CHECKED: WL

SHEET NUMBER:

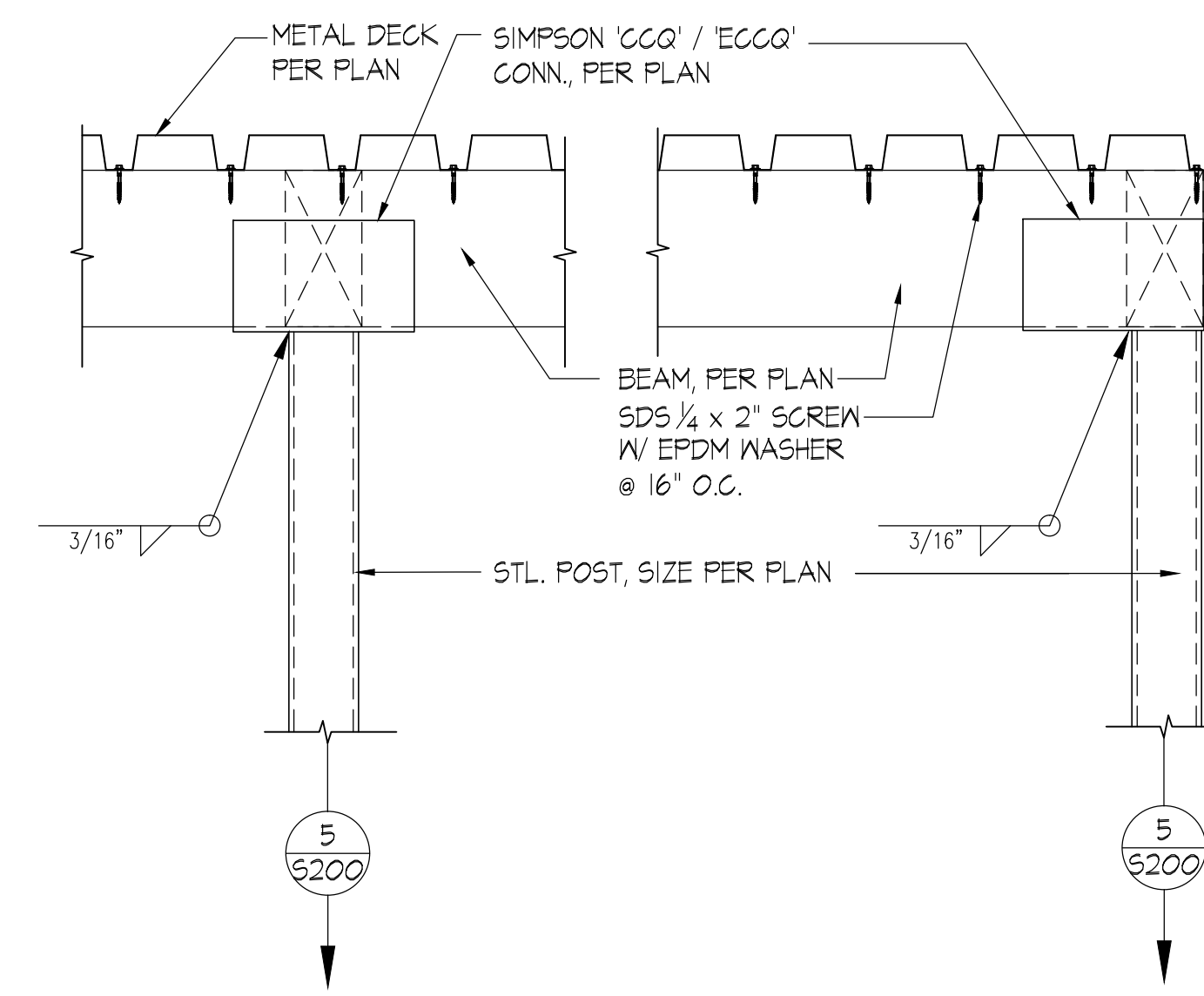
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DATE: 05/11/2021 SHEET: OF



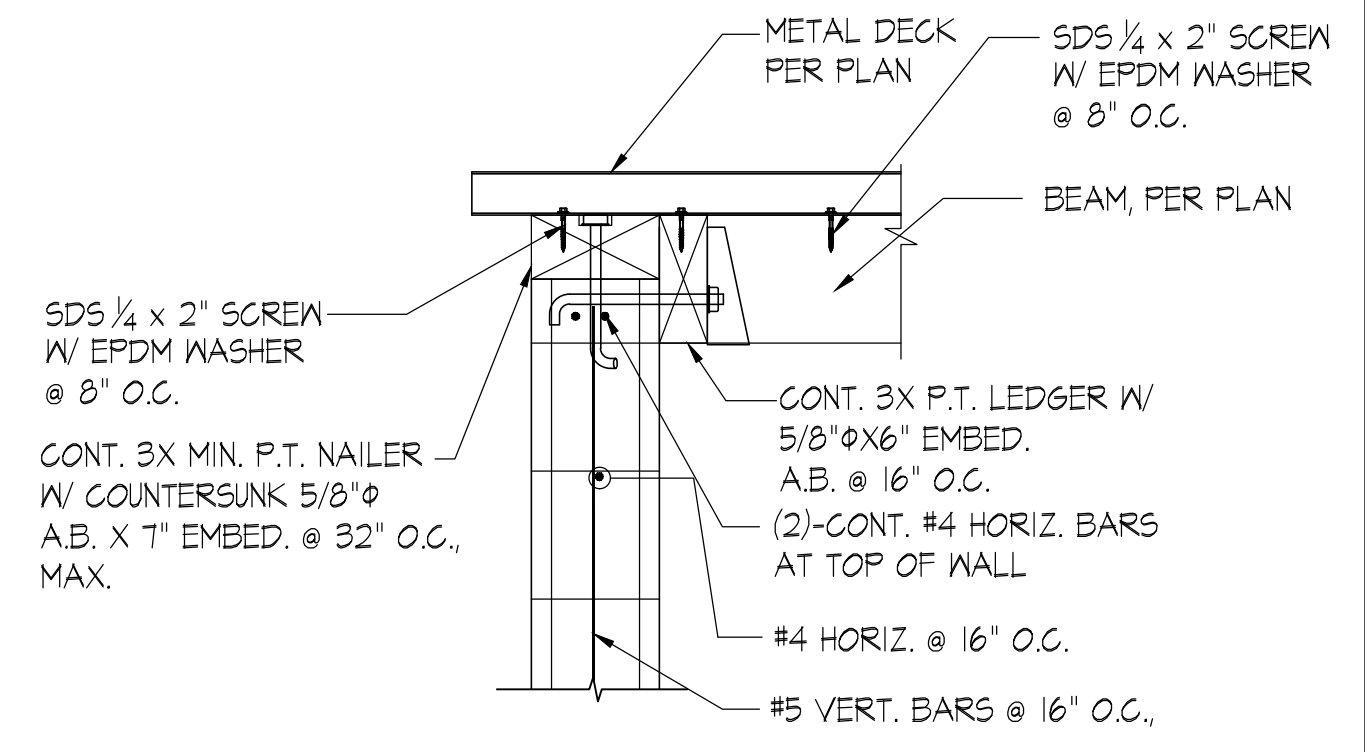
SCALE: 1"=1'-0" 22

SHEAR TRNFER TO CMU WALL



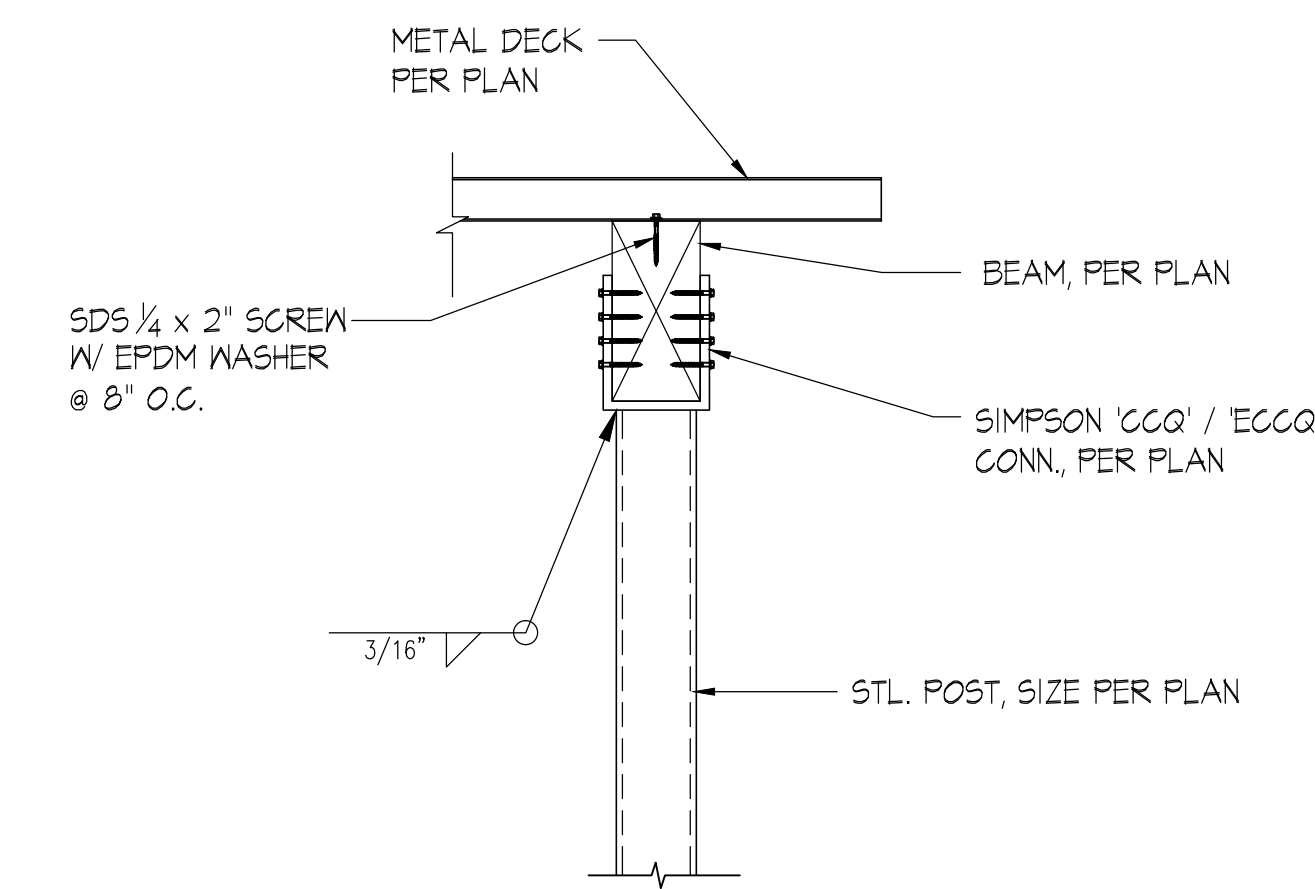
SCALE: 1"=1'-0" 19

STEEL POST CONNECTION AT WOOD BEAM



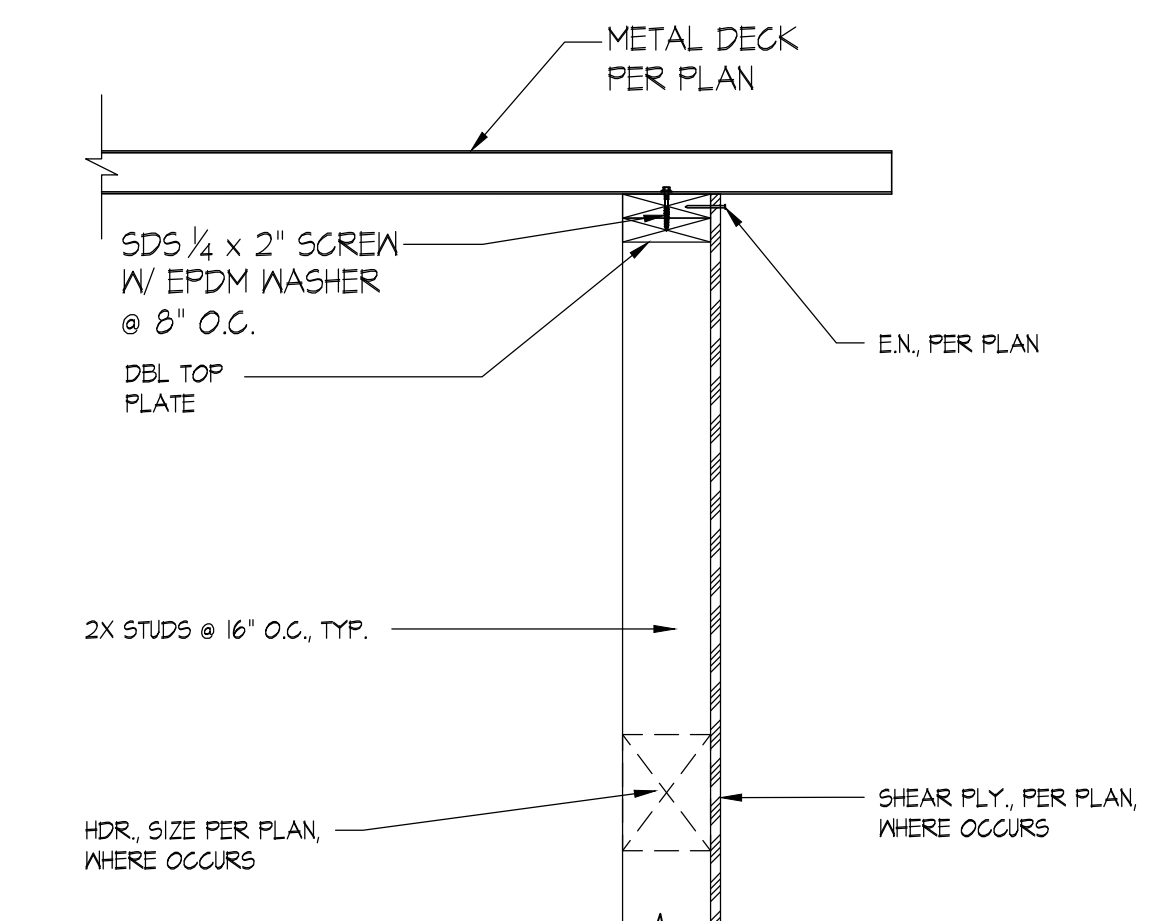
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SHEAR TRNFER TO CMU WALL



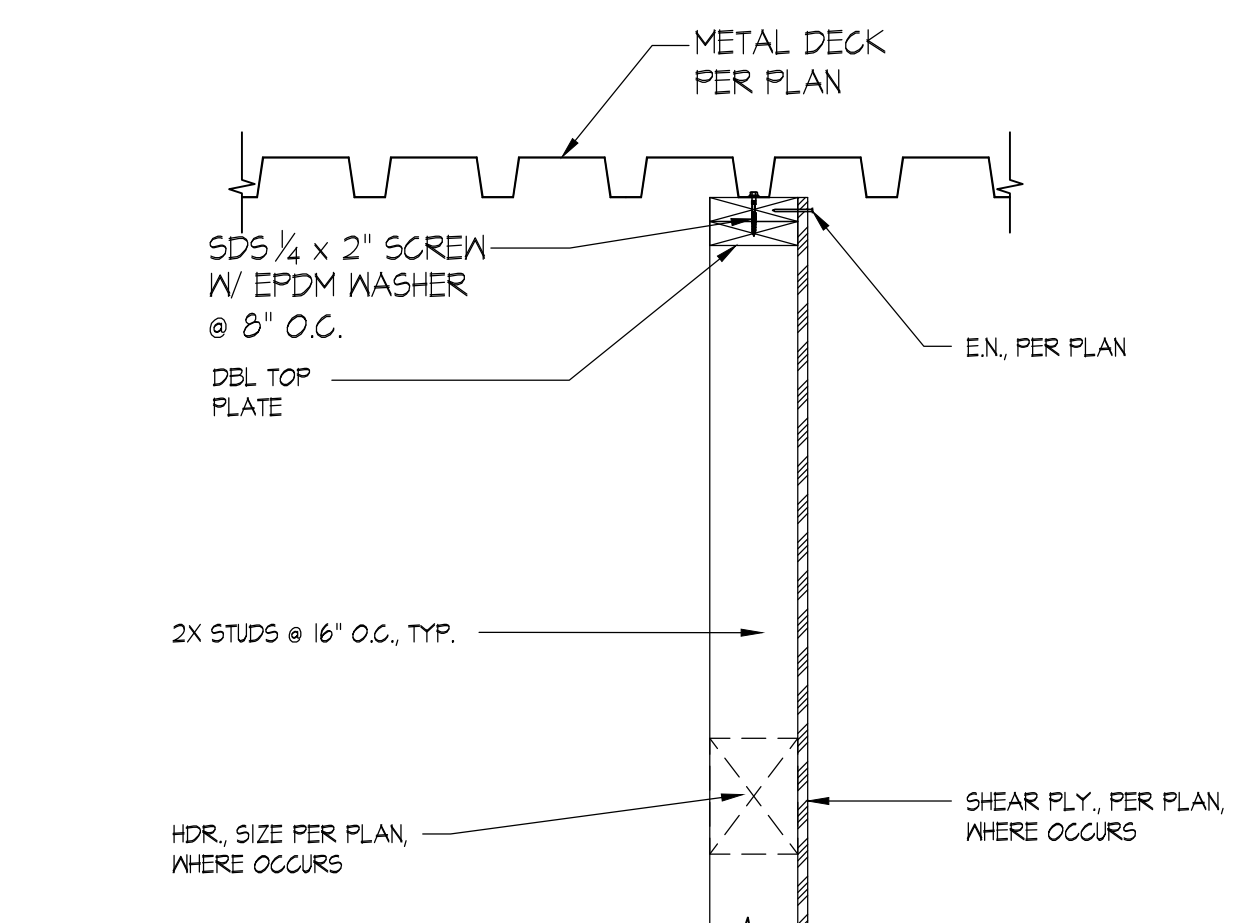
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SHEAR TRANSFER TO STEEL POST



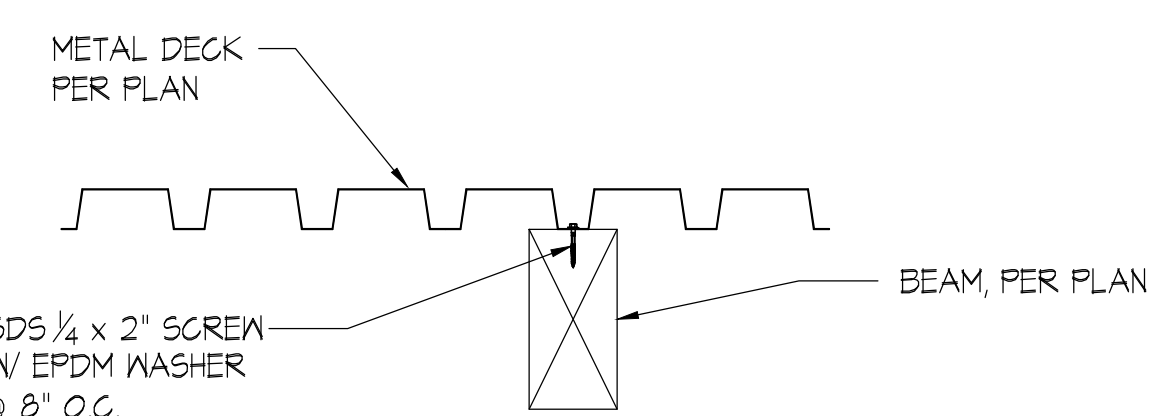
SCALE: 1"=1'-0" 17

MTL DECK CONN. TO DBL TOP PLATE



SCALE: 1"=1'-0" 23

SHEAR TRANSFER AT WOOD SHEAR WALL



SCALE: 1"=1'-0" 14

MTL DECK CONN. TO WOOD BM

SCALE: 1"=1'-0" 24

SHEAR TRANSFER AT WOOD SHEAR WALL

SCALE: 1"=1'-0" 21

SHEAR TRANSFER AT WOOD SHEAR WALL

SCALE: 1"=1'-0" 18

MTL DECK CONN. TO WOOD BM

SCALE: 1"=1'-0" 15

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PROJECT NO.: 020-MPC-037 PROJECT ARCH: Designer
DRAWN: MH CHECKED: WL

SHEET NUMBER:
S201

DATE: 05/11/2021 SHEET: ___ OF ___