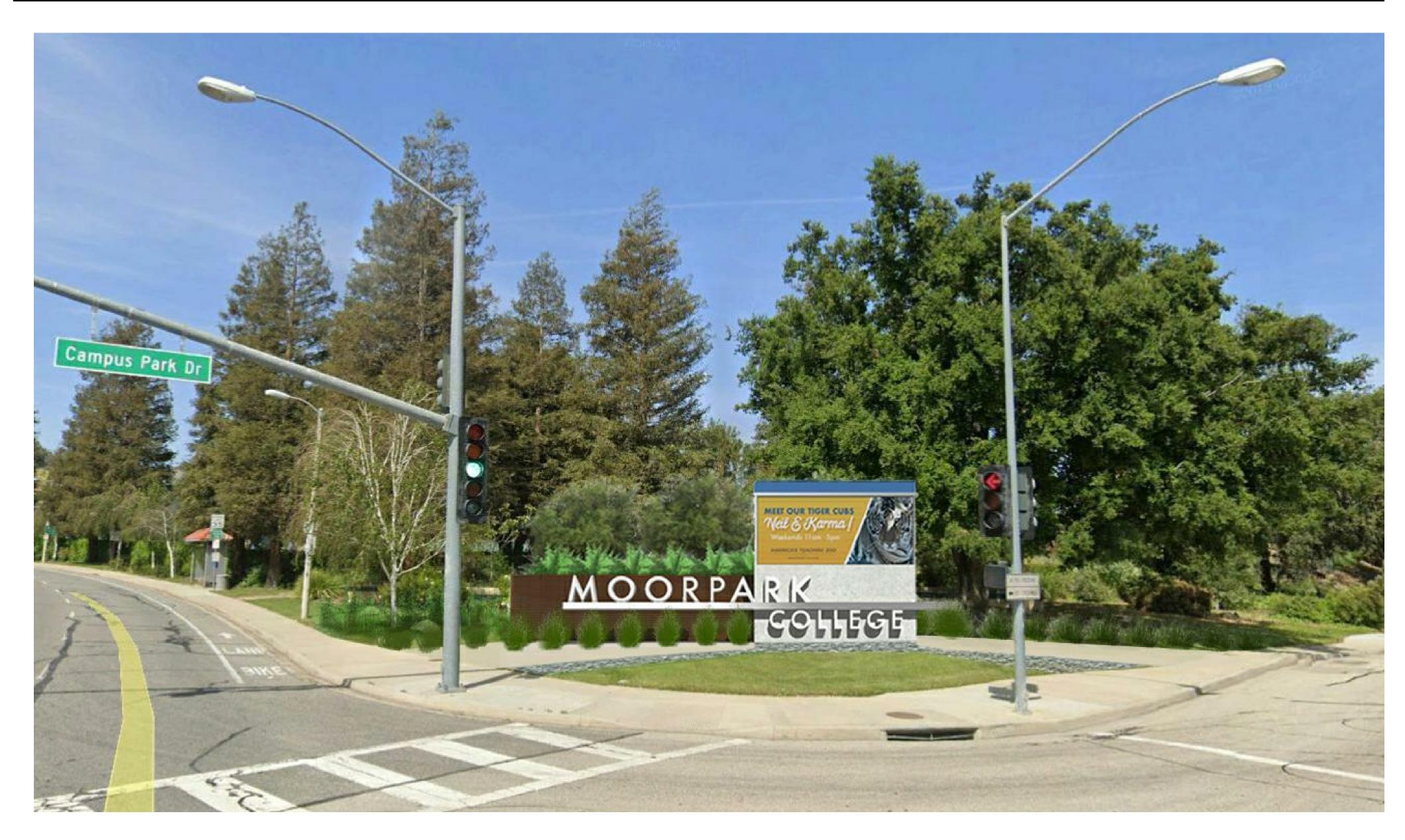
MOORPARK COLLEGE MAIN ENTRY SIGN

VENTURA COUNTY COMMUNITY COLLEGE DISTRICT

NEW SIGN RENDERING



16/2020 5:39:35 PN



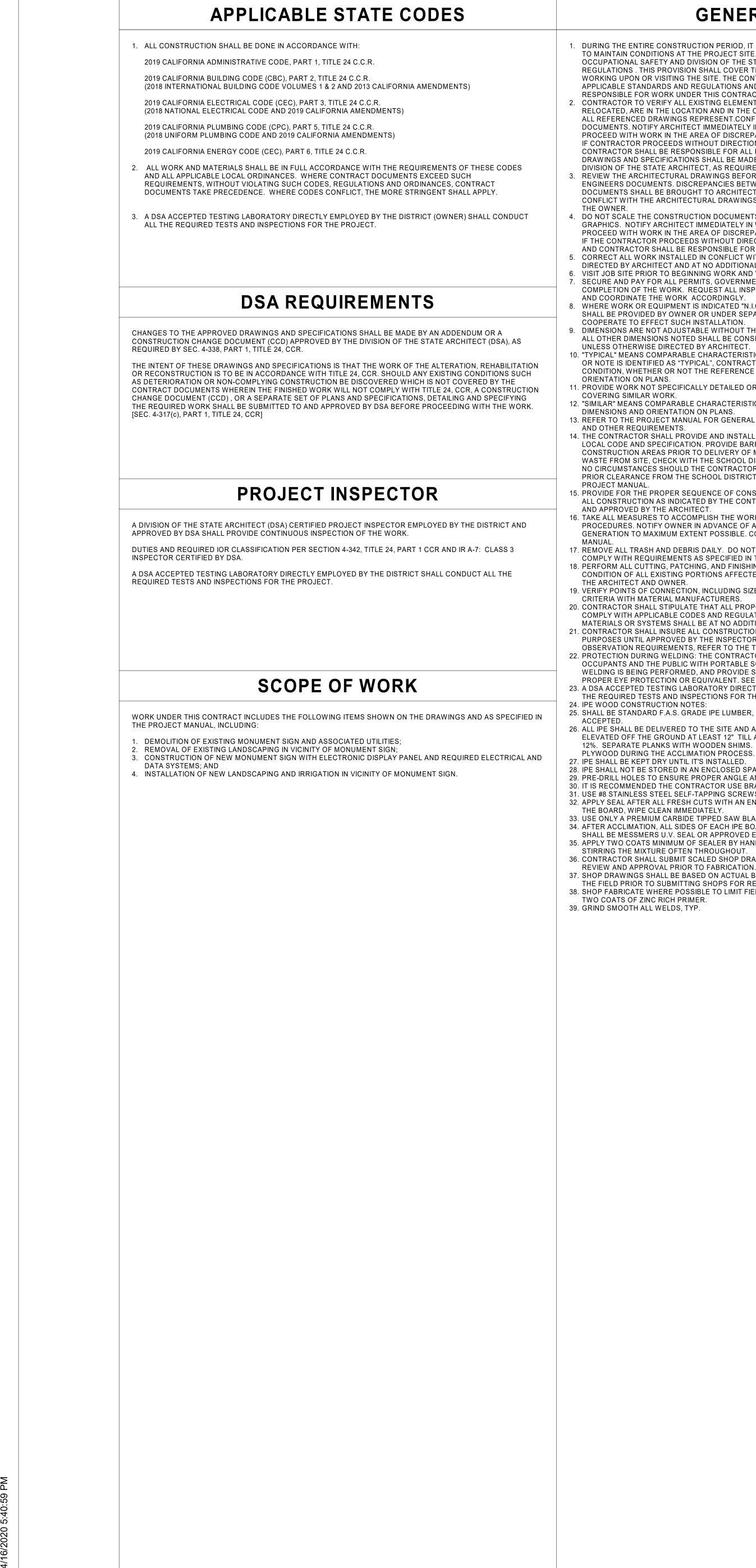


PROJECT LOCATION

NOT TO SCALE



AGENCY REVIEW	
DIV. OF THE S APP. 03-120 REVIEV SS 🗹 FL	ATION STAMP TATE ARCHITECT 493 INC: VED FOR .S I ACS // /30/2020
1300 Dove Newport B	TECTURAL CONSULTING Street, Suite 100 seach, CA. 92660
T: 94 www.littl This drawing and th property of Little Di Consulting. The rep use of this drawing n is prohibited and an to legal action.	9.698.1400 e online.com e design shown are the iversified Architectural roduction, copying or other without their written consent y infringement will be subject
VENTUR COMI	A COUNTY MUNITY E DISTRICT
MOORPARK COLLEGE MAIN ENTRY SIGN	7075 CAMPUS ROAD, MOORPARK, CA 93021
DSA A# 03-12049	3
ISSUE FOR DSA RESUBMITT ISSUE DATE 04/17/2020 REVISIONS	AL
NO. REASO	N DATE
PRINCIPAL IN CHARGE RITA CARTER PROJECT MANAGER EMAN BERMANI DESIGN TEAM JEFF HATFIELD PROJECT NAME	
COLLE ENTF	RPARK GE MAIN RY SIGN
613	696000
SHEET NUMBER	R SHEET
G	000



GENERAL NOTES

DURING THE ENTIRE CONSTRUCTION PERIOD, IT SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO MAINTAIN CONDITIONS AT THE PROJECT SITE, TO MEET THE REQUIREMENTS OF THE FEDERAL OCCUPATIONAL SAFETY AND DIVISION OF THE STATE ARCHITECT (DSA) AND CALIFORNIA OCCUPATIONAL REGULATIONS . THIS PROVISION SHALL COVER THE CONTRACTOR'S EMPLOYEES AND ALL OTHER PERSONS WORKING UPON OR VISITING THE SITE. THE CONTRACTOR SHALL BECOME FULLY INFORMED OF ALL APPLICABLE STANDARDS AND REGULATIONS AND INFORM ALL PERSONS AND REPRESENTATIVES RESPONSIBLE FOR WORK UNDER THIS CONTRACT.

2. CONTRACTOR TO VERIFY ALL EXISTING ELEMENTS, WHETHER THEY ARE TO REMAIN, BE REMOVED, OR RELOCATED, ARE IN THE LOCATION AND IN THE CONDITION THAT THESE CONSTRUCTION DOCUMENTS AND ALL REFERENCED DRAWINGS REPRESENT.CONFIRM ALL EXISTING CONDITIONS WITH THE CONTRACT DOCUMENTS. NOTIFY ARCHITECT IMMEDIATELY IN WRITING OF ALL DISCREPANCIES OR CONFLICTS. DO NOT PROCEED WITH WORK IN THE AREA OF DISCREPANCY OR CONFLICT UNTIL DIRECTION IS GIVEN BY ARCHITECT IF CONTRACTOR PROCEEDS WITHOUT DIRECTION FROM ARCHITECT, IT SHALL BE AT CONTRACTORS RISK, AND CONTRACTOR SHALL BE RESPONSIBLE FOR ALL REQUIRED CORRECTIVE ACTION.CHANGES TO THE APPROVED DRAWINGS AND SPECIFICATIONS SHALL BE MADE BY AN ADDENDUM OR A CHANGE ORDER APPROVED BY THE DIVISION OF THE STATE ARCHITECT, AS REQUIRED BY SECTION 4-338. REVIEW THE ARCHITECTURAL DRAWINGS BEFORE THE INSTALLATION OF SYSTEMS SHOWN ON CONSULTING

ENGINEERS DOCUMENTS. DISCREPANCIES BETWEEN THE ARCHITECTURAL AND CONSULTING ENGINEER'S DOCUMENTS SHALL BE BROUGHT TO ARCHITECT'S ATTENTION FOR DIRECTION. CONSTRUCTION INSTALLED IN CONFLICT WITH THE ARCHITECTURAL DRAWINGS SHALL BE CORRECTED BY CONTRACTOR AT NO EXPENSE TO 4. DO NOT SCALE THE CONSTRUCTION DOCUMENTS. WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED GRAPHICS. NOTIFY ARCHITECT IMMEDIATELY IN WRITING OF ALL ADDITIONAL REQUIRED DIMENSIONS. DO NOT

PROCEED WITH WORK IN THE AREA OF DISCREPANCY OR CONFLICT UNTIL DIRECTION IS GIVEN BY ARCHITECT. IF THE CONTRACTOR PROCEEDS WITHOUT DIRECTION FROM ARCHITECT, IT SHALL BE AT CONTRACTORS RISK, AND CONTRACTOR SHALL BE RESPONSIBLE FOR ALL REQUIRED CORRECTIVE ACTION. . CORRECT ALL WORK INSTALLED IN CONFLICT WITH THE CONSTRUCTION DOCUMENTS BY CONTRACTOR AS DIRECTED BY ARCHITECT AND AT NO ADDITIONAL EXPENSE TO THE OWNER. . VISIT JOB SITE PRIOR TO BEGINNING WORK AND VERIFY ALL DIMENSIONS AND CONDITIONS.

SECURE AND PAY FOR ALL PERMITS, GOVERNMENTAL FEES AND LICENSES REQUIRED FOR PROPER COMPLETION OF THE WORK. REQUEST ALL INSPECTIONS REQUIRED BY LOCAL GOVERNMENTAL AGENCIES

AND COORDINATE THE WORK ACCORDINGLY. 8. WHERE WORK OR EQUIPMENT IS INDICATED "N.I.C." (NOT IN CONTRACT) OR "BY OTHERS" ON THE DRAWINGS, SHALL BE PROVIDED BY OWNER OR UNDER SEPARATE CONTRACT. CONTRACTOR SHALL COORDINATE AND COOPERATE TO EFFECT SUCH INSTALLATION. DIMENSIONS ARE NOT ADJUSTABLE WITHOUT THE REVIEW OF ARCHITECT UNLESS NOTED (+/-) OR "VERIFY". ALL OTHER DIMENSIONS NOTED SHALL BE CONSIDERED AS ABSOLUTE AND USED FOR LAY-OUT CONTROL UNLESS OTHERWISE DIRECTED BY ARCHITECT.

10. "TYPICAL" MEANS COMPARABLE CHARACTERISTICS FOR THE ELEVATION OR DETAIL NOTED. WHEN A DETAIL OR NOTE IS IDENTIFIED AS "TYPICAL", CONTRACTOR SHALL APPLY THIS DETAIL OR NOTE TO EVERY LIKE CONDITION, WHETHER OR NOT THE REFERENCE IS REPEATED IN EVERY INSTANCE. VERIFY DIMENSIONS AND

11. PROVIDE WORK NOT SPECIFICALLY DETAILED OR SPECIFIED IN ACCORDANCE WITH DETAILS OR SIZES COVERING SIMILAR WORK. 12. "SIMILAR" MEANS COMPARABLE CHARACTERISTICS FOR THE ELEVATION OR DETAIL NOTED VERIFY DIMENSIONS AND ORIENTATION ON PLANS.

13. REFER TO THE PROJECT MANUAL FOR GENERAL CONDITIONS, SUPPLEMENTARY AND SPECIAL CONDITIONS AND OTHER REQUIREMENTS. 14. THE CONTRACTOR SHALL PROVIDE AND INSTALL TEMPORARY PEDESTRIAN PROTECTION AS REQUIRED BY LOCAL CODE AND SPECIFICATION. PROVIDE BARRICADES AND PROTECTIVE DEVICES SEPARATING CONSTRUCTION AREAS PRIOR TO DELIVERY OF MATERIALS TO CONSTRUCTION ZONE AND REMOVAL OF WASTE FROM SITE, CHECK WITH THE SCHOOL DISTRICT FOR ACCEPTABLE ACCESS ROUTE AND TIME UNDER NO CIRCUMSTANCES SHOULD THE CONTRACTOR USE AREAS OUTSIDE THE CONSTRUCTION ZONE WITHOUT

PRIOR CLEARANCE FROM THE SCHOOL DISTRICT. COMPLY WITH REQUIREMENTS AS SPECIFIED IN THE 15. PROVIDE FOR THE PROPER SEQUENCE OF CONSTRUCTION, LOCATION AND SIZE OF OPENINGS. COORDINATE ALL CONSTRUCTION AS INDICATED BY THE CONTRACT DOCUMENTS, INCLUDING SHOP DRAWINGS REVIEWED AND APPROVED BY THE ARCHITECT.

16. TAKE ALL MEASURES TO ACCOMPLISH THE WORK WITH THE MINIMUM OF INTERRUPTION TO NORMAL SCHOOL PROCEDURES. NOTIFY OWNER IN ADVANCE OF ANY SYSTEM SHUT-OFFS. MINIMIZE NOISE AND DUST GENERATION TO MAXIMUM EXTENT POSSIBLE. COMPLY WITH REQUIREMENTS AS SPECIFIED IN THE PROJECT 17. REMOVE ALL TRASH AND DEBRIS DAILY. DO NOT STORE BUILDING MATERIALS IN WALKWAYS AT ANY TIME.

COMPLY WITH REQUIREMENTS AS SPECIFIED IN THE PROJECT MANUAL. 18, PERFORM ALL CUTTING, PATCHING, AND FINISHING NECESSARY TO RESTORE THE SITE TO ORIGINAL CONDITION OF ALL EXISTING PORTIONS AFFECTED BY THE CONTRACTOR'S WORK, TO THE SATISFACTION OF THE ARCHITECT AND OWNER. 19. VERIFY POINTS OF CONNECTION, INCLUDING SIZES AND LOCATIONS, AND ALL OTHER REQUIRED OPERATING

CRITERIA WITH MATERIAL MANUFACTURERS. 20. CONTRACTOR SHALL STIPULATE THAT ALL PROPOSED SUBSTITUTIONS ARE EQUAL IN PERFORMANCE AND COMPLY WITH APPLICABLE CODES AND REGULATIONS. CONTRACTOR'S SUBSTITUTION OF ALTERNATE MATERIALS OR SYSTEMS SHALL BE AT NO ADDITIONAL COST TO THE OWNER. 21. CONTRACTOR SHALL INSURE ALL CONSTRUCTION SHALL REMAIN ACCESSIBLE AND EXPOSED FOR INSPECTION PURPOSES UNTIL APPROVED BY THE INSPECTOR OF RECORD. FOR CONTINUOUS INSPECTION, TESTING, AND OBSERVATION REQUIREMENTS, REFER TO THE TESTING AND OBSERVATION PROGRAM. 22. PROTECTION DURING WELDING: THE CONTRACTOR SHALL CONFORM TO TITLE 8, CCR, FURTHER PROTECT OCCUPANTS AND THE PUBLIC WITH PORTABLE SOLID VISION BARRICADES AROUND LOCATIONS WHERE WELDING IS BEING PERFORMED, AND PROVIDE SIGNS WARNING AGAINST LOOKING AT WELDING WITHOUT PROPER EYE PROTECTION OR EQUIVALENT. SEE 2016 CFC FOR REQUIREMENTS FOR ON SITE WELDING. 23. A DSA ACCEPTED TESTING LABORATORY DIRECTLY EMPLOYED BY THE SCHOOL DISTRICT SHALL CONDUCT ALL THE REQUIRED TESTS AND INSPECTIONS FOR THE PROJECT.

24. IPE WOOD CONSTRUCTION NOTES: 25. SHALL BE STANDARD F.A.S. GRADE IPE LUMBER, NO PRE-GROOVED OR TOUNGE AND GROOVE WILL BE 26. ALL IPE SHALL BE DELIVERED TO THE SITE AND ALLOWED TO ACCLIMATE IN A FLAT. DRY STACK W/ SPACERS. ELEVATED OFF THE GROUND AT LEAST 12" TILL ALL LUMBER HAS A MOISTURE CONTENT LESS THAN 12%. SEPARATE PLANKS WITH WOODEN SHIMS. SHALL BE COVERED WITH AN ADDITIONAL LAYER OF

27. IPE SHALL BE KEPT DRY UNTIL IT'S INSTALLED. 28. IPE SHALL NOT BE STORED IN AN ENCLOSED SPACE.

29. PRE-DRILL HOLES TO ENSURE PROPER ANGLE AND AVOID ALL CRACKING AND FISSURES. 30. IT IS RECOMMENDED THE CONTRACTOR USE BRAD POINT OR FOSTNER DRILL BITS FOR CLEANER HOLES. 31. USE #8 STAINLESS STEEL SELF-TAPPING SCREWS TO ATTACH IPE BOARDS TO ANGLE CLIPS. 32. APPLY SEAL AFTER ALL FRESH CUTS WITH AN END-GRAIN SEALER. IF END SEALANT TOUCHES THE FACE OF THE BOARD, WIPE CLEAN IMMEDIATELY. 33. USE ONLY A PREMIUM CARBIDE TIPPED SAW BLADE.

34. AFTER ACCLIMATION, ALL SIDES OF EACH IPE BOARD SHALL BE SEALED PRIOR TO INSTALLATION. SEALANT SHALL BE MESSMERS U.V. SEAL OR APPROVED EQUAL. SUBMIT TO ARCHITECT FOR REVIEW AND APPROVAL. 35. APPLY TWO COATS MINIMUM OF SEALER BY HAND WITH A BRUSH USING LIBERAL AMOUNTS OF SEALER, STIRRING THE MIXTURE OFTEN THROUGHOUT 36. CONTRACTOR SHALL SUBMIT SCALED SHOP DRAWINGS FOR ALL FENCE, GATE, AND WALL PANELS FOR

REVIEW AND APPROVAL PRIOR TO FABRICATION. 37. SHOP DRAWINGS SHALL BE BASED ON ACTUAL BUILT DIMENSIONS OF SIGN AND PLANTER WALL. VERIFY IN THE FIELD PRIOR TO SUBMITTING SHOPS FOR REVIEW. 38. SHOP FABRICATE WHERE POSSIBLE TO LIMIT FIELD WELDING. ALL FIELD WELDS SHALL BE TREATED WITH TWO COATS OF ZINC RICH PRIMER. 39. GRIND SMOOTH ALL WELDS, TYP.

PROJECT DIRECTORY

PROJECT MOORPARK COLLEGE MAIN ENTRY SIGN 705 CAMPUS ROAD. MOORPARK, CA 93021

OWNER VENTURA COUNTY COMMUNITY COLLEGE DISTRICT 761 EAST DAILY DRIVE, CAMARILLO, CA 93010 (805) 652-5500

ARCHITECT

LITTLE DIVERSIFIED ARCHITECTURAL CONSULTING 1300 DOVE STREET, SUITE 100 NEWPORT BEACH, CA 92660 (949) 698-1406

STRUCTURAL

LITTLE DIVERSIFIED ARCHITECTURAL CONSULTING 1300 DOVE STREET, SUITE 100 NEWPORT BEACH, CA 92660 (949) 698-1412

ELECTRICAL

STANDARDS INSTITUTE.

LUCCI & ASSOCIATES, INC. 3251 CORTE MALPASO, SUITE 511 CAMARILLO, CA 93012 (805) 389-6520

DEMOLITION AND CONSTRUCTION NOTES

- VERIFY ALL EXISTING CONDITIONS INCLUDING BUT NOT LIMITED TO, PLUMBING, IRRIGATION, ELECTRICAL AND ALL OTHER EXISTING SYSTEMS. MAKE NECESSARY PROVISIONS TO MAINTAIN THE INTEGRITY OF EXISTING
- SYSTEMS PRIOR TO THE COMMENCEMENT OF ANY DEMOLITION. REFER TO DOCUMENTS PREPARED BY CONSULTING ENGINEERS FOR INFORMATION REGARDING THE
- REMOVAL OF EXISTING CONDITIONS. COMPLY WITH ANSI A10.6 "SAFETY REQUIREMENTS FOR DEMOLITION" PUBLISHED BY THE AMERICAN NATIONAL

- **TRENCHING NOTES**
- PROTECT ALL UNDERGROUND UTILITIES IN PLACE.
- 2. NOTICE REGARDING EXISTING UTILITIES: THE EXISTENCE AND LOCATION OF ANY AND ALL CONDUITS, UTILITY PIPES, AND STRUCTURES SHOWN ON THIS SET OF PLANS ARE OBTAINED BY A SEARCH OF THE AVAILABLE RECORDS AT THE TIME OF DESIGN. THE CONTRACTOR IS REQUIRED TO TAKE DUE PRECAUTIONARY MEASURES TO PROTECT ANY AND ALL UTILITY LINES SHOWN ON THIS SET OF PLANS. THE CONTRACTOR FURTHER ASSUMES ANY AND ALL LIABILITY AND RESPONSIBILITY FOR THE CONDUITS, UTILITY PIPES, AND STRUCTURES SHOWN ON THIS SET OF DRAWINGS. CONTRACTOR AGREES THAT HE SHALL ASSUME SOLE AND COMPLETE RESPONSIBILITY FOR THE JOB SITE CONDITIONS DURING THE COURSE OF CONSTRUCTION OF THIS PROJECT. THIS STIPULATION INCLUDES THE SAFETY OF ANY AND ALL PERSONS AND PROPERTY. THE CONTRACTOR SHALL FURTHER DEFEND, IMDEMNIFY, AND HOLD THE OWNER AND ARCHITECT HARMLESS FROM ANY AND ALL LIABILITY, REAL OR ALLEGED, IN

ARISING FROM THE SOLE NEGLIGENCE OF THE OWNER OR ARCHITECT.

(949) 698-1488

CIVIL

LITTLE DIVERSIFIED ARCHITECTURAL CONSULTING 1300 DOVE STREET, SUITE 100 NEWPORT BEACH, CA 92660 (949) 698-1400

LANDSCAPE LITTLE DIVERSIFIED ARCHITECTURAL CONSULTING 1300 DOVE STREET, SUITE 100 NEWPORT BEACH, CA 92660

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COVER SHEET G000

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E400	ELECTRICAL PLAN NEW WORK
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CONNECTION WITH THE PERFORMANCE OF WORK ON THIS PROJECT, WITH THE EXCEPTION OF LIABILITY

TOTAL SHEET COUNT 26

ELECTRICAL DETAILS

E600

AGENCY REVIEW IDENTIFICATIO DIV. OF THE STATE APP. 03-120493 REVIEWED SS I FLS I DATE: 06/30/2	E ARCHITECT INC: FOR ACS
DIVERSIFIED ARCHITEC	
Newport Beach T: 949.69 w w w . little o This drawing and the desi property of Little Diversif	n, CA. 92660 8.1400 nline.com ign shown are the
Consulting. The reproduct use of this drawing withou is prohibited and any infra- to legal action.	tion, copying or other t their written consent ingement will be subject
VENTURA COMM COLLEGE	UNITY
MOORPARK COLLEGE MAIN ENTRY SIGN	7075 CAMPUS ROAD, MOORPARK, CA 93021
CONSULTANT DSA A# 03-120493 SEAL	
ISSUE FOR DSA RESUBMITTAL DSA A# 03-120493 ISSUE DATE 04/17/2020	
REVISIONS NO. REASON	DATE
PROJECT TEAM PRINCIPAL IN CHARGE RITA CARTER PROJECT MANAGER EMAN BERMANI DESIGN TEAM JEFF HATFIELD	
PROJECT NAME MOORI COLLEG ENTRY PROJECT NO.	E MAIN
61369 Sheet title	
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GENERAL NOTES

WORK SHALL BE PERFORMED ACCORDING TO THE LATEST EDITIONS OF THE STANDARD SPECIFICATIONS AND PLANS FOR PUBLIC WORKS CONSTRUCTION (GREEN BOOK & S.P.P.W.C), THE LATEST EDITION OF CALIFORNIA BUILDING CODE AND CITY OF MOORPARK BUILDING CODE REQUIREMENTS. 2. NO WORK SHALL BE STARTED WITHOUT A PRE-CONSTRUCTION MEETING WITH THE OWNER AND INSPECTOR.

3. THE CONTRACTOR SHALL PROVIDE FOR CONTRIBUTORY DRAINAGE AT ALL TIMES AND TAKE ALL NECESSARY AND PROPER PRECAUTIONS TO PROTECT ADJACENT PROPERTIES AND IMPROVEMENTS FROM ANY AND ALL DAMAGE THAT MAY OCCUR FROM STORM WATER RUNOFF AND/OR DEPOSITION OF DEBRIS RESULTING FROM ANY AND ALL WORK.

4. NO REVISIONS SHALL BE MADE TO THESE PLANS WITHOUT THE

APPROVAL OF THE CIVIL ENGINEER. . IMPORTANT NOTICE - SECTION 4216/4217 OF THE GOVERNMENT CODE REQUIRES A DIG ALERT IDENTIFICATION NUMBER BE ISSUED BEFORE ANY "PERMIT TO FXCAVATE" WILL BE VALID. FOR YOUR DIG ALERT I.D. NUMBER, CALL UNDERGROUND SERVICE ALERT TOLL FREE @ 1-800-422-4133, TWO WORKING DAYS BEFORE YOU DIG.

ANY IMPROVEMENT(S) TO BE CONSTRUCTED WITHIN PUBLIC RIGHT-OF-WAY WILL REQUIRE SEPARATE CONSTRUCTION PERMIT AND INSPECTION FROM THE GOVERNING AGENCY(IES). CONTRACTOR SHALL BE RESPONSIBLE FOR SECURING ALL APPLICABLE PERMITS AND PAYING ANY REQUIRED FEES.

7. FILLS SHALL BE COMPACTED THROUGHOUT TO AT LEAST 90% OF MAXIMUM DRY DENSITY AS DETERMINED BY A.S.T.M. SOIL COMPACTION TEST D 1557.

8. CONTRACTOR SHALL BE RESPONSIBLE FOR PRESERVING ALL GRADE STAKES UNTIL AUTHORIZED BY SURVEYOR TO REMOVE.

9. CONTRACTOR SHALL RESTORE LIKE FOR LIKE, TO THE SATISFACTION OF THE OWNER/ARCHITECT, ALL AREAS DAMAGED OR DISTURBED AS A RESULT OF WORK PERFORMED PURSUANT TO THESE PLANS AT HIS/HERS OWN EXPENSE. IO. FIELD DENSITY MAY BE DETERMINED BY THE NUCLEAR DENSITY METHOD A.S.T.M. D2922 & D3017 PROVIDED NOT LESS THAN 10% OF THE REQUIRED DENSITY TESTS UNIFORMLY DISTRIBUTED ARE BY THE SAND-CONE METHOD. THE METHOD OF DETERMINING FIELD DENSITY AND LOCATION AND APPROXIMATE ELEVATION SHALL BE SHOWN IN THE COMPACTION REPORT. OTHER METHODS MAY BE USED IF RECOMMENDED BY THE SOILS ENGINEER AND APPROVED IN ADVANCE BY THE CITY ENGINEER.

1. CRUSHED AGGREGATE BASE MATERIAL SHALL CONFORM TO SUBSECTION 200-2.2 OF STANDARD SPECIFICATIONS AND SHALL BE COMPACTED TO 95% RELATIVE COMPACTION USING MECHANICAL COMPACTING EQUIPMENT. 12. NEW CONCRETE SHALL BE CLASS 520-C-3250 (310-C-17) CONFORMING

WITH S.S.P.W.C. 201–1.1.2.(UNLESS NOTED OTHERWISE) 13. THE CONTRACTOR IS RESPONSIBLE FOR PROTECTING ALL EXISTING UTILITIES WHETHER SHOWN OR NOT SHOWN ON THESE DRAWINGS. THE CONTRACTOR FURTHER ASSUMES ALL LIABLITY AND RESPONSIBILITY FOR THE UTILITY PIPES, CONDUITS, OR STRUCTURES SHOWN OR NOT SHOWN ON THESE DRAWINGS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROTECTING ALL PUBLIC AND PRIVATE PROPERTY INSOFAR AS I MAY BE AFFECTED BY THESE OPERATIONS. ALL COSTS FOR PROTECTING, REMOVING, AND RESTORING EXISTING IMPROVEMENTS SHALL BE BORNE BY THE CONTRACTOR. 14. CONSTRUCTION CONTRACTOR AGREES THAT IN ACCORDANCE WITH GENERALLY ACCEPTED CONSTRUCTION PRACTICES, CONSTRUCTION CONTRACTOR WILL BE REQUIRED TO ASSUME SOLE AND COMPLETE RESPONSIBILITY FOR JOB SITE CONDITIONS DURING THE COURSE OF CONSTRUCTION OF THE PROJECT, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY. THIS REQUIREMENT SHALL BE IN EFFECT AT ALL TIMES.

15. THE CONTRACTOR SHALL VERIFY ALL JOINT ELEVATIONS PRIOR TO THE REMOVAL OF PAVEMENT, CURB, GUTTER, SIDEWALK AND/OR SLOPE GRADING. ANY DISCREPANCIES SHALL BE REPORTED TO THE ENGINEER OF RECORD PRIOR TO REMOVALS WITHIN THE AREA OF THE DISCREPANCIES. 16. DUST SHALL BE CONTROLLED BY WATERING TO THE SATISFACTION OF

THE INSPECTOR. 17. WHERE THE IRRIGATION SYSTEM IN CONFLICT WITH NEW WORK NEEDS TO BE RELOCATED OR REPLACED, CONTRACTOR SHALL COORDINATE THE

WATER SHUT OFF OR ANY ELECTRICAL RELATED WORK WITH OWNER 48 HOURS PRIOR COMMENCING THE WORK 18. ALL EXPOSED P.C.C. CORNERS SHALL BE ROUNDED WITH A 1/2" RADIUS.

19 ALL EXPORT OF MATERIAL FROM THE SITE MUST GO TO A PERMITTED SITE APPROVED BY THE BUILDING OFFICIAL OR A LEGAL DUMPSITE. RECEIPTS FOR ACCEPTANCE OF EXCESS MATERIAL BY A DUMPSITE ARE REQUIRED AND MUST BE PROVIDED TO THE BUILDING OFFICIAL UPON REQUEST.

20. CONTRACTOR TO CALCULATE HIS/HER OWN EARTHWORK QUANTITIES FOR BIDDING PURPOSES.

21. FOR JOINTS AT NEW CURB AND SIDEWALK REFER TO S.P.P.W.C. STD. PLAN No. 112-2. ALSO SEE DETAILS ON THIS SHEET FOR ADDITIONAL INFORMATION JOINT DETAILS. 22. IF WORK IS COMMENCED DURING RAINY SEASON, CONTRACTOR SHALL SATISFY CITY OF MOORPARK EROSION CONTROL REQUIREMENTS AND INSTALL APPROPRIATE BMPs.

LEGEND

	FS TC	FINISH SURFACE ELEVATION TOP OF CURB ELEVATION
	TS	TOP OF CONCRETE SLAB ELEVATION
	XXX.XX	PROPOSED SPOT ELEVATION
	(XXX.XX)	EXISTING SPOT ELEVATION
		CMU WALL
	—x——	EXISTING FENCE
	G.B.	GRADE BREAK
	ESW	EDGE OF SIDEWALK
	DWY	DRIVEWAY
	C&G	CURB & GUTTER
	H.P.	HIGH POINT
	NG	NATURAL GROUND
	S.P.P.W.C.	STANDARD PLANS FOR PUBLIC WORKS CONSTRUCTION
	S.S.P.W.C.	STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION
	C.F.	
	ELEV.	ELEVATION
	EX.	
	BCR. ECR.	BEGIN CURB RETURN END CURB RETURN
	A.P.	ANGLE POINT
/ NOTE IBOL	$\langle x \rangle$	FURNISH AND INSTALL/CONSTRUCT, DEMOLISH, REMOVE AND REPLACE, OR RELOCATE, AS INDICATED.
	XX.X%	NEW SLOPE
	(<u>XX.X)</u> %	EXISTING SLOPE
	FL	FLOW LINE
	T.B.M.	TEMPORARY BENCH MARK
	CONC.	CONCRETE PAVEMENT
	A.C.	ASPHALT CONCRETE PAVING
	(N)	NEW
	F.F.	FINISH FLOOR
	A.F.F.	ABOVE FINISH FLOOR
	EG	EDGE OF GUTTER
	CLR.	CLEAR
	SCO	SEWER CLEAN-OUT
	SMH	SEWER MANHOLE
	P.A.	PLANTER AREA
	D.I.	DRAINAGE INLET
	TS	TOP OF SLAB
	C.J.	CONTROL JOINT
	E.J.	EXPANSION JOINT
	CPB	COMMUNICATIONS PULL-BOX
	EPB RD	ELECTRICAL PULL-BOX ROOF DRAIN
		DETAIL #



call: TOLL FREE 1-800-422-4133

TWO WORKING DAYS BEFORE YOU DIG

BEST MANAGEMENT PRACTICE NOTES: 1. EVERY EFFORT SHOULD BE MADE TO ELIMINATE THE DISCHARGE OF NON- STORMWATER FROM THE

PROJECT SITE AT ALL TIMES. 2. FRODED SEDIMENTS AND OTHER POLLUTANTS MUST BE RETAINED ON-SITE AND MAY NOT BE

- TRANSPORTED FROM THE SITE VIA SHEET FLOW, SWALES, AREA DRAINS, NATURAL DRAINAGE COURSES OR WIND 3. STOCKPILES OF EARTH AND OTHER CONSTRUCTION RELATED MATERIALS MUST BE PROTECTED FROM
- BEING TRANSPORTED FROM THE SITE BY THE FORCES OF WIND OR WATER. 4. FUELS, OILS, SOLVENTS, AND OTHER TOXIC MATERIALS MUST BE STORED IN ACCORDANCE WITH THEIR LISTING AND ARE NOT TO CONTAMINATE THE SOIL AND SURFACE WATERS. ALL APPROVED STORAGE CONTAINERS ARE TO BE PROTECTED FROM THE WEATHER. SPILLS MUST BE CLEANED UP IMMEDIATELY AND DISPOSED OF IN A PROPER MANNER. SPILLS MAY NOT BE WASHED INTO THE DRAINAGE SYSTEM.
- 5. EXCESS OR WASTE CONCRETE MAY NOT BE WASHED INTO THE PUBLIC WAY OR ANY OTHER DRAINAGE SYSTEM. PROVISIONS SHALL BE MADE TO RETAIN CONCRETE WASTES ON-SITE UNTIL THEY CAN BE DISPOSED OF AS SOLID WASTE.
- 6. TRASH AND CONSTRUCTION RELATED SOLID WASTES MUST BE DEPOSITED INTO A COVERED RECEPTACLE TO PREVENT CONTAMINATION OF RAINWATER AND DISPERSAL BY WIND.
- 7. SEDIMENTS AND OTHER MATERIALS MAY NOT BE TRACKED FROM THE SITE BY VEHICLE TRAFFIC. THE CONSTRUCTION ENTRANCE ROADWAYS MUST BE STABILIZED SO AS TO INHIBIT SEDIMENTS FROM BEING DEPOSITED INTO THE PUBLIC WAY. ACCIDENTAL DEPOSITIONS MUST BE SWEPT UP
- IMMEDIATELY AND MAY NOT BE WASHED DOWN BY RAIN OR OTHER MEANS. 8. ANY SLOPES WITH DISTURBED SOILS OR DENUDED OF VEGETATION MUST BE STABILIZED SO AS TO INHIBIT EROSION BY WIND AND WATER.

THE FOLLOWING BMPS AS OUTLINED IN, BUT NOT LIMITED TO, THE CALIFORNIA STORMWATER BEST MANAGEMENT PRACTICES HANDBOOK THE LATEST REVISED EDITION, MAY APPLY DURING THE CONSTRUCTION OF THIS PROJECT (ADDITIONAL MEASURES MAY BE REQUIRED IF DEEMED APPROPRIATE BY THE PROJECT ENGINEER OR THE BUILDING OFFICIAL)

WIND FROM	SION CONTROL			CONTROL
		EC1		SCHEDULING
VE1 —	WIND EROSION CONTROL	EC2	_	PRESERVATION OF EXISTING VEGETATI HYDRAULIC MULCH
	T TRACKING CONTROL	EC4	_	HYDROSEEDING
		EC5	_	SOIL BINDERS
C1 —	STABILIZED CONSTRUCTION ENTRANCE EXIT STABILIZED CONSTRUCTION ROADWAY	EC6	-	STRAW MULCH
C2 -	STABILIZED CONSTRUCTION ROADWAY	EC7	_	GEOTEXTILES & MATS
	ENTRANCE/OUTLET TIRE WASH	EC0	_	WOOD MULCHING EARTH DIKES AND DRAINAGE SWALES VELOCITY DISSIPATION DEVICES
<u>ON-STO</u>	RMWATER MANAGEMENT	EC10	_	VELOCITY DISSIPATION DEVICES SLOPE DRAINS
IC1 W/A-	FER CONSERVATION PRACTICES	EC11	—	SLOPE DRAINS
	VATERING OPERATIONS	EC12	-	STREAMBANK STABILIZATION POLYACRYLAMIDE
	/ING AND GRINDING OPERATIONS	EC13	_	POLYACRYLAMIDE
	CLEAR WATER DIVERSION			
	ILLICIT CONNECTION/DISCHARGER			
√S8 –	POTABLE WATER/IRRIGATION VEHICLE AND EQUIPMENT CLEANING			
IS9 -	VEHICLE AND EQUIPMENT FUELING			
	VEHICLE AND EQUIPMENT MAINTENANCE			
NS12 -	CONCRETE CURING			
NS13 - NS14 -	CONCRETE FINISHING MATERIAL AND EQUIPMENT USE			
NS14 -	DEMOLITION ADJACENT TO WATER			
	TEMPORARY BATCH PLANTS			
	NAGEMENT & MATERIAL POLLUTION CONTROL			
WM1 —	MATERIAL DELIVERY AND STORAGE			
NM2 —	MATERIAL USE			
WM3 —	STOCKPILE MANAGEMENT			
WM4 -	SPILL PREVENTION AND CONTROL SOLID WASTE MANAGEMENT			
NM5 -	SOLID WASTE MANAGEMENT			
WMD — WM7 —	HAZARDOUS WASTE MANAGEMENT CONTAMINATION SOIL MANAGEMENT			
WM7 — WM8 —	CONCRETE WASTE MANAGEMENT			Р
WMQ _	SANITARY/SEPTIC WASTE MANAGEMENT			1
NM19 — NM10 —	LIQUID WASTE MANAGEMENT			
	Y SEDIMENT CONTROL			
ЪЕ1 — № 2 —	SILT FENCE SEDIMENT BASIN SEDIMENT TRAP			
SE3 -	SEDIMENT TRAP			1
5E4 -	CHECK DAM			

SE3 –	SEDIMENT TRAP
SE4 –	CHECK DAM
SE5 –	FIBER ROLLS

- GRAVEL BAG BERM SE6 – STREET SWEEPING AND VACUUMING SE7 -
- SE8 SANDBAG BARRIER STRAW BALE BARRIER SE9 -STORM DRAIN INLET PROTECTION

PRIVATE ENGINEER'S NOTICE TO CONTRACTOR

THE EXISTENCE AND LOCATION OF ANY AND ALL CONDUITS. UTILITY PIPE AND STRUCTURES SHOWN ON THIS SET OF PLANS ARE OBTAINED BASED ON AVAILABLE RECORDS AT THE TIME OF DESIGN. TO THE BEST OF OUR KNOWLEDGE, THERE ARE NO EXISTING UTILITIES WITHIN THE CONSTRUCTION LIMITS OF THIS PROJECT AT THE TIME OF DESIGN EXCEPT AS SHOW THIS SET OF PLANS. THE CONTRACTOR IS REQUIRED TO TAKE DUE PRECAUTIONARY MEASURES TO PROTECT ANY AND ALL UTILITY LI SHOWN ON THIS SET OF PLANS. THE CONTRACTOR FURTHER ASSUM ANY AND ALL LIABILITY AND RESPONSIBILITY FOR THE CONDUITS, U PIPES, AND STRUCTURES SHOWN ON THIS SET OF DRAWINGS. CONTRACTOR AGREES THAT HE SHALL ASSUME SOLE AND COMPLE RESPONSIBILITY FOR THE JOB SITE CONDITIONS DURING THE COURS CONSTRUCTION OF THIS PROJECT. THIS STIPULATION INCLUDES THE SAFETY OF ANY AND ALL PERSONS AND PROPERTY. THE CONTRA(SHALL FURTHER DEFEND, INDEMNIFY, AND HOLD THE OWNER AND NGINEER HARMLESS FROM ANY AND ALL LIABILITY, REAL OR ALLE IN CONNECTION WITH THE PERFORMANCE OF WORK ON THIS PROJE WITH THE EXCEPTION OF LIABILITY ARISING FROM THE SOLE NEGLIGE

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SECTION 31 22 00 GRADING

PART 1 GENERAL

OF THE OWNER OR ENGINEER.

1.01 SECTION INCLUDE

A. Coordinate work of this Section to compliment and coordinate with field conditions and Civil Drawing noted specific referenced requirements. Utilize the most stringent requirements. B. Removal of topsoil.

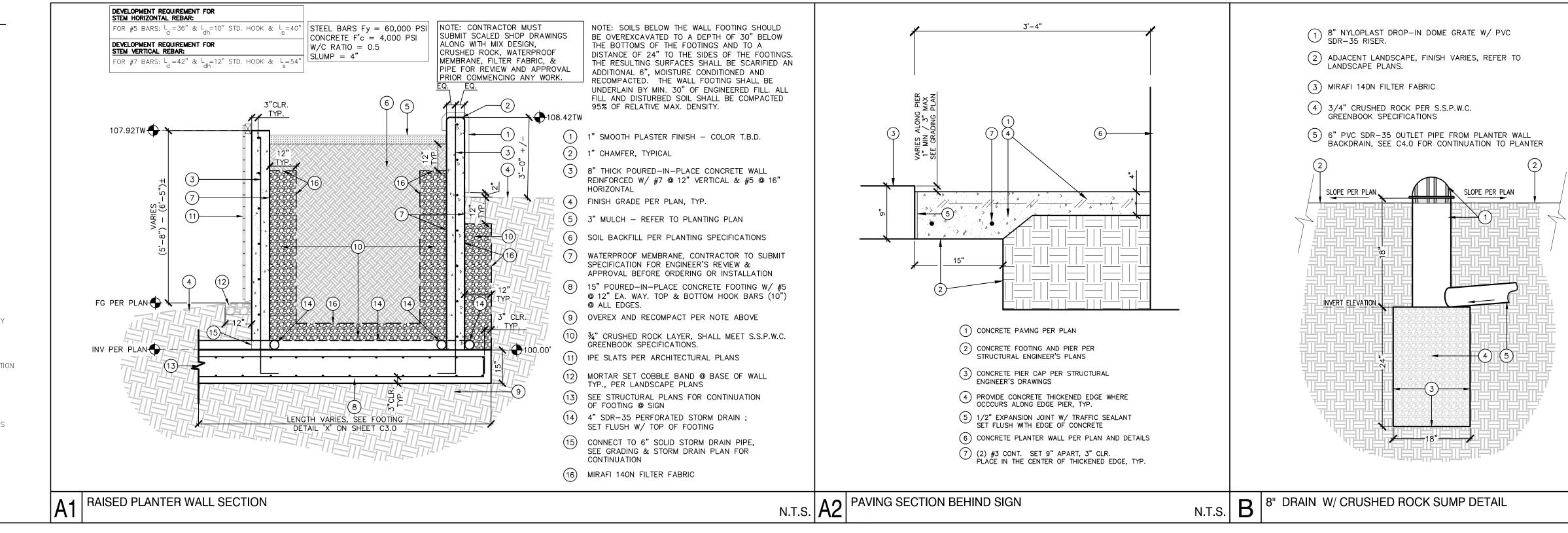
C. Rough grading and consolidation/compaction the site .

- D. Excavation of subsoil, stockpiling for later reuse, and removal of excess from the site.
- E. Preparing of subgrade for walks, pavements and site retaining walls. F. Excavating, backfilling and compaction for wet utility lines.
- G. Finish grading.
- 1.02 RELATED REQUIREMENTS A. Section 31 10 00 - Site Clearing.
- B. Section 31 23 16 Excavation.
- C. Section 31 23 23 Fill: Filling and compaction. D. Section 32 13 13 - Concrete Paving.
- 1.03 SUBMITTALS

A. Project Record Documents: Accurately record actual locations of utilities remaining by horizontal dimensions, elevations or inverts, and slope gradients. 1. Accurately record location of all changes in finish elevations and gradients which materially affect drainage.

- 1.04 QUALITY ASSURANCE
- A. Regulatory Requirements: For conditions not covered in this Section, refer to applicable provisions of the California Building Code (CBC), Chapter 18A - Soils and Foundations, as amended and adopted by authorities having jurisdiction.
- B. Perform Work in accordance with locally adopted SSPWC standards. 1.05 PROTECTION
- A. Dust Control: Comply with requirements specified in Section 01 50 00 Temporary Facilities
- B. Protection: 1. Comply with general requirements specified in Section 01 50 00 - Temporary Facilities
- and Controls 2. Provide protection for walks, curbs, drains, and trees and boxing around corners of existing buildings to prevent damage.

GRADING SPECIFICATIONS



SECTION 31 10 00 SITE CLEARING

PART 1 GENERAL .01 SECTION INCLUDES

- A. Clearing and protection of vegetation.
- B. Grubbing of root systems of trees and shrubs, abandoned utility lines and structures and other below grade obstructions and debris.
- C. Removal of existing debris.
- .02 RELATED REQUIREMENTS
- A. Section 01 10 00 Summary: Limitations on Contractor's use of site and premises
- B. Section 01 50 00 Temporary Facilities and Controls: Site fences, security, protective barriers, and waste removal.
- C. Section 01 57 13 Temporary Erosion and Sediment Control.
- D. Section 01 70 00 Execution and Closeout Requirements: Project conditions; protection of bench marks, survey control points, and existing construction to remain; reinstallation o
- removed products.
- E. Section 01 74 19 Construction Waste Management and Disposal: Limitations on disposal of
- removed materials; requirements for recycling. F. Section 02 41 00 - Demolition: Removal of built elements and utilities.
- 1. Removal of paving and removal if indicated of abandoned utilities.
- 2. Sitework (Area of Work), removal of designated fences, walls, and other elements; capping and identifying utilities; landscape paving, and removal of concrete foundations.
- G. Section 31 23 16 Excavation: Site preparation for structure and paving.
- H. Section 31 23 23 Fill: Filling holes, pits, and excavations generated as a result of removal operations.

QUALITY ASSURANCE

A. Clearing Firm: Company specializing in the type of work required. 1. Minimum of five years of documented experience.

ART 2 PRODUCTS

- MATERIALS
- A. Fill Material: As specified in Section 31 23 23 Fill and Backfill

RT 3 EXECUTION

- SITE CLEARING
- A. Comply with other requirements specified in Section 01 70 00.
- LEARING SPECIFICATIONS
 - 3. Keep adjacent roads, streets and drives clear of dirt and debris from earthwork operations.
- C. Underground Utilities:
- 1. Buried utility lines may exist.
- 2. If such are encountered, notify Owner Representative, Architect and District and for directions to be followed for preservation, relocation or demolition of utilities.

PART 2 PRODUCTS

- 2.01 MATERIALS
- A. Topsoil: See Section 31 23 23
- B. Shoring and Bracing: Provide all materials and services necessary to properly engineer and construct shoring for excavations. Selection of materials and design of shoring, underpinning
- and bracing of new and existing structures shall be solely the responsibility of the Contractor. 1. Shoring design shall comply with State of California Trenching and Shoring Manual issued
- by Offices of Structure Construction; 2011.

PART 3 EXECUTION

- 3.01 EXAMINATION
- A. Verify that survey bench mark and intended elevations for the Work are as indicated. B. Verify the absence of standing or ponding water.
- C. Upon discovery of unknown utility or concealed conditions, discontinue affected Work and notify Owner Representative, Architect and District for direction. Unforeseen conditions shall be resolved in accordance with the General Conditions.

3.02 PREPARATION

- A. Identify required lines, levels, contours, and datum.
- B. Stake and flag locations of known utilities.
- C. Locate, identify, and protect from damage above- and below-grade utilities to remain. 1. Maintain and protect existing utilities remaining which pass through Project area.
- D. Notify utility company to remove and relocate utilities, as required.
- E. Provide temporary means and methods to remove all standing or ponding water from areas
- prior to grading F. Protect site features to remain, including but not limited to bench marks, survey control
- s, paving, and curbs, norn damage by grading equipment and vehicular traffic.
- G. Protect trees to remain by providing substantial fencing around entire tree at the outer tips of its branches; no grading is to be performed inside this line.
- H. Protect plants, lawns, and other features to remain as a portion of final landscaping.

- B. Minimize production of dust due to clearing operations; do not use water if that will result in ice, flooding, sedimentation of public waterways or storm sewers, or other pollution.
- 3.02 EXISTING UTILITIES AND BUILT ELEMENTS A. Coordinate work with utility companies; notify before starting work and comply with their requirements; obtain required permits.
- B. Protect existing utilities to remain from damage. C. Do not disrupt public utilities without permit from authority having jurisdiction.
- D. Protect existing structures and other elements that are not to be removed.
- 3.03 CLEARING
- A. Perform clearing Work within confines of Project area indicated on Drawings or specified elsewhere herein and with strict adherence to the Contract Documents and Geotechnical recommendations.
- 3.04 VEGETATION
- A. Scope: Remove trees, shrubs, brush, and stumps in areas to be covered by building structure, paving, lawns, and planting beds. Do not remove or damage vegeta
- C. Install substantial, highly visible fences at least 3 feet high to prevent inadvertent damage to vegetation to remain: 1. At vegetation removal limits.
- D. Remove only trees within area to be cleared that have been marked for removal. Confirm trees to be removed with District and Architect before beginning removal process. 1. Cut trunks close and parallel to ground.
- 2. Remove roots where under or within five feet of proposed structures. 3. Neither remove nor prune trees and shrubbery in public rights-of-way except by written
- approval of authorities having jurisdiction E. In areas where vegetation must be removed but no construction will occur other than
- pervious paving, remove vegetation with minimum disturbance of the subsoil. F. Vegetation Removed: Do not burn, bury, landfill, or leave on site, except as indicated.
- 1. Chip, grind, crush, or shred vegetation for mulching, composting, or other purposes; preference should be given to on-site uses.
- 2. Trees: Sell if marketable; if not, treat as specified for other vegetation removed; remove stumps and roots to depth of 18 inches
- 3. Existing Stumps: Treat as specified for other vegetation removed; remove stumps and roots to depth of 18 inches 4. Sod: Re-use on site if possible; otherwise sell if marketable, and if not, treat as specified
- for other vegetation removed. G. Dead Wood: Remove all dead trees (standing or down), limbs, and dry brush on entire site; treat as specified for vegetation removed.

3.03 ROUGH GRADING

- A. Comply with Geotechnical Report and field directives of geotechnical engineer on-site. B. Remove topsoil from areas to be further excavated, re-landscaped, or re-graded, without mixing with foreign materials.
- 1. Coordinate topsoil with Section 31 10 00 Site Clearing.
- C. Do not remove topsoil when wet. D. Remove subsoil from areas to be further excavated, re-landscaped, or re-graded.
- See Section 31 23 16 Excavation. E. Do not remove wet subsoil, unless it is subsequently processed to obtain optimum moisture
- F. When excavating through roots, perform work by hand and cut roots with sharp axe. G. See Section 31 23 23 for filling procedures.
- H. All permanent cut or fill slopes shall have a maximum slope of 2:1 (H:V) ratio, horizontal to vertical and shall comply with applicable requirements of the Geotechnical Report and California Building Code (CBC).
- I. Benching Slopes: Horizontally bench existing slopes greater than 5:1 (H:V) to key fill material to slope for firm bearing. J. Stability: Replace damaged or displaced subsoil to same requirements as for specified fill.
- K. Remove and replace soils deemed unsuitable by classification and which are excessively moist due to lack surface water control.
- L. Grade top perimeter of excavations to prevent surface water from draining into excavation. 1. Provide dewatering of excavations as required to ensure suitable conditions for concrete and backfilling operations.
- M. Uniformly grade areas as shown on Drawings to tolerances specified in this Section. 1. Evenly grade between points where elevations are shown or between points of Work and existing grades
- N. Slope rough grade away from building perimeter at gradient indicated. 1. Upaved area slope for a distance of 10 feet from the building: Not less than one unit vertical in 20 units horizontal or 5 percent.
- a. CBC Section 1804A.4. 2. When supported by soil conditions and climate; slope not less than 1:48 or 2 percent in unpaved areas.
- a. CBC Section 1804A.4, Exception.
- O. Make grade changes gradual. Blend slopes into level areas.
- 3.04 SOIL REMOVAL AND STOCKPILING
- A. Stockpile topsoil to be re-used on site; remove remainder from site. 1. Topsoil and vegetation layers, root zones, and similar surface materials should be stripped and stockpiled for either reuse in landscape surface areas or removed from the

H. Restoration: If vegetation outside removal limits or within specified protective fences is damaged or destroyed due to subsequent construction operations, replace at no cost to

3.05 GRUBBING

- A. At pipelines, remove all trees or stumps within five feet of the pipeline
- B. Perform grubbing where indicated on Drawings or as specified herein. Grubbing shall include removal from the ground of all stumps, roots, buried logs and other vegetation not otherwise indicated to remain, and removal and disposal of resulting refuse.
- C. Completely grub areas where unsuitable surface material is to be removed. 3.06 DAMAGED VEGETATION
- Neatly prune damaged branches and severed roots.
- B. Apply wound paint to above-ground cuts and abrasions.
- C. If trees and shrubs indicated to remain are damaged excessively, as determined by Owner Representative, Architect or authorities having jurisdiction, remove and replace damaged plants with comparable plants.
- 3.07 DEBRIS
- A. Remove debris, junk, and trash from site.
- B. Remove logs, rocks and other debris. C. Dispose of Debris resulting from clearing and thoroughly clean rights-of-way.
- D. Leave site in clean condition, ready for subsequent work.
- E. Clean up spillage and wind-blown debris from public and private lands.
- 3.08 DISPOSAL A. Debris Disposal: Dispose of all cleared and grubbed materials in a legal manner off site.
- B. Hazardous Materials
- 1. Immediately notify the Owner Representative should hazardous materials or suspected hazardous materials be encountered 2. Dispose of such materials in accordance with all applicable laws and regulations and as
- directed by authorities having jurisdiction.
- 3. Unforeseen conditions will be resolved in accordance with the Conditions of the Contract.
- C. Saleable Materials:
- 1. Unless otherwise indicated, all felled trees from which merchantable lumber or firewood
- can be produced shall become the property of the Contractor. 2. Unless otherwise indicated, all metallic debris of salvageable value shall become the
- property of the Contractor.
- 3. The Contractor shall remove all saleable materials from the site in a timely manner 4. Sale of salvaged and merchantable materials shall be done on site only with prior
- approval of the District.

D. Stockpiling Vegetation: Only if specified or indicated under landscape work, stockpile vegetation for subsequent mulching E. Burial and Burning: Debris shall not be buried or burned on site.

3.09 DUST CONTROL

- A. Refer to requirements of:
- 1. Section 01 50 00 Temporary Construction Facilities and Controls. 2. Section 31 22 00 - Grading.
- B. Minimize dust during clearing and grubbing to protect adjoining property and vehicles parked
- in the vicinity. C. Clean-up: Keep public thoroughfares clear of dust and debris by periodic sweeping and washing down, at least daily at the end of working hours. END OF SECTION

- C. Other Existing Vegetation to Remain: If damaged due to this work, replace with vegetation of equivalent species and size. 3.08 FIELD QUALITY CONTROL
- A. See Section 31 23 23 for compaction density testing.
- B. Field Quality Control:
- 1. Field inspections and testing shall be performed in accordance with requirements specified in Section 01 40 00 and 01 45 33. 2. Make required quality control submittals in accordance with requirements specified.
- C. Non-compliance: Should grade elevations, tests of fill or backfill indicate non-compliance with required elevations or density, Contractor shall over-excavate, recompact and retest until specified grade or density is obtained.
- 1. Costs and Time associated with remedial Work and retesting shall be in accordance with provisions of the General Conditions
- District and shall be at Contractor's expense.

END OF SECTION

G. Place topsoil during dry weather. H. Remove roots, weeds, rocks, and foreign material while spreading.

B. Stockpile subsoil on site for backfill, if soil is appropriate.

1. Stockpile subsoil to depth not exceeding 8 feet.

D. Stockpile subsoil to be re-used on site; remove remainder from site.

2. Verify subgrade has been contoured and compacted.

C. Where topsoil is to be placed, scarify surface to depth of 6 inches.

contaminated with petroleum products.

1. Comply with CBC Section 1804A.3.

F. Place topsoil where required to level finish grade.

- I. Near plants spread topsoil manually to prevent damage.
- J. Fine grade topsoil to eliminate uneven areas and low spots. Maintain profiles and contour of subgrade.

from erosion.

A. Before Finish Grading:

3.05 FINISH GRADING

K. Lightly compact placed topsoil.

E. Place topsoil in areas indicated.

- L. Maintain stability of topsoil during inclement weather. Replace topsoil in areas where surface
- water has eroded thickness below specifications.
- 3.06 TOLERANCES
- A. Top Surface of Subgrade: Plus or minus 0.10 foot (1-3/16 inches) from required elevation. B. Top Surface of Finish Grade: Plus or minus 0.04 foot (1/2 inch).
- C. Top Surface Under Paving: Plus or minus 0.04 foot (1/2 inch) from required elevation.
- D. Top Surface Under Footings and Foundations: Plus 0, minus 0.2 foot (2.4 inch).
- E. Top Surface Under Slabs on Grade: Plus 0, minus 0.04 foot (1/2 inch).
- 3.07 REPAIR AND RESTORATION A. Existing Facilities, Utilities, and Site Features to Remain: If damaged due to this work, repair or
- replace to original condition. B. Trees to Remain: If damaged due to this work, trim broken branches and repair bark wounds;

if root damage has occurred, obtain instructions from Architect as to remedy.

- B. Leave site clean and raked, ready to receive landscaping. 3.10 PROTECTION
- B. Over-excavate and recompact areas damaged by construction activities and weather.
- 3.09 CLEANING
- 2. Retesting to demonstrate compliance shall be by a testing laboratory acceptable to
- - A. Remove unused stockpiled topsoil and subsoil. Grade stockpile area to prevent standing

- A. Protect completed grading from erosion from weather and traffic.

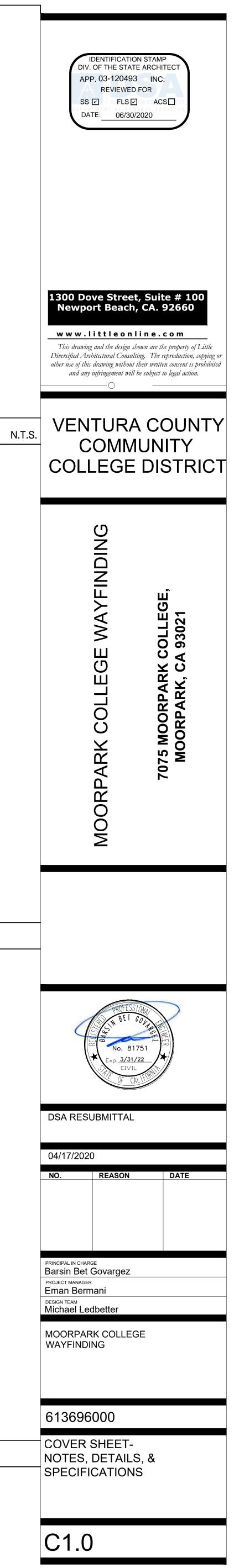
- 1. Verify building and trench backfilling have been inspected.

C. Remove all lumped subsoil, boulders and rock in excess of 6 inches in greatest dimension.

E. Stockpiles: Use areas designated on site; pile depth not to exceed 8 feet; cover to protect

B. Remove debris, roots, branches, stones, in excess of 1/2 inch in size. Remove soil

D. In areas where vehicles or equipment have compacted soil, scarify surface to depth of 6



SECTION 03 30 10	1.3.4. Mix designs: Prepare mix designs for Architect's review. Include the following information in mix design data:		2.2.6. Do NOT use heat to bend bars.	2.4.2.3.2. Perm Rating: Maximum 0.01 grains/square foot/hour ASTM F 1249.
CAST-IN-PLACE CONCRETE	1.3.4.1.1. Design Method, mix number.	2.1.2.3. Class/Face Veneer: Class I or II, B-B Veneer.	2.2.7. Remove and replace reinforcement with following fabrication defects:	ASTM F 1249. 2.4.2.3.3. Water Vapor Transmission Rate: Maximum 0.
	1.3.4.1.2. Specified compressive strength, maximum aggregate size,	2.1.2.4. Panel Finish: Where concrete will be exposed to view in final project, with painted and non-painted finish, provide HDO resin fiber overlay.	2.2.7.1. Bar lengths, depths and bends exceeding specified fabrication tolerances.	grains/square foot/hour per ASTM F 1249.
PART 1 - GENERAL SECTION INCLUDES	slump, and placement method. 1.3.4.1.3. Application and location in structure.	2.1.3. Lumber Forms: Any grade or species, S4S.	2.2.7.2. Bends or kinks not shown on drawings or final shop drawings.2.2.7.3. Bars with reduced cross-section due to excessive rusting or other causes.	2.4.2.3.4. Puncture resistance: Minimum puncture resistance of grams per ASTM D 1709.
1.1.1. Formwork and anchorage.	1.3.4.1.4. Water-Cement Ratio.	2.1.4. Form Ties:	2.2.7.3. Dars with reduced closs-section due to excessive rusting of other causes.2.2.8. Locate reinforcing splices as shown on Drawings. Obtain approval of Structural Engineer	2.4.2.3.5. Tensile Strength: Minimum tensile strength of 50 pound ASTM D 882. Values shall be based on ASTM E
1.1.2. Concrete reinforcement and accessories.	1.3.4.1.5. Cement: Type, amount, and compliance with specified	2.1.4.1. Concealed Condition: Meadow Burke Penta-Tie or equal. Snap-off type, fixed length, cone type, 1 inch back break dimension, free of defects that could	for splices not shown on drawings.	Resistance to Decay test portion.
1.1.3. Cast-in-place concrete.	criteria statement. 1.3.4.1.6. Aggregates: Source(s), gradations (individual and	leave holes larger than one inch in concrete surface; provide flush plugs for cone holes or grout fill as specified.	 2.3. CONCRETE MATERIALS 2.3.1. Cement: Conform to CBC Section 1913A.1, and ASTM C150; normal - Type II, low alkali, 	2.4.2.3.6. Low Temperature Brittleness: Pass per ASTM D 1790.2.4.2.4. Accessories.
1.2. REFERENCES	combined).	2.1.4.2. Exposed Condition: Snap-off type, fixed length, cone type, 1 inch back break dimension, free of defects that could leave holes larger than one inch in	grey color. Type V cement shall be used for concrete in contact with soil.	2.4.2.4. Accessories. 2.4.2.4.1. Provide all required seam tapes and mastics as supplie
1.2.1. ACI 117 - Specification for Tolerances for Concrete Construction and Materials.	1.3.4.1.7. Signature and stamp of licensed civil engineer responsible for mix design.	concrete surface; provide semi-recessed plugs for cone holes.	2.3.2. Fine and Coarse Aggregates: Conform to CBC Section 1903A.6, ASTM C33 and the following:	manufacturer.
 1.2.2. ACI 301 - Specification for Structural Concrete. 1.2.3. ACI 302.1R - Guide for Concrete Floor and Slab Construction. 	1.4. QUALITY ASSURANCE	2.1.5. Form Release Agent: Cresset or equal, colorless, water based material which will not stain concrete, or absorb moisture, or impair natural bonding or color characteristics of coating intended for use on concrete.	2.3.2.1. Coarse Aggregate: Clean, hard, fine-grained, sound, crushed rock or washed	2.4.3. Non-Shrink Grout: Premixed compound consisting of non-metallic aggregate, cer water reducing and plasticizing admixtures, capable of developing a mini compressive strength of 8.000 psi at 28 days when tested in accordance with CRD-C
1.2.4. ACI 305R - Hot Weather Concreting, and ACI 306.1, Cold Weather Concreting.	1.4.1. Comply with applicable portions of referenced ACI 315 and ACI 347 standards for construction of concrete work specified in this Section.	2.1.5.1. Select type suitable and appropriate for achieving CCS 2 surface at exposed	gravel; 2.3.2.1.1. Slabs, Columns, Walls: Class Designation 5M per ASTM C	and ASTM C 1107.
1.2.5. ACI 308 - Standard Practice for Curing Concrete.	1.4.2. Comply with Chapters 7 and 12 of ACI 318 for details of reinforcement and laps at bar	concrete applications.	33, Table 3, with 1 inch grading.	2.4.4. Admixtures: Concrete admixtures shall be subject to prior approval by DSA jurisdictional authority and Architect. Calcium chloride or admixtures containing chl
1.2.6. ACI 318 - Building Code Requirements for Structural Concrete.	splices respectively. 1.5. REGULATORY REQUIREMENTS	2.1.6. Corners: Chamfered, rigid plastic or wood strip type; 3/4 x 3/4 inch size; maximum possible lengths.	2.3.2.1.2. Foundations: Class Designation 3M per ASTM C 33, Table 3, with 1-1/2 inch grading.	shall not be used. Admixture(s) shall not adversely affect concrete strength or col colored concrete, where occurs.
1.2.7. ASTM C 33 - Concrete Aggregates.	1.5.1. Conform to applicable sections of Chapter 19A, Part 2, Title 24, CCR.	2.1.7. Form Stakes: Steel bar stock, pre-drilled for nails.	2.3.2.2. Fine Aggregate: Washed natural or manufactured sand, hard, strong, durable particles: not more than 1 percent deleterious materials.	2.4.5. Polymer Modified Concrete: Provide Sika Armatec 110 polymer modified concrete, 3/8 inch minus aggregate conforming to manufacturers criteria.
1.2.8. ASTM C 94 - Ready-Mixed Concrete.	1.6. PRODUCT HANDLING	2.1.8. Formwork Panel Edge: Provide foam edge stripping at exposed formwork panel edges to minimize mortar leakage.	2.3.2.3. Aggregate shall be non-reactive per ASTM C 289.	2.4.6. Bonding Agent: Provide SikaDur 32 Hi-Mod bonding agent/adhesive conformi
 ASTM C 150 - Portland Cement. ASTM D1752 - Preformed Sponge Rubber and Cork Expansion Joint Fillers. 	1.6.1. On delivery to Project Site, place materials in area protected from weather.	2.2. REINFORCING STEEL	2.3.3. Water: Clean, potable, and not detrimental to concrete.	manufacturers criteria. 2.5. SCREED SYSTEMS AND JOINT FORMING MATERIALS
1.3. SUBMITTALS	1.6.2. Store materials above ground on framework or blocking and cover with protective waterproof covering providing for adequate air circulation and ventilation. Handle	2.2.1. Reinforcing Steel:	2.3.4. Concrete slab-on-grade rock base: Clean, washed crushed rock base, 3/8 inch grading, complying with ASTM C 33.	2.5.1. Screed Systems: Provide Grann Adjustable Quick Screed or equal chairs, avai
1.3.1. Provide submittals under provisions of Division 01.	materials to prevent damage.	2.2.1.1. Non-Welded Systems: ASTM A615, deformed billet steel bars, uncoated.	2.4. ACCESSORIES	through Dayton Richmond (800-745-3700).
1.3.2. Product Data/Materials List: Submit data indicating product standards, physical and chemical characteristics, technical specifications, limitations, maintenance instructions,	2. PART 2 - PRODUCTS	2.2.1.1.1. Bar size #3 and larger: Grade 60.2.2.1.2. Provide reinforcing steel complying with ASTM A 706. Grade 60. deformed.	2.4.1. Membrane Curing Blankets	2.5.2. Formed Construction Joints: Meadow Burke Keyed Kold or equal, galvanized s tongue and groove type.
and general recommendations regarding each material proposed for the work in this Section.	2.1. FORM MATERIALS	uncoated steel, where shown.	2.4.1.1. Provide Whitecap or equal 483-Curelap light colored plastic faced 10 oz. burlap curing blankets complying with ASTM C 171.	2.5.3. Isolation Joint at radiused conditions: W.R. Meadows, <u>www.wrmeadows.com</u> , or ed Ceramar, 3/8 inch thick by full depth of slab.
1.3.3. Test Reports: Submit certified copies of mill test report of reinforcing steel analysis to	2.1.1 Conform to ACI 347.	2.2.2. Dowels: Same as 2.2.1.	2.4.2. Vapor Retarder Membrane	2.5.4. Isolation Joint at perimeter conditions: W.R. Meadows or equal, Sealtight Fiber, 3/8
testing laboratory, indicating products meet or exceed specified requirements.	2.1.2. Softwood Plywood - Vertical and horizontal surfaces.	2.2.3. Chairs, Bolsters, Bar Supports, Spacers: Sized and shaped for strength and support of reinforcement during concrete placement conditions including load bearing pad on bottom to provent upper retarder/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice/partice	2.4.2.1. Manufacturer: Stego Wrap, www.stegoindutries.com, or equal.	thick by full depth of slab.
 Steel Source and Description Ultimate tensile strength, Bend test, Elongation percentage and Yield point. 	2.1.2.1. Grade Certification: APA Grade stamped, complying with PS-1.	to prevent vapor retarder/retarder/barrier puncture.2.2.4. Fabricate concrete reinforcing in accordance with CRSI Manual of Standard Practice, ACI	2.4.2.2. Type: Polyolefin geomembrane film.	2.5.5. Weakened plane/control joints: Provide Soff-Cut system or sawcutting at all slab ar Use of cast-in-place concrete joints is not acceptable.
1.3.3.2. Oltimate tensile strength, Bend test, Elongation percentage and Yield point. 1.3.3.3. Heat number and Chemical analysis.	2.1.2.2. Type: APA Plyform, Exterior Type.	315, and ACI 318.	2.4.2.3. Product Characteristics:	JOINT SEALERS
		2.2.5. Do NOT bend or straighten bars in manner that will weaken or injure bar. Do not re-bend bars #5 and larger.	2.4.2.3.1. Thickness: Minimum 15 mils.	2.5.6. Provide sealants per Section 07 90 00.
03 30 10 - 1	03 30 10 - 2	03 30 10 - 3	03 30 10 - 4	03 30 1
6. CONCRETE MIX	2.7.2. Reinforcing	3.1.2. In the event of discrepancy, immediately notify the Architect.	3.2.10.1. Provide bracing to ensure stability of formwork.	3.2.13.6. Exercise care in placing reinforcing steel and concrete to avoid puncturing
2.6.1. Prepare concrete mix design, stamped and signed by a CA. Professional Engineer and in accordance with ACI 318, Chapter 5, section 1904A.	2.7.2.1. Reinforcing Bars: Section 1913A.2, Chapter 19A,Part 2, Title 24, CCR and AWS D1.4.	3.1.3. Do not proceed with installation in areas of discrepancy until all such discrepancies have been fully resolved.	3.2.10.2. Align form joints.	vapor retarder/barrier membrane. Do not drive stakes through the membrane. Use flat base screed supports.
2.6.1.1. Provide concrete mixes as necessary to attain strengths and characteristics	2.7.2.2. Cost of testing for unidentified stock shall be reimbursed to the Owner by the	3.2. FORMWORK AND REINFORCING	3.2.10.3. Place plywood panels with horizontal joints level, vertical joints plumb.	3.2.14. Place all concrete reinforcing in accordance with CRSI Placing Reinforcing Bars.
as noted on the drawings and in the specifications.2.6.1.2. Concrete strength: minimum compressive strength of 4,000 psi at 28 days	Contractor. 2.7.3. Cement and Aggregate	3.2.1. Erect formwork, shoring and bracing to achieve design requirements, in accordance with requirements of ACI 347.	3.2.10.3.1. Unless noted otherwise, place panel edge at center of surface and extend in equal dimension in each direction.	3.2.15. Before placing, clean reinforcing of loose scale, rust, oil, dirt, and any coating adversely affecting concrete bond.
2.6.1.2. Concrete strength. minimum compressive strength of 4,000 psi at 26 days	2.7.3. Cement: CBC Section 1913A.1, Chapter 19A, Part 2, Title 24, CCR.	3.2.2. Verify lines, levels, and measurement before proceeding with formwork.	3.2.10.3.2. Set form tie so that visible cones are placed in a uniform and aligned pattern. Maintain cones level and vertically aligned.	3.2.16. Repair vapor retarder/barrier damaged during placement of concrete reinforcing. Repair with same material; lap over damaged areas minimum 6 inches and seal watertight with
according to indicated concrete strength.	2.7.3.2. Aggregate: CBC Section 1903A.4, Chapter 19A, Part 2, Title 24, CCR.	3.2.3. Do not apply form release agent where concrete surfaces receive special finishes or	3.2.10.4. Keep form joints to a minimum. Use maximum size panels.	manufacturer's approved seam tape.
2.6.1.4. Mix concrete in accordance with ASTM C942.6.1.5. Deliver to Inspector on site, with each mixer load of concrete, certificate	2.7.3.3. Batch Plant Inspection: CBC Section 1705A.3.2, Chapter 19A, Part 2, Title 24, CCR and criteria specified in this Section.	applied coatings which may be affected by agent.3.2.4. Coordinate work of other Sections in forming and setting openings, slots, recesses,	3.2.10.5. Back all joints by a stud or solid blocking, and provide shaped filler where necessary for smoothness. Provide foam form edge striping to prevent grout	3.2.17. Place, support and secure reinforcement against displacement. Do not deviate from required position. Do not bend or straighten bars after placement.
bearing signature of the Weighmaster and Testing Laboratory batch plant inspector stating quantities of each material contained in load and time mixer	2.8. OTHER MATERIALS	chases, sleeves, bolts, anchors, and other inserts.	washout.	3.2.18 . Do not displace or damage vapor retarder /retarder/barrier.
was loaded.	2.8.1. Provide all other materials, not specifically described but required for complete and proper installation of the wark on calendary by the	3.2.5. Locate and set in place items which will be cast directly into concrete.	3.2.10.6. Reused panels shall be thoroughly cleaned, damaged edges or surfaces repaired, and both sides and edges coated with specified material.	3.2.19. Accommodate placement of formed openings. Maintain concrete cover around reinforcing as indicated.
2.6.1.6. Delivery Requirements:2.6.1.6.1. Licensed Weighmaster shall positively identify materials as	installation of this work, as selected by the contractor and subject to the approval of the Architect.	3.2.5.1. Set all anchor bolts, hold downs and related embeds with plywood templates, anchored to formwork as required to maintain in alignment and position during concrete placement.	3.2.10.7. Nail plywood along edges and to intermediate supports with common wire nails spaced as necessary to maintain alignment and prevent warping.	3.2.20. Provide dowel joints at concrete joints as shown on drawings.
to quantity and certify each load by ticket.		3.2.6. Arrange and assemble formwork to permit dismantling and stripping. Do not damage	3.2.11. Apply form release agent on formwork in accordance with manufacturer's	3.2.21. Prepare previously placed concrete by cleaning with steel brush and applying bonding agent in accordance with manufacturer's instructions. Before concrete is deposited upon
2.6.1.6.2. Ticket shall be transmitted to Project Inspector by truck driver.	3. PART 3 - EXECUTION	concrete during stripping. Provide crush plates or other approved guards where stripping operation may damage concrete. Kerf wood inserts to permit easy removal.	recommendations. Apply prior to placement of reinforcing steel, anchoring devices, and embedded items.	or against concrete that has taken its initial set or has hardened, mechanically roughen hardened concrete to minimum 1/4" amplitude. Remove all encrustations from forms and
2.6.1.6.3. Project Inspector shall keep daily record of pours, identify each truck, its load and time of receipt and transmit duplicate	3.1. SURFACE CONDITIONS 3.1.1. Inspection	3.2.7. Chamfer exposed corners. Seal Joints between chamfer and form panel. Miter chamfer strips at changes in direction.	3.2.11.1. Do not apply form release agent where concrete surfaces will receive special finishes or applied coverings which are effected by agent. Soak inside	reinforcements. 3.2.22. Screed Placement and Leveling:
copy of record to DSA.	3.1.1.1. Prior to work of this section, carefully inspect previously installed work. Verify	3.2.8. Openings in structural members which are not indicated on Drawing are not permitted.	surfaces of untreated forms with clean water. Keep surfaces wet prior to placement of concrete.	3.2.22.1. Space screeds at manufacturers recommended spacing.
2.6.1.6.4. Concrete arriving at Work without Weighmaster ticket will be rejected.	all such work is complete to the point where this installation may properly commence.	3.2.9. Foundation Formwork	3.2.12. Do not remove forms or bracing until concrete has gained sufficient strength to carry its	3.2.22.2. Space screed for strip pours.
2.6.1.7. Representative of Testing Laboratory shall maintain continuous inspection of batch plant preparation of concrete, including review of aggregate and cement	3.1.1.2. Verify that work of this section may be installed in strict accordance with the original design, all pertinent codes and regulations, and all pertinent portions	3.2.9.1. Hand trim sides and bottom of earth forms; remove loose dirt.	own weight and imposed loads, and in accordance with the following: 3.2.12.1. Loosen forms carefully. Do not wedge pry bars, hammers, or tools against	3.2.22.3. Level screeds by use of laser level equipment to specified slab elevation.
loading and mixing procedures, and final quantities contained in each truck load.	of the referenced standards.	3.2.9.2. Construct wood edge forms, as specified below, to extend not less than 2 inches below soil level. Do not permit stakes to extend into or through footing	finish concrete surfaces scheduled for exposure to view.	3.3. PLACING CONCRETE
2.6.1.7.1. Where batch plant is certified as conforming to quality control	3.1.1.3. Verify all excavations have been inspected and approved by the Geotechnical Engineer. Verify all reinforcement and forms have been inspected and	zone. Form all concrete without penetrating footing concrete.	3.2.12.2. Store removed forms in manner that surfaces to be in contact with fresh concrete will not be damaged. Discard damaged forms.	3.3.1. Place concrete in accordance with ACI 304
and equipment criteria defined by CBC Section 1705A.3.3 Chapter 17A, Part 2, Title 24, CCR, batch plant inspection may be waived following acceptance of such certification by	approved. 3.1.1.4. Verify concrete elevations, dimensions, and alignment with work specified in	3.2.9.3. Fill over-excavated footings and foundations with concrete at no additional contract cost.	3.2.13. Vapor retarder/barrier membrane installation	3.3.2. Ready mix concrete shall be delivered in accordance with ASTM C94. Concrete shall be placed within 90 minutes after start of mixing.
may be waived, following acceptance of such certification by DSA.	3.1.1.4. Verify concrete elevations, dimensions, and alignment with work specified in other sections.	3.2.9.4. Excavate as necessary to accommodate installation and removal of formwork.	3.2.13.1. Proof roll subgrade.	3.3.3. Conform to ACI 305R when concreting during hot weather or when weather conditions may cause rapid evaporation of moisture. Conform to ACI 306R for concrete placement
2.6.1.7.2. In the absence of such certification, batch plant inspection may be waived for concrete used on single story wood	3.1.1.5. Verify requirements for concrete cover over reinforcement.	3.2.9.5. Prior to pouring footings or foundations, remove all debris, loose material, and	3.2.13.2. Place 4 inch crushed rock base over sub-grade.	in cold weather conditions.
framed structures when criteria defined in CBC Section 1705A.3.3, Chapter 17A, Part 2, Title 24, CCR are met.	3.1.1.6. Identify, verify, and coordinate placement of piping and conduit sleeves through concrete.	water from excavation. Where water has accumulated in excavation, obtain Architects and Geotechnical Engineers review of the suitability of sub-grade	3.2.13.3. Place vapor retarder/barrier membrane over rock base, lapping edges 12 inches. Tape and seal edges and penetrations. Extend membrane to footing	3.3.4. Ensure reinforcement, inserts, embedded parts, formed joint fillers, and joint devices are not disturbed during concrete placement.
Comply with initial batch plant inspection and criteria specified in paragraph 2.6.2.2 of this Section.	3.1.1.7. Identify, verify, and coordinate the location, dimension, and requirements of all depressions, recesses, block-outs and other provisions.	condition. 3.2.9.6. Do not place concrete on mud or saturated soils. Repair sub-grade as	face and turn down to bottom of footing. 3.2.13.4. Provide pipe "boots" at all pipe and conduit penetrations in accordance with	3.3.5. Maintain records of concrete placement. Record date, location, quantity, air temperature, and test samples taken.
SOURCE QUALITY CONTROL AND TESTS	3.1.1.8. Verify anchors, seats, plates, reinforcement and other items embedded in	3.2.9.6. Do not place concrete on mud or saturated soils. Repair sub-grade as required by Geotechnical Engineer at no additional contract cost.	manufactures recommendations.	and test samples taken.3.3.6. Place concrete continuously between predetermined expansion, control, and construction
2.7.1. Provide for testing under the provisions of Section 01 45 00.	concrete are accurately placed, positioned securely, and will permit proper concrete placement.	3.2.10. Wall and structure formwork	3.2.13.5. Obtain inspectors approval of membrane installation before placing concrete.	joints.
03 30 10 - 6	03 30 10 - 7	03 30 10 - 8	03 30 10 - 9	03 30 10 - 10
3.3.6.1. Place construction joints only at locations shown on drawings or as approved	3.5.1.2. Control joint [saw cut] spacing shall generally not exceed 5-ft in any direction.	3.8.2. Classification shall be per General Building: Cast-in-Place, ACI 117, Section 4.0, unless	3.11. EQUIPMENT BASES	
by Architect.	3.5.2. Place formed construction joints in floor slab. Set top screed to required elevations. Secure	noted otherwise.	3.11.1. Provide concrete bases and anchorage for mechanical, electrical, and other work as	
3.3.6.2. Install construction joints as indicated on drawings3.3.6.3. Once concrete operation has begun, it shall be continued until the specific	to resist movement of wet concrete.3.5.3. Install isolation /expansion joints with sealant between slab edges and vertical structural	3.8.3. Unless noted otherwise, depressions in slab floors between high spots shall be a maximum 3/16 inch in ten feet, using a metal straight edge placed at any location on slab, and measured within 72 hours of pour.	required and shown on the drawings and in accordance with reviewed Shop Drawings of related trades.	
panel, component, or section is complete.	elements.		3.12. MISCELLANEOUS CONCRETE WORK	
3.3.7. Thoroughly consolidate concrete during placement using mechanical vibrators. Do not allow vibrators to contact forms or reinforcing.	 3.5.4. Install sealants in accordance with Division 07. 3.5.5. Provide construction joints or equal weakened plane joints at locations shown on 	3.9. PATCHING	3.12.1. Provide areaways, cast-in-place valve boxes, pits, splash blocks, bases, and other miscellaneous concrete as shown and required to complete the Work. Conform to applicable requirements as specified in this section.	
3.3.8. Screed floors and slabs on grade level or slope to drain as noted on drawings.	3.5.5. Provide construction joints or equal weakened plane joints at locations shown on drawings.	3.9.1. Exposed formed concrete surfaces, both interior and exterior, including surfaces designated to receive painted finish, shall provide surfaces suitable for subsequent	applicable requirements as specified in this section.	
3.4. CONCRETE FINISHING	3.5.5.1. Saw cut slab before random shrinkage cracks form, and as soon as slab is firm enough to not be damaged by saw blade. Complete sawcutting within 12 bours of pour	finishing, free from imperfect joints, fins, "honeycombing", air pockets or "bug" holes, or other such imperfections.	END OF SECTION	
3.4.1. Slab Finish3.4.1.1. Produce hard and impervious surfaces, free from defects and blemishes.	hours of pour. 3.6. CURING AND PROTECTION	3.9.2. Remove rough spots, stains and hardened mortar or grout from intended smooth surfaces by rubbing such surfaces lightly with fine Carborundum stone. Use liberal amount of water		
3.4.1.2. Provide steel troweled finish, Class 3, per ACI 302.1R. Steel troweling shall	3.6.1. Maintain concrete above 50 degrees F and in a thoroughly moist condition for at least the	and rub sufficiently to remove defects without changing texture of concrete.		
consist of three separate operations. Obtain Architect's approval of finish prior to proceeding.	first 7 days after placing concrete. 3.6.2. Floor Surface Curing	3.9.3. Filling Snap Tie Cone Holes:3.9.3.1. Break off tie rods at bottom of cone holes.		
3.4.1.3. At all corridors, utility areas, and similar surfaces not receiving subsequent finish, provide medium swirl texture.	3.6.2. Floor Surface Curing3.6.2.1. Cure floor surfaces in accordance with ACI 308.	3.9.3.1. Break off tie rods at bottom of cone noies.3.9.3.2. Concealed Applications: Flush hole with water, and allow to dry. Coat entire		
Slabs receiving thin set ceramic tile, waterproofing membranes or traffic	3.6.2.2. Curing Blanket Placement: Install blankets immediately after finishing and	inner surface of cone hole with liquid bonding agent, then grout holes solid with approved cement grout and grind smooth.		
topping; provide steel trowel and very light broom finish.3.4.2. Exterior Slab Floors at service areas:	joint placement is completed. Place over all surfaces, including face of footings and depressions. Anchor as required to maintain in place for a period of 7 days.	3.9.3.3. Exposed Applications: Flush hole with water, and allow to dry. Coat entire inner surface of cone hole with liquid bonding agent. Insert semi-recessed		
3.4.2.1. Exterior Service/Utility Concrete Slabs: Provide steel trowel and light broom	3.6.2.3. Membrane sealing Compounds: Apply in accordance with manufacturer's	plug with approved cement.		
finish. Broom finish shall be placed in a pattern as directed by the Architect. 3.4.3. All Other Surfaces	instructions.	3.10. DEFECTIVE CONCRETE 3.10.1 Concrete will be considered defective if strength characteristics indicated by tests of		
3.4.3. All Other Surfaces3.4.3.1. All surfaces shall be as-cast, subject to repair of surface deficiencies as	3.7. FIELD QUALITY CONTROL3.7.1. Field inspection and testing will be performed in accordance with provisions of code	3.10.1. Concrete will be considered defective if strength characteristics indicated by tests of molded cylinders and core tests fall below the minimum 28-day strengths specified or indicated. Replace or adequately strengthen such defective concrete in a manner		
specified.	3.7.2. Provide free access to Work and cooperate with appointed firm.	acceptable to the Architect and Structural Engineer.		
3.4.4. Surface Defects3.4.4.1. Surface defects shall be as defined in ACI 309.2R.	3.7.3. Comply with requirements of CBC Section 1905A.1.2 regarding frequency of testing for concrete test specimens.	3.10.2. Concrete will be considered defective if any one of the following conditions occurs:3.10.2.1. Any concrete work not formed as indicated or is not in conformance with		
3.4.4.2. Surface irregularities shall be as defined in ACI 347 for Class B surfaces for	3.7.4. One slump test will be taken for each set of test cylinders taken.	specified tolerances.		
semi exposed surfaces, and Class A surfaces at all exposed to view conditions.	3.7.4.1. Prepare concrete sample(s) for each type of concrete placed each day.	3.10.2.2. Any concrete with voids or honeycomb that has been cut, resurfaced or filled, unless under the direction of the Structural Engineer.		
3.4.4.3. All surface defects shall be repaired per approved methods and as specified.	3.7.4.2. Prepare one sample for each 50 cubic yards or fraction thereof.	3.10.2.3. Any concrete with sawdust, shavings, wood, or embedded debris.		

SECTION 03 30 10	1.3.4. Mix designs: Prepare mix designs for Architect's review. Include the following information in mix design data:	2.1.2.3. Class/Face Veneer: Class I or II, B-B Veneer.	2.2.6. Do NOT use heat to bend bars.2.2.7. Remove and replace reinforcement with following fabrication defects:	2.4.2.3.2. Perm Rating: Maximum 0.01 grains/square foot/ho ASTM F 1249.
CAST-IN-PLACE CONCRETE	1.3.4.1.1. Design Method, mix number.	2.1.2.4. Panel Finish: Where concrete will be exposed to view in final project, with	2.2.7.1. Bar lengths, depths and bends exceeding specified fabrication tolerances.	2.4.2.3.3. Water Vapor Transmission Rate: Maximum grains/square foot/hour per ASTM F 1249.
1. PART 1 - GENERAL	1.3.4.1.2. Specified compressive strength, maximum aggregate size, slump, and placement method.	painted and non-painted finish, provide HDO resin fiber overlay. 2.1.3. Lumber Forms: Any grade or species, S4S.	2.2.7.2. Bends or kinks not shown on drawings or final shop drawings.	2.4.2.3.4. Puncture resistance: Minimum puncture resistance grams per ASTM D 1709.
1.1. SECTION INCLUDES	1.3.4.1.3. Application and location in structure.	2.1.3. Eulinder Forms. Any grade of species, 343.	2.2.7.3. Bars with reduced cross-section due to excessive rusting or other causes.	2.4.2.3.5. Tensile Strength: Minimum tensile strength of 50 pc
1.1.1. Formwork and anchorage.	1.3.4.1.4. Water-Cement Ratio.	2.1.4.1. Concealed Condition: Meadow Burke Penta-Tie or equal. Snap-off type, fixed	2.2.8. Locate reinforcing splices as shown on Drawings. Obtain approval of Structural Engineer for splices not shown on drawings.	ASTM D 882. Values shall be based on ASTM Resistance to Decay test portion.
 Concrete reinforcement and accessories. Cast-in-place concrete. 	1.3.4.1.5. Cement: Type, amount, and compliance with specified criteria statement.	length, cone type, 1 inch back break dimension, free of defects that could leave holes larger than one inch in concrete surface; provide flush plugs for	2.3. CONCRETE MATERIALS	2.4.2.3.6. Low Temperature Brittleness: Pass per ASTM D 17
1.1.3. Cast-in-place concrete.	1.3.4.1.6. Aggregates: Source(s), gradations (individual and combined).	cone holes or grout fill as specified.2.1.4.2. Exposed Condition: Snap-off type, fixed length, cone type, 1 inch back break	2.3.1. Cement: Conform to CBC Section 1913A.1, and ASTM C150; normal - Type II, low alkali, grey color. Type V cement shall be used for concrete in contact with soil.	2.4.2.4. Accessories.
1.2.1. ACI 117 - Specification for Tolerances for Concrete Construction and Materials.	1.3.4.1.7. Signature and stamp of licensed civil engineer responsible	dimension, free of defects that could leave holes larger than one inch in concrete surface; provide semi-recessed plugs for cone holes.	2.3.2. Fine and Coarse Aggregates: Conform to CBC Section 1903A.6, ASTM C33 and the	2.4.2.4.1. Provide all required seam tapes and mastics as supmanufacturer.
1.2.2. ACI 301 - Specification for Structural Concrete.	for mix design.	2.1.5. Form Release Agent: Cresset or equal, colorless, water based material which will not stain	following:	2.4.3. Non-Shrink Grout: Premixed compound consisting of non-metallic aggregate,
1.2.3. ACI 302.1R - Guide for Concrete Floor and Slab Construction.	 1.4. QUALITY ASSURANCE 1.4.1. Comply with applicable portions of referenced ACI 315 and ACI 347 standards for 	concrete, or absorb moisture, or impair natural bonding or color characteristics of coating intended for use on concrete.	2.3.2.1. Coarse Aggregate: Clean, hard, fine-grained, sound, crushed rock or washed gravel;	water reducing and plasticizing admixtures, capable of developing a compressive strength of 8,000 psi at 28 days when tested in accordance with CF and ASTM C 1107.
1.2.4. ACI 305R - Hot Weather Concreting, and ACI 306.1, Cold Weather Concreting.	construction of concrete work specified in this Section.	2.1.5.1. Select type suitable and appropriate for achieving CCS 2 surface at exposed concrete applications.	2.3.2.1.1. Slabs, Columns, Walls: Class Designation 5M per ASTM C 33, Table 3, with 1 inch grading.	2.4.4. Admixtures: Concrete admixtures shall be subject to prior approval by
1.2.5. ACI 308 - Standard Practice for Curing Concrete.	1.4.2. Comply with Chapters 7 and 12 of ACI 318 for details of reinforcement and laps at bar splices respectively.	2.1.6. Corners: Chamfered, rigid plastic or wood strip type; 3/4 x 3/4 inch size; maximum possible	2.3.2.1.2. Foundations: Class Designation 3M per ASTM C 33, Table	jurisdictional authority and Architect. Calcium chloride or admixtures containing shall not be used. Admixture(s) shall not adversely affect concrete strength or
 1.2.6. ACI 318 - Building Code Requirements for Structural Concrete. 1.2.7. ASTM C 33 - Concrete Aggregates. 	1.5. REGULATORY REQUIREMENTS	lengths.2.1.7. Form Stakes: Steel bar stock, pre-drilled for nails.	3, with 1-1/2 inch grading.2.3.2.2. Fine Aggregate: Washed natural or manufactured sand, hard, strong, durable	colored concrete, where occurs.2.4.5. Polymer Modified Concrete: Provide Sika Armatec 110 polymer modified concrete
1.2.8. ASTM C 94 - Ready-Mixed Concrete.	1.5.1. Conform to applicable sections of Chapter 19A, Part 2, Title 24, CCR.	2.1.7. Formwork Panel Edge: Provide foam edge stripping at exposed formwork panel edges to	particles: not more than 1 percent deleterious materials.	3/8 inch minus aggregate conforming to manufacturers criteria.
1.2.9. ASTM C 150 - Portland Cement.	1.6. PRODUCT HANDLING	minimize mortar leakage.	2.3.2.3. Aggregate shall be non-reactive per ASTM C 289.	2.4.6. Bonding Agent: Provide SikaDur 32 Hi-Mod bonding agent/adhesive conf manufacturers criteria.
1.2.10. ASTM D1752 - Preformed Sponge Rubber and Cork Expansion Joint Fillers.	1.6.1. On delivery to Project Site, place materials in area protected from weather.1.6.2. Store materials above ground on framework or blocking and cover with protective	2.2. REINFORCING STEEL 2.2.1. Reinforcing Steel:	2.3.3. Water: Clean, potable, and not detrimental to concrete.2.3.4. Concrete slab-on-grade rock base: Clean, washed crushed rock base, 3/8 inch grading,	2.5. SCREED SYSTEMS AND JOINT FORMING MATERIALS
1.3. SUBMITTALS	waterproof covering providing for adequate air circulation and ventilation. Handle materials to prevent damage.	 2.2.1. Reinforcing Steel. 2.2.1.1. Non-Welded Systems: ASTM A615. deformed billet steel bars. uncoated. 	complying with ASTM C 33.	2.5.1. Screed Systems: Provide Grann Adjustable Quick Screed or equal chairs, through Dayton Richmond (800-745-3700).
1.3.1. Provide submittals under provisions of Division 01.		2.2.1.1.1. Bar size #3 and larger: Grade 60.	2.4. ACCESSORIES	2.5.2. Formed Construction Joints: Meadow Burke Keyed Kold or equal, galvaniz
1.3.2. Product Data/Materials List: Submit data indicating product standards, physical and chemical characteristics, technical specifications, limitations, maintenance instructions,	2. PART 2 - PRODUCTS	2.2.1.2. Provide reinforcing steel complying with ASTM A 706, Grade 60, deformed,	2.4.1. Membrane Curing Blankets	tongue and groove type.
and general recommendations regarding each material proposed for the work in this Section.	2.1. FORM MATERIALS	uncoated steel, where shown. 2.2.2. Dowels: Same as 2.2.1.	2.4.1.1. Provide Whitecap or equal 483-Curelap light colored plastic faced 10 oz. burlap curing blankets complying with ASTM C 171.	2.5.3. Isolation Joint at radiused conditions: W.R. Meadows, <u>www.wrmeadows.com</u> , Ceramar, 3/8 inch thick by full depth of slab.
1.3.3. Test Reports: Submit certified copies of mill test report of reinforcing steel analysis to testing laboratory, indicating products meet or exceed specified requirements.	2.1.1. Conform to ACI 347.	2.2.2. Dowels: Same as 2.2.1.2.2.3. Chairs, Bolsters, Bar Supports, Spacers: Sized and shaped for strength and support of	2.4.2. Vapor Retarder Membrane	2.5.4. Isolation Joint at perimeter conditions: W.R. Meadows or equal, Sealtight Fiber thick by full depth of slab.
1.3.3.1. Steel Source and Description	2.1.2. Softwood Plywood - Vertical and horizontal surfaces.	reinforcement during concrete placement conditions including load bearing pad on bottom to prevent vapor retarder/retarder/barrier puncture.	2.4.2.1. Manufacturer: Stego Wrap, www.stegoindutries.com, or equal.	2.5.5. Weakened plane/control joints: Provide Soff-Cut system or sawcutting at all sl
1.3.3.2. Ultimate tensile strength, Bend test, Elongation percentage and Yield point.	2.1.2.1. Grade Certification: APA Grade stamped, complying with PS-1.	2.2.4. Fabricate concrete reinforcing in accordance with CRSI Manual of Standard Practice, ACI	2.4.2.2. Type: Polyolefin geomembrane film.	Use of cast-in-place concrete joints is not acceptable.
1.3.3.3. Heat number and Chemical analysis.	2.1.2.2. Type: APA Plyform, Exterior Type.	315, and ACI 318.	2.4.2.3. Product Characteristics:	JOINT SEALERS
		2.2.5. Do NOT bend or straighten bars in manner that will weaken or injure bar. Do not re-bend bars #5 and larger.	2.4.2.3.1. Thickness: Minimum 15 mils.	2.5.6. Provide sealants per Section 07 90 00.
03 30 10 - 1	03 30 10 - 2	03 30 10 - 3	03 30 10 - 4	03
S. CONCRETE MIX	2.7.2. Reinforcing	3.1.2. In the event of discrepancy, immediately notify the Architect.	3.2.10.1. Provide bracing to ensure stability of formwork.	3.2.13.6. Exercise care in placing reinforcing steel and concrete to avoid puncturing
2.6.1. Prepare concrete mix design, stamped and signed by a CA. Professional Engineer and	2.7.2. Reinforcing 2.7.2.1. Reinforcing Bars: Section 1913A.2, Chapter 19A,Part 2, Title 24, CCR and	3.1.2. In the event of discrepancy, immediately notify the Architect.3.1.3. Do not proceed with installation in areas of discrepancy until all such discrepancies have	3.2.10.1. Provide bracing to ensure stability of formwork.3.2.10.2. Align form joints.	3.2.13.6. Exercise care in placing reinforcing steel and concrete to avoid punctur vapor retarder/barrier membrane. Do not drive stakes through membrane. Use flat base screed supports.
in accordance with ACI 318, Chapter 5, section 1904A.	AWS D1.4.	been fully resolved.	3.2.10.3. Place plywood panels with horizontal joints level, vertical joints plumb.	3.2.14. Place all concrete reinforcing in accordance with CRSI Placing Reinforcing Bars.
2.6.1.1. Provide concrete mixes as necessary to attain strengths and characteristics as noted on the drawings and in the specifications.	2.7.2.2. Cost of testing for unidentified stock shall be reimbursed to the Owner by the Contractor.	3.2. FORMWORK AND REINFORCING	3.2.10.3.1. Unless noted otherwise, place panel edge at center of	3.2.15. Before placing, clean reinforcing of loose scale, rust, oil, dirt, and any coating advers
2.6.1.2. Concrete strength: minimum compressive strength of 4,000 psi at 28 days	2.7.3. Cement and Aggregate	3.2.1. Erect formwork, shoring and bracing to achieve design requirements, in accordance with requirements of ACI 347.	surface and extend in equal dimension in each direction.	affecting concrete bond.
2.6.1.3. Maximum Water-Cement ratio: Maximum 0.5 at point of placement, or according to indicated concrete strength.	2.7.3.1. Cement: CBC Section 1913A.1, Chapter 19A, Part 2, Title 24, CCR.	3.2.2. Verify lines, levels, and measurement before proceeding with formwork.	3.2.10.3.2. Set form tie so that visible cones are placed in a uniform and aligned pattern. Maintain cones level and vertically aligned.	3.2.16. Repair vapor retarder/barrier damaged during placement of concrete reinforcing. Re with same material; lap over damaged areas minimum 6 inches and seal watertight manufacturer's approved seam tape.
2.6.1.4. Mix concrete in accordance with ASTM C94	2.7.3.2. Aggregate: CBC Section 1903A.4, Chapter 19A, Part 2, Title 24, CCR.	3.2.3. Do not apply form release agent where concrete surfaces receive special finishes or applied coatings which may be affected by agent.	3.2.10.4. Keep form joints to a minimum. Use maximum size panels.	3.2.17. Place, support and secure reinforcement against displacement. Do not deviate fi
2.6.1.5. Deliver to Inspector on site, with each mixer load of concrete, certificate	2.7.3.3. Batch Plant Inspection: CBC Section 1705A.3.2, Chapter 19A, Part 2, Title 24, CCR and criteria specified in this Section.	3.2.4. Coordinate work of other Sections in forming and setting openings, slots, recesses,	3.2.10.5. Back all joints by a stud or solid blocking, and provide shaped filler where necessary for smoothness. Provide foam form edge striping to prevent grout	required position. Do not bend or straighten bars after placement.
bearing signature of the Weighmaster and Testing Laboratory batch plant inspector stating quantities of each material contained in load and time mixer was loaded	2.8. OTHER MATERIALS	chases, sleeves, bolts, anchors, and other inserts.	washout.	 3.2.18. Do not displace or damage vapor retarder /retarder/barrier. 3.2.19. Accommodate placement of formed openings. Maintain concrete cover around reinforce
was loaded.2.6.1.6. Delivery Requirements:	2.8.1. Provide all other materials, not specifically described but required for complete and proper installation of this work, as selected by the contractor and subject to the approval of the	 3.2.5. Locate and set in place items which will be cast directly into concrete. 3.2.5.1. Set all anchor bolts, hold downs and related embeds with plywood templates, 	3.2.10.6. Reused panels shall be thoroughly cleaned, damaged edges or surfaces repaired, and both sides and edges coated with specified material.	3.2.19. Accommodate placement of formed openings. Maintain concrete cover around reinforci as indicated.
2.6.1.6.1. Licensed Weighmaster shall positively identify materials as	Architect.	3.2.5.1. Set all anchor bolts, hold downs and related embeds with plywood templates, anchored to formwork as required to maintain in alignment and position during concrete placement.	3.2.10.7. Nail plywood along edges and to intermediate supports with common wire nails spaced as necessary to maintain alignment and prevent warping.	3.2.20. Provide dowel joints at concrete joints as shown on drawings.
to quantity and certify each load by ticket.		3.2.6. Arrange and assemble formwork to permit dismantling and stripping. Do not damage	3.2.11. Apply form release agent on formwork in accordance with manufacturer's	3.2.21. Prepare previously placed concrete by cleaning with steel brush and applying bond agent in accordance with manufacturer's instructions. Before concrete is deposited up
2.6.1.6.2. Ticket shall be transmitted to Project Inspector by truck driver.	3. PART 3 - EXECUTION	concrete during stripping. Provide crush plates or other approved guards where stripping operation may damage concrete. Kerf wood inserts to permit easy removal.	recommendations. Apply prior to placement of reinforcing steel, anchoring devices, and embedded items.	or against concrete that has taken its initial set or has hardened, mechanically rough hardened concrete to minimum 1/4" amplitude. Remove all encrustations from forms
2.6.1.6.3. Project Inspector shall keep daily record of pours, identify each truck, its load and time of receipt and transmit duplicate	3.1. SURFACE CONDITIONS 3.1.1. Inspection	3.2.7. Chamfer exposed corners. Seal Joints between chamfer and form panel. Miter chamfer strips at changes in direction.	3.2.11.1. Do not apply form release agent where concrete surfaces will receive special finishes or applied coverings which are effected by agent. Soak inside	reinforcements. 3.2.22. Screed Placement and Leveling:
copy of record to DSA.	3.1.1. Inspection 3.1.1.1. Prior to work of this section, carefully inspect previously installed work. Verify	3.2.8. Openings in structural members which are not indicated on Drawing are not permitted.	surfaces of untreated forms with clean water. Keep surfaces wet prior to placement of concrete.	3.2.22. Screed Placement and Leveling:3.2.22.1. Space screeds at manufacturers recommended spacing.
2.6.1.6.4. Concrete arriving at Work without Weighmaster ticket will be rejected.	all such work is complete to the point where this installation may properly commence.	3.2.9. Foundation Formwork	3.2.12. Do not remove forms or bracing until concrete has gained sufficient strength to carry its	3.2.22.1. Space screed for strip pours.
2.6.1.7. Representative of Testing Laboratory shall maintain continuous inspection of	3.1.1.2. Verify that work of this section may be installed in strict accordance with the	3.2.9.1. Hand trim sides and bottom of earth forms; remove loose dirt.	own weight and imposed loads, and in accordance with the following:	3.2.22.3. Level screeds by use of laser level equipment to specified slab elevation
batch plant preparation of concrete, including review of aggregate and cement loading and mixing procedures, and final quantities contained in each truck load	original design, all pertinent codes and regulations, and all pertinent portions of the referenced standards.	3.2.9.2. Construct wood edge forms, as specified below, to extend not less than 2 inches below soil level. Do not permit stakes to extend into or through footing	3.2.12.1. Loosen forms carefully. Do not wedge pry bars, hammers, or tools against finish concrete surfaces scheduled for exposure to view.	3.3. PLACING CONCRETE
load. 2.6.1.7.1. Where batch plant is certified as conforming to quality control	3.1.1.3. Verify all excavations have been inspected and approved by the Geotechnical Engineer. Verify all reinforcement and forms have been inspected and	inches below soil level. Do not permit stakes to extend into or through footing zone. Form all concrete without penetrating footing concrete.	3.2.12.2. Store removed forms in manner that surfaces to be in contact with fresh concrete will not be damaged. Discard damaged forms.	3.3.1. Place concrete in accordance with ACI 304
2.6.1.7.1. Where batch plant is certified as conforming to quality control and equipment criteria defined by CBC Section 1705A.3.3 Chapter 17A, Part 2, Title 24, CCR, batch plant inspection	Engineer. Verify all reinforcement and forms have been inspected and approved.	3.2.9.3. Fill over-excavated footings and foundations with concrete at no additional contract cost.	concrete will not be damaged. Discard damaged forms. 3.2.13. Vapor retarder/barrier membrane installation	3.3.2. Ready mix concrete shall be delivered in accordance with ASTM C94. Concrete sha placed within 90 minutes after start of mixing.
may be waived, following acceptance of such certification by DSA.	3.1.1.4. Verify concrete elevations, dimensions, and alignment with work specified in other sections.	3.2.9.4. Excavate as necessary to accommodate installation and removal of	3.2.13. Vapor retarder/barner membrane installation 3.2.13.1. Proof roll subgrade.	3.3.3. Conform to ACI 305R when concreting during hot weather or when weather condit
2.6.1.7.2. In the absence of such certification, batch plant inspection	3.1.1.5. Verify requirements for concrete cover over reinforcement.	formwork.	3.2.13.2. Place 4 inch crushed rock base over sub-grade.	may cause rapid evaporation of moisture. Conform to ACI 306R for concrete placen in cold weather conditions.
may be waived for concrete used on single story wood framed structures when criteria defined in CBC Section 17054 3.2. Chapter 174, Bart 3. Title 34, CCB are met	3.1.1.6. Identify, verify, and coordinate placement of piping and conduit sleeves	3.2.9.5. Prior to pouring footings or foundations, remove all debris, loose material, and water from excavation. Where water has accumulated in excavation, obtain	3.2.13.3. Place vapor retarder/barrier membrane over rock base, lapping edges 12 inches. Tappe and each addres and penetrations. Extend membrane to festing	3.3.4. Ensure reinforcement, inserts, embedded parts, formed joint fillers, and joint devices
1705A.3.3, Chapter 17A, Part 2, Title 24, CCR are met. Comply with initial batch plant inspection and criteria	through concrete.	Architects and Geotechnical Engineers review of the suitability of sub-grade condition.	inches. Tape and seal edges and penetrations. Extend membrane to footing face and turn down to bottom of footing.	not disturbed during concrete placement.
specified in paragraph 2.6.2.2 of this Section.	3.1.1.7. Identify, verify, and coordinate the location, dimension, and requirements of all depressions, recesses, block-outs and other provisions.	3.2.9.6. Do not place concrete on mud or saturated soils. Repair sub-grade as required by Geotechnical Engineer at no additional contract cost.	3.2.13.4. Provide pipe "boots" at all pipe and conduit penetrations in accordance with manufactures recommendations.	3.3.5. Maintain records of concrete placement. Record date, location, quantity, air temperat and test samples taken.
2.7.1. Provide for testing under the provisions of Section 01 45 00.	3.1.1.8. Verify anchors, seats, plates, reinforcement and other items embedded in concrete are accurately placed, positioned securely, and will permit proper	3.2.10. Wall and structure formwork	3.2.13.5. Obtain inspectors approval of membrane installation before placing concrete.	3.3.6. Place concrete continuously between predetermined expansion, control, and construct joints.
	concrete placement.			
03 30 10 - 6	03 30 10 - 7	03 30 10 - 8	03 30 10 - 9	03 30 10
3.3.6.1. Place construction joints only at locations shown on drawings or as approved by Architect.	3.5.1.2. Control joint [saw cut] spacing shall generally not exceed 5-ft in any direction.	3.8.2. Classification shall be per General Building: Cast-in-Place, ACI 117, Section 4.0, unless noted otherwise.	3.11. EQUIPMENT BASES	
3.3.6.2. Install construction joints as indicated on drawings	3.5.2. Place formed construction joints in floor slab. Set top screed to required elevations. Secure to resist movement of wet concrete.	3.8.3. Unless noted otherwise, depressions in slab floors between high spots shall be a	3.11.1. Provide concrete bases and anchorage for mechanical, electrical, and other work as required and shown on the drawings and in accordance with reviewed Shop Drawings of	
3.3.6.3. Once concrete operation has begun, it shall be continued until the specific	3.5.3. Install isolation /expansion joints with sealant between slab edges and vertical structural	maximum 3/16 inch in ten feet, using a metal straight edge placed at any location on slab, and measured within 72 hours of pour.	related trades.	
panel, component, or section is complete.3.3.7. Thoroughly consolidate concrete during placement using mechanical vibrators. Do not	elements. 3.5.4. Install sealants in accordance with Division 07.		3.12. MISCELLANEOUS CONCRETE WORK3.12.1. Provide areaways, cast-in-place valve boxes, pits, splash blocks, bases, and other	
allow vibrators to contact forms or reinforcing.	3.5.4. Install sealants in accordance with Division 07.3.5.5. Provide construction joints or equal weakened plane joints at locations shown on	3.9. PATCHING	3.12.1. Provide areaways, cast-in-place valve boxes, pits, splash blocks, bases, and other miscellaneous concrete as shown and required to complete the Work. Conform to applicable requirements as specified in this section.	
3.3.8. Screed floors and slabs on grade level or slope to drain as noted on drawings.	drawings.	3.9.1. Exposed formed concrete surfaces, both interior and exterior, including surfaces designated to receive painted finish, shall provide surfaces suitable for subsequent		
3.4. CONCRETE FINISHING 3.4.1. Slab Finish	3.5.5.1. Saw cut slab before random shrinkage cracks form, and as soon as slab is firm enough to not be damaged by saw blade. Complete sawcutting within 12 hours of pour.	finishing, free from imperfect joints, fins, "honeycombing", air pockets or "bug" holes, or other such imperfections.	END OF SECTION	
3.4.1. Slab Finish3.4.1.1. Produce hard and impervious surfaces, free from defects and blemishes.	hours of pour. 3.6. CURING AND PROTECTION	3.9.2. Remove rough spots, stains and hardened mortar or grout from intended smooth surfaces by rubbing such surfaces lightly with fine Carborundum stone. Use liberal amount of water		
3.4.1.2. Provide steel troweled finish, Class 3, per ACI 302.1R. Steel troweling shall	3.6.1. Maintain concrete above 50 degrees F and in a thoroughly moist condition for at least the	and rub sufficiently to remove defects without changing texture of concrete.		
consist of three separate operations. Obtain Architect's approval of finish prior to proceeding.	first 7 days after placing concrete.	3.9.3. Filling Snap Tie Cone Holes:		
3.4.1.3. At all corridors, utility areas, and similar surfaces not receiving subsequent	3.6.2. Floor Surface Curing	3.9.3.1. Break off tie rods at bottom of cone holes.		
finish, provide medium swirl texture.	 3.6.2.1. Cure floor surfaces in accordance with ACI 308. 3.6.2.2 Curing Rightst Rissonant: Install blankets immediately after finishing and 	3.9.3.2. Concealed Applications: Flush hole with water, and allow to dry. Coat entire inner surface of cone hole with liquid bonding agent, then grout holes solid with approved compart grout and grind smooth.		
Slabs receiving thin set ceramic tile, waterproofing membranes or traffic topping; provide steel trowel and very light broom finish.	3.6.2.2. Curing Blanket Placement: Install blankets immediately after finishing and joint placement is completed. Place over all surfaces, including face of footings and depressions. Anchor as required to maintain in place for a period	with approved cement grout and grind smooth. 3933 Exposed Applications: Flush hole with water, and allow to dry. Coat entire		
3.4.2. Exterior Slab Floors at service areas:	footings and depressions. Anchor as required to maintain in place for a period of 7 days.	3.9.3.3. Exposed Applications: Flush hole with water, and allow to dry. Coat entire inner surface of cone hole with liquid bonding agent. Insert semi-recessed plug with approved cement.		
3.4.2.1. Exterior Service/Utility Concrete Slabs: Provide steel trowel and light broom finish. Broom finish shall be placed in a pattern as directed by the Architect.	3.6.2.3. Membrane sealing Compounds: Apply in accordance with manufacturer's instructions.	plug with approved cement. 3.10. DEFECTIVE CONCRETE		
3.4.3. All Other Surfaces	3.7. FIELD QUALITY CONTROL	3.10.1. Concrete will be considered defective if strength characteristics indicated by tests of		
3.4.3.1. All surfaces shall be as-cast, subject to repair of surface deficiencies as	3.7.1. Field inspection and testing will be performed in accordance with provisions of code	molded cylinders and core tests fall below the minimum 28-day strengths specified or indicated. Replace or adequately strengthen such defective concrete in a manner		
specified.	3.7.2. Provide free access to Work and cooperate with appointed firm.	acceptable to the Architect and Structural Engineer.		
3.4.4. Surface Defects3.4.4.1. Surface defects shall be as defined in ACI 309.2R.	3.7.3. Comply with requirements of CBC Section 1905A.1.2 regarding frequency of testing for concrete test specimens.	3.10.2. Concrete will be considered defective if any one of the following conditions occurs:3.10.2.1. Any concrete work not formed as indicated or is not in conformance with		
3.4.4.1. Surface defects shall be as defined in ACI 309.2R.3.4.4.2. Surface irregularities shall be as defined in ACI 347 for Class B surfaces for	3.7.4. One slump test will be taken for each set of test cylinders taken.	3.10.2.1. Any concrete work not formed as indicated or is not in conformance with specified tolerances.		
semi exposed surfaces, and Class A surfaces at all exposed to view		3.10.2.2. Any concrete with voids or honeycomb that has been cut, resurfaced or filled,		

3.4.4.2. Surface irregularities shall be as defined in ACI 347 for Class B surfaces for semi exposed surfaces, and Class A surfaces at all exposed to view 3.7.4.1. Prepare concrete sample(s) for each type of concrete placed each day. conditions. 3.4.4.3. All surface defects shall be repaired per approved methods and as specified. 3.7.4.2. Prepare one sample for each 50 cubic yards or fraction thereof. 3.7.4.3. Prepare one sample for each 2,000 square feet of slab or wall surface are

3.5. EXPANSION AND WEAKENED PLANE JOINT INSTALLATION **3.5.1.** Locate and form expansion control and contraction joints. Coordinate location with joint pattern shown for finish flooring. **3.5.1.1.** Expansion joint spacing shall generally not exceed 15-ft in any direction – 400-sf max area.

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3.8.1. All tolerances shall be as defined in ACI 117 and as specified.

placed, or a fraction thereof.

3.8. TOLERANCES

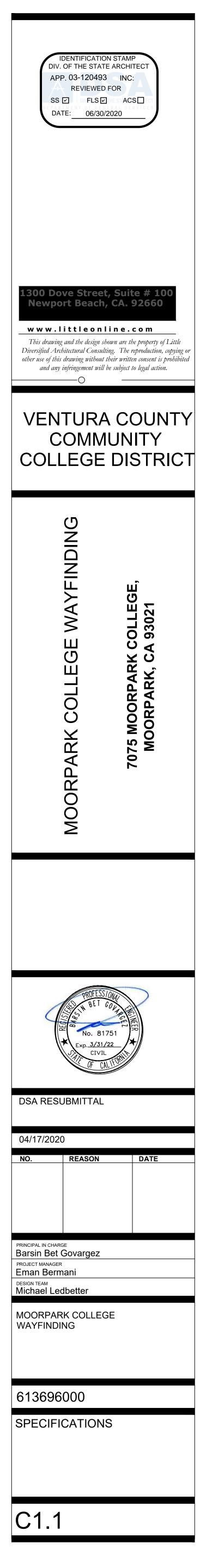
03 30 10 - 12

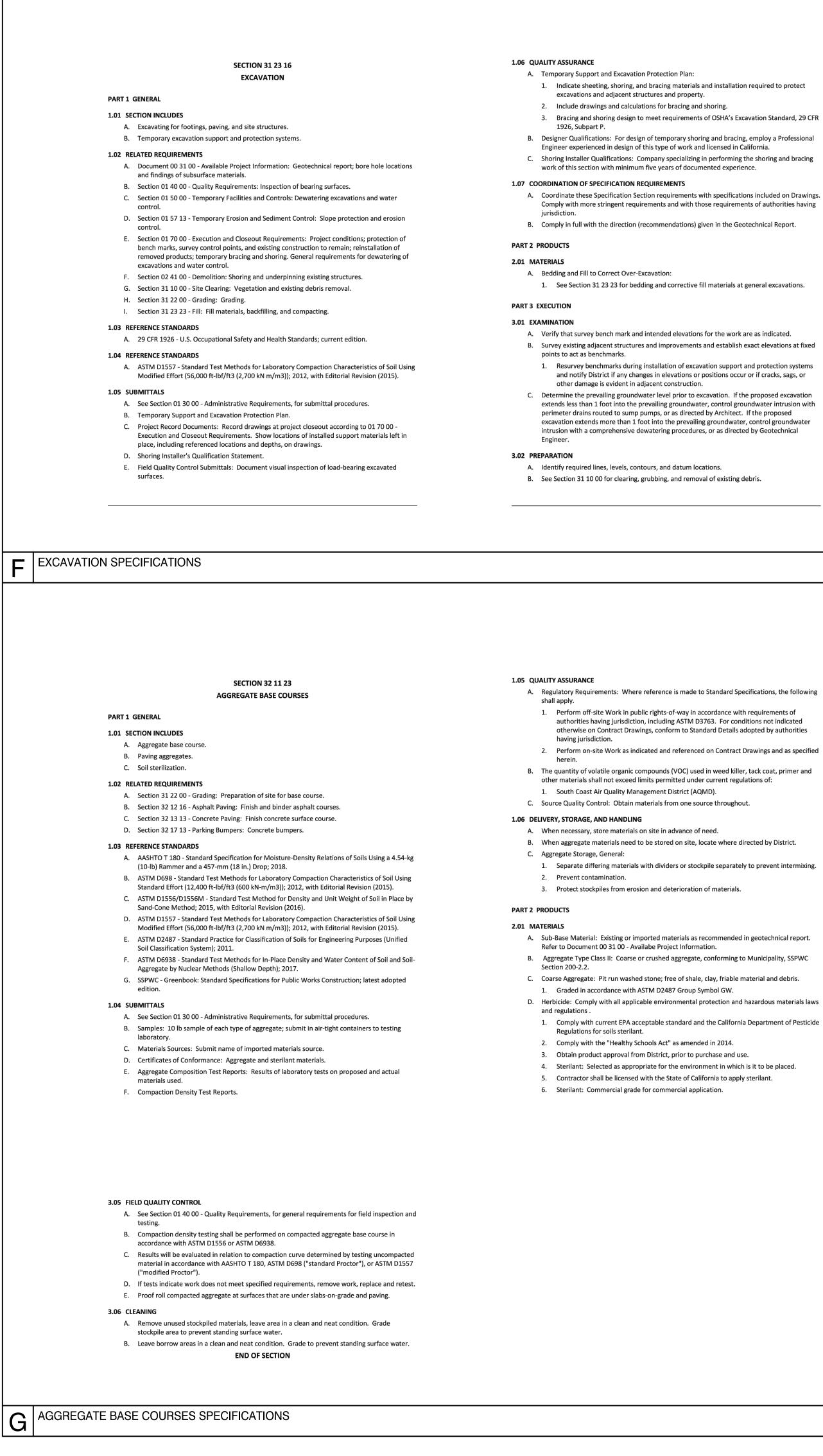
03 30 10 - 13

3.10.2.5. Replace or repair such defective concrete to the satisfaction of the Architect at no extra cost to the Owner.

3.10.2.4. Any concrete placed more than 90 minutes after batching.

03 30 10 - 14





C. Locate, identify, and protect utilities that remain and protect from damage.

3. Bracing and shoring design to meet requirements of OSHA's Excavation Standard, 29 CFR B. Designer Qualifications: For design of temporary shoring and bracing, employ a Professional

- A. Coordinate these Specification Section requirements with specifications included on Drawings. Comply with more stringent requirements and with those requirements of authorities having
- 1. Resurvey benchmarks during installation of excavation support and protection systems and notify District if any changes in elevations or positions occur or if cracks, sags, or
- C. Determine the prevailing groundwater level prior to excavation. If the proposed excavation extends less than 1 foot into the prevailing groundwater, control groundwater intrusion with perimeter drains routed to sump pumps, or as directed by Architect. If the proposed excavation extends more than 1 foot into the prevailing groundwater, control groundwater

- D. Protect bench marks, survey control points, existing structures, fences, sidewalks, paving, and curbs from excavating equipment and vehicular traffic. E. Grade top perimeter of excavation to prevent surface water from draining into excavation. Provide temporary means and methods, as required, to maintain surface water diversion until no longer needed, or as directed by Architect. 3.03 TEMPORARY EXCAVATION SUPPORT AND PROTECTION
 - A. Excavation Safety: Comply with OSHA's Excavation Standard, 29 CFR 1926, Subpart P. 1. Excavations in stable rock or in less than 5 feet in depth in ground judged as having no cave-in potential do not require excavation support and protection systems. 2. Depending upon excavation depth, time that excavation is open, soil classification, configuration and slope of excavation sidewalls, design and provide an excavation support and protection system that meets the requirements of 29 CFR 1926, Subpart P: a. Sloping and benching systems.
 - b. Support systems, shield systems, and other protective systems. B. Shoring Design: Comply with State of California Trenching and Shoring Manual issued by Offices of Structure Construction; 2011. 1. Provide all materials and services necessary to properly engineer and construct shoring for excavations. Selection of materials and design of shoring, underpinning and bracing of new and existing structures shall be solely the responsibility of the Contractor.
 - C. Underpin adjacent structures that could be damaged by excavating work, including utilities and pipe chases. D. Protect excavations from cave-in and from loose soil and other matter from falling in. E. Leave excavation support and protection systems, used as formwork or within 10 feet of
 - existing foundations, permanently in place, unless otherwise noted. 1. Cut off top 4 feet below grade, abandon remainder. F. Excavation support and protection systems not required to remain in place may be removed
 - subject to approval of District or District's Representative. 1. Remove temporary shoring and bracing in a manner to avoid harmful disturbance to underlying soils and damage to buildings, structures, pavements, facilities and utilities.
- 3.04 EXCAVATING A. Excavate to accommodate construction operations and paving/site structures. Excavate to the specified elevations.
 - 2. Excavate to the length and width required to safely install, adjust, and remove any forms,
 - bracing, or supports necessary for the installation of the work. 3. Hand trim excavations. Remove loose matter. 4. Excavate subsoil from areas to be filled with topsoil, to construct foundations, footings,
 - slabs on grade, paving and to achieve final finish grades. 5. Over-excavate to working elevations for backfilling and compaction operations.
- 6. Specific Site requirements:

1. Perform off-site Work in public rights-of-way in accordance with requirements of

- 1. Separate differing materials with dividers or stockpile separately to prevent intermixing.

- A. Sub-Base Material: Existing or imported materials as recommended in geotechnical report.
- D. Herbicide: Comply with all applicable environmental protection and hazardous materials laws
- 1. Comply with current EPA acceptable standard and the California Department of Pesticide

- 7. Payment for soil sterilization: Include full compensation for application and all materials and incidental work required.
- 8. Application Rate: Follow manufacturer recommendations. 9. Acceptable Manufacturers:
- a. Dow AgroSciences; Spike 80DF: www.dowagro.com.
- b. Pro-Serve Inc.; Bare-Spot Monobor-Chlorate: www.pro-serveinc.com. c. Casoron 50W by Uniroyal Chemical Co., Inc.
- d. Substitutions: See Section 01 60 00 Product Requirements.
- 2.02 SOURCE QUALITY CONTROL A. See Section 01 40 00 - Quality Requirements, for general requirements for testing and analysis
- of aggregate materials. B. Where aggregate materials are specified using ASTM D2487 classification, testing of samples
- for compliance shall be provided before delivery to site. C. If tests indicate materials do not meet specified requirements, change material and retest.
- D. Provide materials of each type from same source throughout the Work.

PART 3 EXECUTION

- 3.01 EXAMINATION
- A. Establishment of Grades 1. Set grade stakes per Section 01 70 00 - Execution and Closeout Requirements. 2. All work shall conform to the lines, elevations, and grades shown on the Drawings.
 - a. Use three consecutive points set on the same slope together so that any variation from a straight grade can be detected.
 - b. Report any such variation to the Architect. Contractor shall be responsible for any error in the grade of the finished work.
- 3. Grade or location stakes lost or disturbed, shall be reset by the Surveyor at no additional expense to District. 4. Areas having drainage gradients of 2 percent or more, provide elevation stakes, set with
- instrument, at grid intervals of 25 feet. a. Intermediate stakes may be set by using a tightly-drawn string line over the tops of adjacent stakes b. Grade stakes must be set at all grade breaks, grade changes, etc.
- 5. Areas having drainage gradients of less than 2 percent; provide elevation stakes, set with instrument, at 10 foot intervals.
- a. Grade stakes must be set at all grade breaks, grade changes, etc. B. Verify that survey bench marks and intended elevations for the work are as indicated.
- C. Verify substrate has been inspected, gradients and elevations are correct, and is dry.
- 3.02 PREPARATION A. Stockpiling:

- Representative to prevent segregation. 4. Materials obtained from different sources shall be stockpiled separately. B. Soil Sterilant: 1. Sterilize soil areas to receive paving. 2. Apply soil sterilant in accordance with manufacturer's instructions and applicable environmental regulations 3. Take care to confine application to the areas to be paved. Sterilant shall not be applied within 2 feet of planting areas. compacting. D. Do not place aggregate on soft, muddy, or frozen surfaces. 3.03 INSTALLATION vehicular traffic and 90 percent at pedestrian-only traffic. B. Application of Base Course: traveled way as frequently as may be necessary to remove ruts, cuts, and breaks in the surface.
- prepared subgrade. C. Under Bituminous Concrete Paving:

a. Flatwork/Hardscape/wall foundation and sign structure:

consultant.

the footing.

Contract Sum

1.a) In areas of proposed concrete flatwork or pavement, provide a minimum overexcavation and recompaction of 1.5 feet below existing grade or 18 inches below The lowest proposed subgrade elevation, whichever is deeper. 1.b) In areas of proposed sign structure and wall foundation, provide a minimum over-excavation and recompaction of 1.5 feet below bottom of the lowest footing or existing grade, whichever is deeper.

2) Extend over-excavation and recompaction a minimum horizontal distance of 2 feet from outside hardscape limits and structure foundation. Proof-roll the bottom of the removal to identify yielding subgrade conditions (for additional removal, if necessary) under the observation of the geotechnical 4) onsite soil may be used for fill once they are cleaned of all organic material, rock ,

debris, and irreducible material larger than 6 inches. Import soil shall be equal to, or better than on-site soils in strength, expansion, and compressibility characteristics. All onsite and import soil material shall evaluate and test the import soils in order to confirm the quality of the material. Also, fill and backfill should be placed at, or slightly above optimum moisture in layers with loose thickness not greater than 6 inches. Each layer should be compacted to a minimum of 90% of the maximum dry density obtainable by the ASTM D 1557 test method. The upper one foot of subgrade below areas to be paved should be compacted to minimum of 95% of the maximum dry density. b. After completion of the removal of existing fill soils and prior to fill placement, scarify the exposed surface to a minimum depth of 6 inches, moisture condition as necessary to near optimum moisture content and recompact using heavy

compaction equipment to an unyielding condition. 7. Where excavations are made to a depth greater than that indicated, such additional depth shall be filled with concrete having the same compressive strength as specified for a. Correct unauthorized and erroneous excavation at no change in Contract Time or

B. Notify Architect of unexpected subsurface conditions and discontinue affected Work in area until notified to resume work.

C. Slope banks of excavations deeper than 4 feet to angle of repose or less until shored, per CalOSHA requirements for Type C Soil. 1. Machine slope banks of excavations to minimum 1 to 1 ratio horizontal to vertical or angle of repose, if less, until shored.

a. Exception: If authorized in writing by Geotechnical Engineer. b. Slope must comply with local codes, ordinances and requirements of agencies having jurisdiction. c. See Section 00 31 00 - Available Project Information.

D. Do not interfere with 45 degree influence line of bearing splay of foundations.

1. Avoid interference at footings by providing additional width, depth, and other provisions. E. Remove lumped subsoil, boulders, and rock up to 1/3 cu yd measured by volume.

F. Provide temporary means and methods, as required, to remove all water from excavations until directed by Architect. Remove and replace soils deemed suitable by classification and which are excessively moist due to lack of dewatering or surface water control.

- 3.05 SUBGRADE PREPARATION A. See Section 31 23 23 for subgrade preparation at general excavations.
- 3.06 FILLING AND BACKFILLING
- A. Do not fill or backfill until all debris, water, unsatisfactory soil materials, obstructions, and deleterious materials have been removed from excavation. B. See Section 31 23 23 for fill, backfill, and compaction requirements at general excavations. C. See Section 31 22 00 for rough and final grading and topsoil replacement requirements.
- 3.07 REPAIR
- A. Correct areas that are over-excavated and load-bearing surfaces that are disturbed; see Section 31 23 23 at no additional cost.
- 3.08 FIELD QUALITY CONTROL
- A. See Section 01 40 00 Quality Requirements, for general requirements for field inspection and
- B. Provide for visual inspection of load-bearing excavated surfaces by Architect before placement of foundations.
- C. Scarification, over excavation and all other excavations will be subject to the approval of the Soils Engineer.
- 3.09 CLEANING
- A. Stockpile excavated material to be re-used in area designated on site in accordance with Section 31 22 00. B. Remove excavated material that is unsuitable for re-use from site.
- C. Remove excess excavated material from site. 1. Geotechnical engineer or other consistant as selected by District to test soils prior to

export for disposition. 3.10 PROTECTION

placement.

- A. Divert surface flow from rains or water discharges from the excavation.
- B. Prevent displacement of banks and keep loose soil from falling into excavation; maintain soil stability.
- C. Protect open excavations from rainfall, runoff, freezing groundwater, or excessive drying so as to maintain foundation subgrade in satisfactory, undisturbed condition.
- D. Protect bottom of excavations and soil adjacent to and beneath foundation from freezing. E. Keep excavations free of standing water and completely free of water during concrete

END OF SECTION

2. Stockpile all materials, including approved material available from excavation and grading, in the manner and at the locations designated.

3. Aggregates shall be stockpiled on the cleared and leveled areas designated by the Owner

1. Clear and level storage sites prior to stockpiling of material.

C. Correct irregularities in substrate gradient and elevation by scarifying, reshaping, and re-

A. Place and compact aggregate base material in accordance with ASTM D3763, Subsection 301-2. Place aggregate base below curbs and gutters and paving also, compacted to 95 percent at

1. After preparing the subgrade, Avoid all vehicular or machine traffic on the subgrade. a. Should it be necessary to haul over the prepared subgrade, drag and roll the

b. Rake and hand tamp all cuts, ruts, and breaks in the surface of the subgrade that are not removed by the above operations.

c. Equip with pneumatic tires all equipment used for transporting materials over the 2. Do not permit continued use of sections of prepared subgrade for hauling, so as to cut up or deform it from the true cross-section. Protect the prepared subgrade from all traffic.

3. Maintain the surface in its finished condition until the succeeding layer is placed. 1. It is required that areas of exterior asphalt pavement be underlain by a layer of aggregate

base material which meets the requirements, Thickness of base layer is as shown on the Drawings and varies per the Usage Type area. a. It is required that the upper 12 inches of soils below asphalt pavement base material

be over-excavated and consist predominantly of satisfactory soil materials and/or approved imported fill.

1) Engineered Fill: See Section 31 23 23 - Fill. b. It is required that the exposed bottom surface soils, below overexcavation, be scarified to the recommended depth of 8 inches, moisture conditioned to achieve optimum moisture content, but not higher than 2 percent above optimum, and then re-compacted to a minimum 90 percent relative compaction before any fill materials are placed.

- 2. The above subgrade preparation recommendations are based on the assumption that soils encountered during field exploration are representative of soils throughout the site. a. However, there can be unforeseen and unanticipated variations in soils between points of subsurface exploration. For this reason, the actual subgrade preparation
- will have to be determined on the basis of in-grading observations and testing performed by representatives of the project geotechnical consultant. 3. Provide grade stakes and elevations by a California Licensed Surveyor (LS) for the
- Geotechnical Engineer. a. Verify that the over-excavation depths, shown on the construction drawings for asphalt concrete pavement structural sections, have been achieved prior to recompaction
- 4. Correct irregularities by dressing down or filling as may be required, to bring areas to true subgrade elevations 5. Where filling is required, scarify the subgrade to bond the new material to the in place
- material; use additional material as required at no additional cost. Subject to the approval of the Architect.
- 6. Remove excess material from the site to a legal disposal area. D. Under Portland Cement Concrete Paving:
- 1. Compact to 95 percent of maximum dry density and 90 percent at pedestrian-only traffic.
- E. Place aggregate in maximum 4 inch layers and roller compact to specified density. F. Level and contour surfaces to elevations and gradients indicated.
- G. Add small quantities of fine aggregate to coarse aggregate as appropriate to assist
- compaction
- H. Add water to assist compaction. If excess water is apparent, remove aggregate and aerate to reduce moisture content. I. Use mechanical tamping equipment in areas inaccessible to compaction equipment.
- J. Apply herbicide to finished surface.
- 3.04 TOLERANCES
- A. Subgrade Tolerances:
- 1. Subgrade for Pavement: Do not vary more than 0.02 ft..
- 2. Subgrade for Subbase or Base Material: Do not vary more than 0.04 ft..
- 3. Variations within the above specified tolerances shall be compensating so that the average grade and cross section specified are met.
- B. Flatness: Maximum variation of 1/4 inch measured with 10 foot straight edge. C. Scheduled Compacted Thickness: Within 1/4 inch.
- D. Variation From Design Elevation: Within 1/2 inch.

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	IDENTIFICATION STAMP DIV. OF THE STATE ARCHITECT APP. 03-120493 INC: REVIEWED FOR SS ☑ FLS ☑ ACS ☐ DATE: 06/30/2020
	1300 Dove Street, Suite # 100 Newport Beach, CA. 92660 www.littleonline.com
	This drawing and the design shown are the property of Little Diversified Architectural Consulting. The reproduction, copying or other use of this drawing without their written consent is prohibited and any infringement will be subject to legal action.
	VENTURA COUNTY COMMUNITY COLLEGE DISTRICT
	WAYFINDING NLLEGE, 3021
	А У FI EGE, 21
	MOORPARK COLLEGE WAYFI 7075 MOORPARK COLLEGE, MOORPARK, CA 93021
	No. 81751 Exp. 3/31/22 CIVIL
	DSA RESUBMITTAL
	04/17/2020 NO. REASON DATE
	PRINCIPAL IN CHARGE Barsin Bet Govargez PROJECT MANAGER Eman Bermani DESIGN TEAM
	Michael Ledbetter MOORPARK COLLEGE
	MOORPARK COLLEGE WAYFINDING
	613696000 SPECIFICATIONS
	C1.2

SECTION 31 23 23 FILL	 Submit samples directly to Geotechnical Engineer for testing and analysis copy transmittals to Architect and District. Materials Sources: Submit name of imported materials source.
PART 1 GENERAL	 D. Fill Composition Test Reports: Results of laboratory tests on proposed and actual materials
	used, including manufactured fill.
 1.01 SECTION INCLUDES A. Filling, backfilling, and compacting for footings, paving, and site structures. 	 E. Compaction Density Test Reports. F. Manufacturer's Instructions.
 Filling holes, pits, and excavations generated as a result of removal (demolition) operations. 	G. Manufacturer's Qualification Statement.
1.02 RELATED REQUIREMENTS	H. Specimen Warranty.
A. Section 00 31 00 - Available Project Information: Geotechnical report; bore hole locations and	 Provide proof that all imported materials conform to the requirements of DTSC-Clean Fill Imported Fill Materials for School Sites by proper documentation for the imported materia
findings of subsurface materials.	1.06 QUALITY ASSURANCE
B. Section 01 57 13 - Temporary Erosion and Sediment Control: Slope protection and erosion control.	A. Manufacturer Qualifications: Company specializing in manufacturing products specified in
C. Section 03 30 00 - Cast-in-Place Concrete.	section, with not less than ten years of documented experience.
D. Section 31 22 00 - Grading: Removal and handling of soil to be re-used.	B. Testing Agency Qualifications: Independent firm specializing in performing testing and inspections of the type specified in this section.
 E. Section 31 22 00 - Grading: Site grading. F. Section 31 23 16 - Excavation: Removal and handling of soil to be re-used. 	C. Copies of Documents at Project Site: Maintain at the project site a copy of each reference
1.03 DEFINITIONS	document that prescribes execution requirements.
A. Finish Grade Elevations: Indicated on drawings.	1.07 DELIVERY, STORAGE, AND HANDLING
B. Subgrade Elevations: Indicated on drawings.	 A. When necessary, store materials on site in advance of need. B. When fill materials need to be stored on site, locate stockpiles where agreed to.
1.04 REFERENCE STANDARDS	 Separate differing materials with dividers or stockpile separately to prevent intermix
A. AASHTO T 180 - Standard Specification for Moisture-Density Relations of Soils Using a 4.54-kg	2. Prevent contamination.
(10-lb) Rammer and a 457-mm (18 in.) Drop; 2018. B. ASTM D4829 - Standard Test Method for Expansion Index of Soils; 2011.	3. Protect stockpiles from erosion and deterioration of materials.
C. ASTM D1556/D1556M - Standard Test Method for Density and Unit Weight of Soil in Place by	1.08 WARRANTY
Sand-Cone Method; 2015, with Editorial Revision (2016).	A. See Section 01 78 00 - Closeout Submittals, for additional warranty requirements.
D. ASTM D1557 - Standard Test Methods for Laboratory Compaction Characteristics of Soil Using Modified Effort (56,000 ft-lbf/ft3 (2,700 kN m/m3)); 2012, with Editorial Revision (2015).	B. Correct defective Work within a five year period after Date of Substantial Completion.
E. ASTM D2487 - Standard Practice for Classification of Soils for Engineering Purposes (Unified	PART 2 PRODUCTS
Soil Classification System); 2011.	2.01 FILL MATERIALS
 F. DTSC-Clean Fill - California Department of Toxic Substances Control - Clean Imported Fill Material; Current. 	A. General Fill: Subsoil excavated on-site.
G. Greenbook - Greenbook: Standard Specifications for Public Works Construction; latest	1. Graded.
adopted edition.	2. Free of lumps larger than 3 inches, rocks larger than 4 inches, and debris.
1.05 SUBMITTALS	 Complying with ASTM D2487 Group Symbol CL. B. Structural Fill: Subsoil excavated on-site.
 A. See Section 01 30 00 - Administrative Requirements, for submittal procedures. B. Soil Samples: 10 pounds sample of each type of fill; submit in air-tight containers to testing 	1. Graded.
laboratory.	
B. Structural Fill:	b. In conjunction with each density calibration check, check the calibration curves
 Use general fill. Fill up to subgrade elevations. 	 furnished with the moisture gages in accordance with ASTM D6938. c. If field tests are performed using nuclear methods, make calibration checks of bo density and moisture gages at beginning of Work, on each different type of mate
 Fill up to subgrade elevations. Maximum depth per lift: 6 inches, compacted. 	 furnished with the moisture gages in accordance with ASTM D6938. c. If field tests are performed using nuclear methods, make calibration checks of be density and moisture gages at beginning of Work, on each different type of mate encountered, and at intervals as directed by Architect or District's testing and
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 Fill up to subgrade elevations. Maximum depth per lift: 6 inches, compacted. Compact to minimum 90 percent of maximum dry density. At Footings: Use general fill. Fill up to subgrade elevation. Compact each lift to 90 percent of maximum dry density. Do not backfill against unsupported foundation walls. Backfill simultaneously on each side of unsupported foundation walls until supports are in place. Under Monolithic Paving : Compact subsoil to 90 percent of its maximum dry density before placing fill. Use general fill. Use general fill. Gompact subsoil to 90 percent of its maximum dry density before placing fill. Use general fill. Fill up to subgrade elevation. Compact to 90 percent of maximum dry density. See Section 32 11 23 for aggregate base course placed over fill. 3.05 TOLERANCES Top Surface of General Filling: Plus or minus 1 inch from required elevations. Top Surface of Filling Under Paved Areas: Plus or minus 1/2 inch from required elevations. A. See Section 01 40 00 - Quality Requirements, for general requirements for field inspection and testing. 	 furnished with the moisture gages in accordance with ASTM D6938. c. If field tests are performed using nuclear methods, make calibration checks of b density and moisture gages at beginning of Work, on each different type of mat encountered, and at intervals as directed by Architect or District's testing and inspection agency. c. Evaluate results in relation to compaction curve determined by testing uncompacted mat in accordance with ASTM D 1557 ("modified Proctor") or AASHTO T 180. D. Non-compliance: If tests indicate work does not meet specified requirements, remove wor replace and retest. 1. Should tests of fill or backfill indicate non-compliance with required density, Contract shall over-excavate, recompact and retest until specified density is obtained. 2. Costs and Time associated with remedial Work and retesting shall be in accordance or provisions of the General Conditions. 3. Retesting to demonstrate compliance shall be by a testing laboratory acceptable to District and shall be at Contractor's expense. E. Frequency of Tests: a. For each strata of soil on which footings will be placed, perform at least one tes verify required design bearing capacities. b. Subsequent verification and approval of each footing subgrade may be based or visual comparison of each subgrade With related tested strata when acceptable Geotechnical Engineer. Paved Areas and Building Slab Subgrade Testing: a. Perform at least one field density test of subgrade for every 2,000 sf of paved at or building slab, but in no case fewer than three tests. b. In each compacted fill layer, perform one field density test for every 2,000 sf of overlaying building slab or paved area, but in no case fewer than three tests.
 Fill up to subgrade elevations. Maximum depth per lift: 6 inches, compacted. Compact to minimum 90 percent of maximum dry density. At Footings: Use general fill. Fill up to subgrade elevation. Compact each lift to 90 percent of maximum dry density. Do not backfill against unsupported foundation walls. Backfill simultaneously on each side of unsupported foundation walls until supports are in place. Under Monolithic Paving : Compact subsoil to 90 percent of its maximum dry density before placing fill. Use general fill. Fill up to subgrade elevation. Compact subsoil to 90 percent of its maximum dry density before placing fill. Use general fill. Fill up to subgrade elevation. Compact to 90 percent of maximum dry density. See Section 32 11 23 for aggregate base course placed over fill. 3.05 TOLERANCES A Top Surface of General Filling: Plus or minus 1 inch from required elevations. Top Surface of Filling Under Paved Areas: Plus or minus 1/2 inch from required elevations. A See Section 01 40 00 - Quality Requirements, for general requirements for field inspection and 	 furnished with the moisture gages in accordance with ASTM D6938. c. If field tests are performed using nuclear methods, make calibration checks of b density and moisture gages at beginning of Work, on each different type of mat encountered, and at intervals as directed by Architect or District's testing and inspection agency. C. Evaluate results in relation to compaction curve determined by testing uncompacted mat in accordance with ASTM D 1557 ("modified Proctor") or AASHTO T 180. D. Non-compliance: If tests indicate work does not meet specified requirements, remove work replace and retest. 1. Should tests of fill or backfill indicate non-compliance with required density, Contract shall over-excavate, recompact and retest until specified density is obtained. 2. Costs and Time associated with remedial Work and retesting shall be in accordance or provisions of the General Conditions. 3. Retesting to demonstrate compliance shall be by a testing laboratory acceptable to District and shall be at Contractor's expense. E. Frequency of Tests: 1. Footing Subgrade Testing: a. For each strata of soil on which footings will be placed, perform at least one tes verify required design bearing capacities. b. Subsequent verification and approval of each footing subgrade may be based or visual comparison of each subgrade Testing: a. Paved Areas and Building Slab Subgrade Testing: a. Perform at least one field density test of subgrade for every 2,000 sf of paved at or building slab, but in no case fewer than three tests. b. In each compacted fill layer, perform one field density test for every 2,000 sf of paved at or building slab, but in no case fewer than three tests.
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 Fill up to subgrade elevations. Maximum depth per lift: 6 inches, compacted. Compact to minimum 90 percent of maximum dry density. CAR Footings: Use general fill. Fill up to subgrade elevation. Compact each lift to 90 percent of maximum dry density. Do not backfill against unsupported foundation walls. Backfill simultaneously on each side of unsupported foundation walls until supports are in place. Under Monolithic Paving : Compact each lift to 90 percent of its maximum dry density before placing fill. Use general fill. Genpact subsoil to 90 percent of its maximum dry density before placing fill. Use general fill. Genpact subsoil to 90 percent of maximum dry density. See Section 32 11 23 for aggregate base course placed over fill. A Top Surface of General Filling: Plus or minus 1 inch from required elevations. Top Surface of Filling Under Paved Areas: Plus or minus 1/2 inch from required elevations. See Section 01 40 00 - Quality Requirements, for general requirements for field inspection and testing. Laboratory Tests and Analyses: Where backfill is required to be compacted to a specified density, tests for compliance shall be made in accordance with ASTM D1556 or ASTM D6938. Field inspections and testing shall be performed and submitted in accordance with 	 furnished with the moisture gages in accordance with ASTM D6938. c. If field tests are performed using nuclear methods, make calibration checks of I density and moisture gages at beginning of Work, on each different type of mai encountered, and at intervals as directed by Architect or District's testing and inspection agency. C. Evaluate results in relation to compaction curve determined by testing uncompacted mat in accordance with ASTM D 1557 ("modified Proctor") or AASHTO T 180. D. Non-compliance: If tests indicate work does not meet specified requirements, remove we replace and retest. 1. Should tests of fill or backfill indicate non-compliance with required density, Contranshall over-excavate, recompact and retest until specified density is obtained. 2. Costs and Time associated with remedial Work and retesting shall be in accordance provisions of the General Conditions. 3. Retesting to demonstrate compliance shall be by a testing laboratory acceptable to District and shall be at Contractor's expense. E. Frequency of Tests: For each strata of soil on which footings will be placed, perform at least one test verify required design bearing capacities. b. Subsequent verification and approval of each footing subgrade may be based o visual comparison of each subgrade Testing: Paved Areas and Building Slab Subgrade Testing: Perform at least one field density test for every 2,000 sf of paved a or building slab, but in no case fewer than three tests. b. In each compacted fill layer, perform at least two field density tests at location elevations as directed.
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 Fill up to subgrade elevations. Maximum depth per lift: 6 inches, compacted. Compact to minimum 90 percent of maximum dry density. At Footings: Use general fill. Fill up to subgrade elevation. Compact each lift to 90 percent of maximum dry density. Do not backfill against unsupported foundation walls. Backfill simultaneously on each side of unsupported foundation walls until supports are in place. Under Monolithic Paving : Compact subsoil to 90 percent of its maximum dry density before placing fill. Use general fill. Fill up to subgrade elevation. Compact to 90 percent of maximum dry density before placing fill. Use general fill. Fill up to subgrade elevation. Compact to 90 percent of maximum dry density before placing fill. Use general fill. Fill up to subgrade elevation. Compact to 90 percent of maximum dry density. See Section 32 11 23 for aggregate base course placed over fill. 305 TOLERANCES Top Surface of General Filling: Plus or minus 1 inch from required elevations. Top Surface of Filling Under Paved Areas: Plus or minus 1/2 inch from required elevations. 306 FIELD QUALITY CONTROL Laboratory Tests and Analyses: Where backfill is required to be compacted to a specified density, tests for compliance shall be made in accordance with requirements specified in Section 01 40 00 - Quality Requirements. Perform compaction density testing on compacted fill in accordance with ASTM D1556 or ASTM D6938. Field inspections and testing shall be performed and submitted in accordance with 	 furnished with the moisture gages in accordance with ASTM D6938. c. If field tests are performed using nuclear methods, make calibration checks of I density and moisture gages at beginning of Work, on each different type of mal encountered, and at intervals as directed by Architect or District's testing and inspection agency. c. Evaluate results in relation to compaction curve determined by testing uncompacted mal in accordance with ASTM D 1557 ("modified Protor") or AASHTO T 180. D. Non-compliance: if tests indicate work does not meet specified requirements, remove we replace and retest. 1. Should tests of fill or backfill indicate non-compliance with regulated density, Contrar shall over-excavate, recompact and retest until specified density is obtained. 2. Costs and Time associated with remedial Work and retesting shall be in accordance provisions of the General Conditions. 3. Retesting to demonstrate compliance shall be by a testing laboratory acceptable to District and shall be at Contractor's expense. E. Frequency of Tests: Fore ach strata of soil on which footings will be placed, perform at least one test verify required design bearing capacities. Subsequent verification and approval of each footing subgrade may be based o visual comparison of each subgrade With related tested strata when acceptable Geotechnical Engineer. Paved Areas and Building Slab Subgrade Testing: Paved Areas and Building Slab Subgrade Testing: Paved Areas and Building slab or paved area, but in no case fewer than three tests. In each compacted fill layer, perform one field density test for every 2,000 sf of overlaying building slab, but in no case fewer than three tests. In each compacted fill layer, perform one field density test at location elevations as directed. For for foll compacted fill at surfaces that will be under slabs-on-grade. Surf CLEANING See Sec
 Fill up to subgrade elevations. Maximum depth per lift: 6 inches, compacted. Compact to minimum 90 percent of maximum dry density. At Footings: Use general fill. Fill up to subgrade elevation. Compact each lift to 90 percent of maximum dry density. Do not backfill against unsupported foundation walls. Backfill simultaneously on each side of unsupported foundation walls until supports are in place. Under Monolithic Paving : 	 furnished with the moisture gages in accordance with ASTM D6938. c. If field tests are performed using nuclear methods, make calibration checks of I density and moisture gages at beginning of Work, on each different type of mai encountered, and at intervals as directed by Architect or District's testing and inspection agency. c. Evaluate results in relation to compaction curve determined by testing uncompacted mai in accordance with ASTM D 1557 ("modified Proctor") or AASHTO T 180. D. Non-compliance: if tests indicate work does not meet specified requirements, remove we replace and retest. 1. Should tests of fill or backfill indicate non-compliance with required density, Contras shall over-excavate, recompact and retest until specified density is obtained. 2. Costs and Time associated with remedial Work and retesting shall be in accordance provisions of the General Conditions. 3. Retesting to demonstrate compliance shall be by a testing laboratory acceptable to District and shall be at Contractor's expense. E. Frequency of Tests: Fore ach strata of soil on which footings will be placed, perform at least one test verify required design bearing capacities. Subsequent verification and approval of each footing subgrade may be based o visual comparison of each subgrade Testing: Paved Areas and Building Slab Subgrade Testing: Paved Areas and Building Slab Subgrade Testing: Paved Areas and Building Slab but in no case fewer than three tests. In each compacted fill layer, perform one field density test for every 2,000 sf of overlaying building slab, but in no case fewer than three tests. In each compacted fill layer, perform one field density test at location elevations as directed. For for foll compacted fill at surfaces that will be under slabs-on-grade. Surface and surfaces that will be under slabs-on-grade. Surf cuesting Slab compacted test wat
 Fill up to subgrade elevations. Maximum depth per lift: 6 inches, compacted. Compact to minimum 90 percent of maximum dry density. At Footings: Use general fill. Fill up to subgrade elevation. Compact each lift to 90 percent of maximum dry density. Fill up to subgrade elevation. Compact each lift to 90 percent of maximum dry density. Do not backfill against unsupported foundation walls. Backfill simultaneously on each side of unsupported foundation walls until supports are in place. Under Monolithic Paving : Compact subsolit to 90 percent of its maximum dry density before placing fill. Use general fill. Fill up to subgrade elevation. Compact to 90 percent of maximum dry density. See Section 32 11 23 for aggregate base course placed over fill. Sup Surface of General Filling: Plus or minus 1 inch from required elevations. Top Surface of Filling Under Paved Areas: Plus or minus 1/2 inch from required elevations. As See Section 01 40 00 - Quality Requirements, for general requirements for field inspection and testing. Laboratory Tests and Analyses: Where backfill is required to be compacted to a specified in Section 01 40 00 - Quality Requirements. Perform compaction density testing on compacted fill in accordance with ASTM D1556 or ASTM D6938. Field inspections and testing shall be performed and submitted in accordance with requirements specified in Section 01 40 00 - Quality Requirements. Allow testing service to inspect and approve each subgrade and fill layer before furt	 furnished with the moisture gages in accordance with ASTM D6938. c. If field tests are performed using nuclear methods, make calibration checks of i density and moisture gages at beginning of Work, on each different type of ma encountered, and at intervals as directed by Architect or District's testing and in accordance with ASTM D 1557 ("modified Proctor") or AASHTO T 180. D. Non-compliance: If tests indicate work does not meet specified requirements, remove w replace and retest. 1. Should tests of fill or backfill indicate non-compliance with required density, contra shall over-excavate, recompact and retest until specified density is obtained. 2. Costs and Time associated with remedial Work and retesting shall be in accordance provisions of the General Conditions. 3. Retesting to demonstrate compliance shall be by a testing laboratory acceptable to District and shall be at Contractor's expense. E. Frequency of Tests: 1. Footing Subgrade Testing: a. For each strata of soil on which footings will be placed, perform at least one te visual comparison of each subgrade testing: b. Subsequent verification and approval of each footing subgrade may be based o visual comparison of each subgrade Testing: a. Perform at least one field density test of subgrade for every 2,000 sf of paved a or building slab, but in no case fewer than three tests. b. In each compacted fill ayr, perform at least two field density tests of very 2,000 sf of overlaying building slab or paved area, but in no case fewer than three tests. b. In each compacted fill as urfaces that will be under slabs-on-grade. A. Foro forl Compacted fill as urfaces that will be under slabs-on-grade. B. Foro forl Compacted fill as urfaces that will be under slabs-on-grade. B. Remove unused stockpiled materials, leave area in a clean and neat condition. Grade stockpile area to prevent standing surface water.
 Fill up to subgrade elevations. Maximum depth per lift: 6 inches, compacted. Compact to minimum 90 percent of maximum dry density. At Footings: Use general fill. Fill up to subgrade elevation. Compact each lift to 90 percent of maximum dry density. Do not backfill against unsupported foundation walls. Backfill simultaneously on each side of unsupported foundation walls until supports are in place. Under Monolithic Paving : 	 furnished with the moisture gages in accordance with ASTM D6938. c. If field tests are performed using nuclear methods, make calibration checks of I density and moisture gages at beginning of Work, on each different type of ma encountered, and at intervals as directed by Architect or District's testing and inspection agency. c. Evaluate results in relation to compaction curve determined by testing uncompacted main accordance with ASTM D 1557 ("modified Proctor") or AASHTO T 180. D. Non-compliance: If tests indicate work does not meet specified requirements, remove wireplace and retest. 1. Should tests of fill or backfill indicate non-compliance with required density, Contrashall over-excavate, recompact and retest until specified density is obtained. 2. Costs and Time associated with remedial Work and retesting shall be in accordance provisions of the General Conditions. 3. Retesting to demonstrate compliance shall be by a testing laboratory acceptable to District and shall be at Contractor's expense. E. Frequency of Tests: 1. Footing Subgrade Testing: a. For each strata of soil on which footings will be placed, perform at least one test verify required design bearing capacities. b. Subsequent verification and approval of each footing subgrade may be based o visual comparison of each subgrade Testing: a. Porform at least one field density test of subgrade for every 2,000 sf of paved a or building slab, but in no case fewer than three tests. b. In each compacted fill as urfaces that will be under slabs-on-grade. A. Porfor orl Compacted fill as urfaces that will be under slabs-on-grade. A. Porfor orl Compacted fill as urfaces that will be under slabs-on-grade. A. See Section 01 74 19 - Construction Waste Management and Disposal, for additional requirements. B. Remove unused stockpiled materials, leave area in a clean and neat condition. Grade stockpil
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Engineer for testing and analysis copy

materials source. pratory tests on proposed and actual materials

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r additional warranty requirements.

2. Free of organic matter, debris, and oversize particles (e.g., cobbles, rubble, etc. that are larger than 3 inches, rocks larger than 4 inches. Fill shall contain at least fifty percent of material smaller than 1/4 inch in size.

- 3. Imported fill materials: The soil shall be tested for potential contamination in accordance with DTSC-Clean Fill protocols. Submit to Geotechnical Engineer. a. Import sandy soil shall be free of organics, debris and oversize particles (e.g.,
- cobbles, rubble, etc. that are greater than 3 inches in the largest dimension). b. Additionally, import soils shall not have any corrosion impacts to buried concrete;
- and be non-expansive (Expansion Index less than 50 per ASTM D4829). c. Prior to import, geotechnical consultant shall evaluate and test the import soils in order to confirm the quality of the material.
- 4. On-site soils should only be used as specified in the Soils Report. 5. Complying with ASTM D2487 Group Symbol CL.
- C. Concrete for Fill: As specified in Section 03 30 00; compressive strength of 2500 psi.
- 1. Exception: Concrete used under footings and foundations to correct over-excavation shall be same as for footings and foundation.
- D. Granular Fill Fill Type GM, GW: Coarse aggregate, conforming to Uniform Standard Specifications for Public Works Construction Off-Site Improvements standard.
- 1. Grade in accordance with ASTM D2487 Group Symbol SP or SW.
- G. Topsoil: Topsoil excavated on-site. 1. Unclassified.
- a. The soil shall be tested for potential contamination in accordance with DTSC-Clean Fill protocols.
- 2. Graded.
- 3. Free of roots, rocks larger than 1/2 inch, subsoil, debris, large weeds and foreign matter. 4. Acidity range (pH) of 5.5 to 7.5.
- 5. Containing a minimum of 4 percent and a maximum of 25 percent inorganic matter.
- 6. Complying with ASTM D2487 Group Symbol OH.
- 7. Limit decaying matter to 5 percent of total content by volume. H. Type F - Subsoil: Reused, free of rocks larger than 3 inch size, and debris.
- 1. Existing fill and alluvium or older alluvium may be considered suitable for re-use as compacted fills provided the recommendations of the geotechnical report and
- observations of the geotechnical engineer are followed. 2. Expansive soils (EI>51) are not be placed with the upper 3 feet of subgrade soils
- 2.02 ACCESSORIES
- A. Geotextile Fabric: Non-biodegradable, non-woven; Geotex 801 manufactured by Propex Geotextile Systems, geotextile.com.

of soil material. B. Where fill materials are specified by reference to a specific standard, test and analyze samples for compliance before delivery to site. C. If tests indicate materials do not meet specified requirements, change material and retest. D. Provide materials of each type from same source throughout the Work. E. Comply with EPA/DTSC-Clean Fill requirements. PART 3 EXECUTION 3.01 EXAMINATION A. Verify structural or other backfill materials to be reused or imported are acceptable to the satisfaction of the Geotechnical Engineer. Approval shall be obtained in advance of re-use or importation onto the site. 1. The soil shall be tested for potential contamination in accordance with DTSC-Clean Fill protocols. 2. Provide imported fill materials compatible with on-site soils in addition to being suitable for its intended use with the following criterion, as allowed by the Geotechnical Engineer. a. Predominantly granular in nature. b. Containing no rocks larger than 6 inch maximum dimension. c. Free of organic material (loss on ignition less-than 2 percent). d. Very low expansion potential (with an Expansion Index less than 21). e. Low corrosion impact to the proposed improvements. B. Verify that survey bench marks and intended elevations for the Work are as indicated. C. Identify required lines, levels, contours, and datum locations. D. See Section 31 22 00 for additional requirements. E. Verify subdrainage, dampproofing, or waterproofing installation has been inspected. F. Verify structural ability of unsupported walls to support imposed loads by the fill. G. Verify areas to be filled are not compromised with surface or ground water. 3.02 PREPARATION A. Scarify and proof roll subgrade surface to a depth of 8 inches to identify soft spots. B. Cut out soft areas of subgrade not capable of compaction in place. Backfill with Greenbook, Type II or concrete fill and compact to density equal to or greater than requirements for subsequent backfill material. C. Compact subgrade to density equal to or greater than requirements for subsequent fill

2.03 SOURCE QUALITY CONTROL

- material.
- percent of its maximum dry density in accordance with ASTM D1557. E. Until ready to fill, maintain excavations and prevent loose soil from falling into excavation.

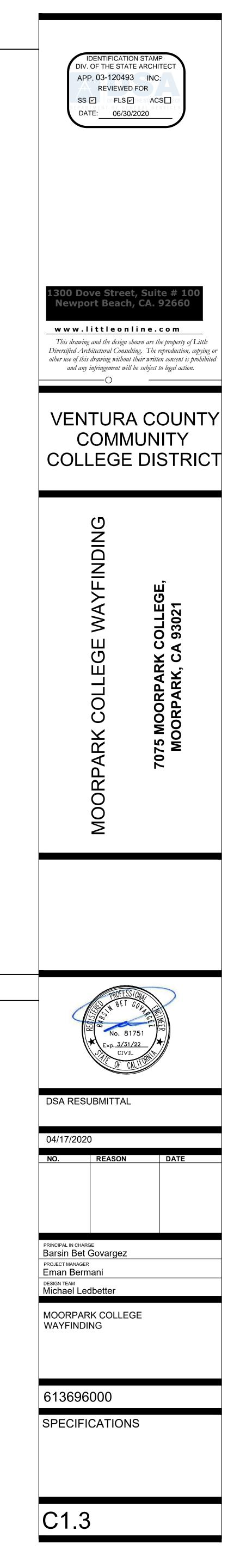
A. See Section 01 40 00 - Quality Requirements, for general requirements for testing and analysis

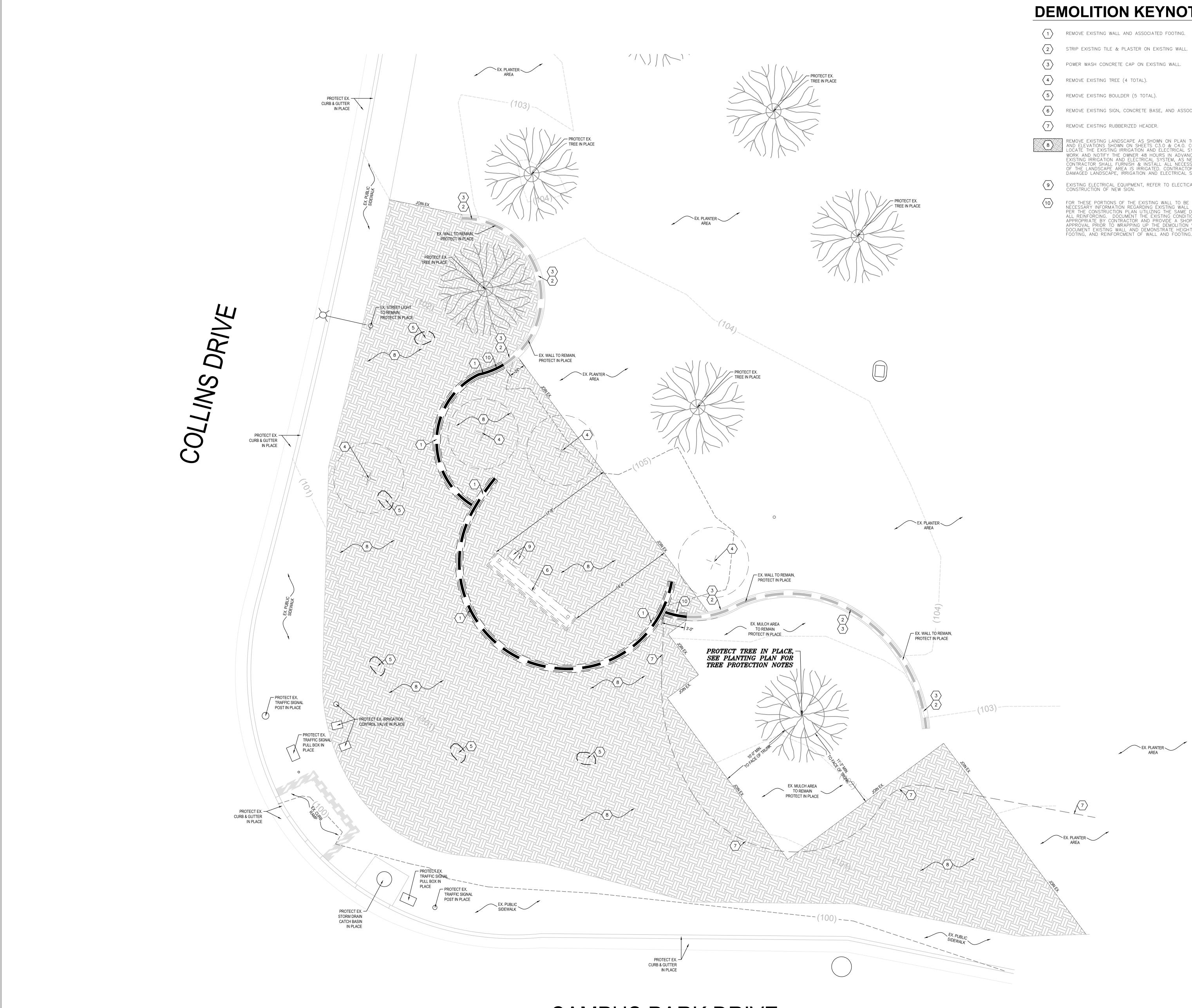
D. Prior to placement of aggregate base course material at paved areas, compact subsoil to 95

3.03 FILLING

A. Fill to contours and elevations indicated using unfrozen materials.

- B. Fill up to subgrade elevations unless otherwise indicated.
- 1. Place fill soils compacted in horizontal lifts to a relative compaction of 90 percent or more in general accordance with ASTM D1557.
- 2. Lift thickness for fill soils will vary depending on the type of compaction equipment used but should generally be placed in horizontal lifts not exceeding 8 inches in loose thickness.
- 3. Place fill soils at slightly above optimum moisture content as evaluated by ASTM D1557. 4. Avoid damage to wet and dry utility lines when compacting fill and subgrade materials.
- C. Employ a placement method that does not disturb or damage other work. 1. Do not disturb or damage foundation perimeter drainage and foundation waterproofing and protective cover utilities in trenches.
- D. Systematically fill and compact per geotechnical report. Do not fill over porous, wet, frozen or spongy subgrade surfaces.
- E. Maintain optimum moisture content of fill materials to attain required compaction density. F. Granular Fill: Place and compact materials in equal continuous layers not exceeding 6 inches compacted depth.
- G. Soil Fill: Place and compact material in equal continuous layers not exceeding 8 inches compacted depth. 1. Expansive soils (EI>20) are not be placed with the upper 3 feet of subgrade soils. CBC
- Section 1803.5.3. H. Slope grade away from building minimum 2 inches in 10 feet, unless noted otherwise. Make gradual grade changes. Blend slope into level areas.
- I. Correct areas that are over-excavated. 1. Load-bearing foundation surfaces: Fill with concrete.
- 2. Other areas: Use general fill, flush to required elevation, compacted to minimum 90 or 95 percent of maximum dry density in subgrade zone.
- J. Compaction Density Unless Otherwise Specified or Indicated: 1. Under paving, slabs-on-grade, and similar construction: 90 percent of maximum dry density.
- 2. At other locations: 90 percent of maximum dry density. K. Reshape and re-compact fills subjected to vehicular traffic.
- L. Maintain temporary means and methods, as required, to remove all water while fill is being placed as required, or until directed by the Architect. Remove and replace soils deemed unsuitable by classification and which are excessively moist due to lack of dewatering or surface water control.
- M. Remove surplus fill and backfill materials from site.
- 3.04 FILL AT SPECIFIC LOCATIONS
- A. Use general fill unless otherwise specified or indicated.





CAMPUS PARK DRIVE

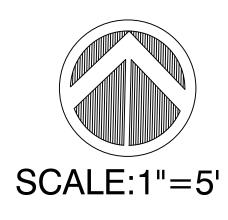
DEMOLITION KEYNOTES :

REMOVE	EXISTING	WALL	AND	ASSOCIATED	FOOTING.	
	REMOVE	REMOVE EXISTING	REMOVE EXISTING WALL	REMOVE EXISTING WALL AND	REMOVE EXISTING WALL AND ASSOCIATED	REMOVE EXISTING WALL AND ASSOCIATED FOOTING.

- 6 Remove existing sign, concrete base, and associated footing.

REMOVE EXISTING LANDSCAPE AS SHOWN ON PLAN TO ACCOMMODATE THE NEW IMPROVEMENTS AND ELEVATIONS SHOWN ON SHEETS C3.0 & C4.0. CONTRACTOR SHALL FIELD VERIFY AND LOCATE THE EXISTING IRRIGATION AND ELECTRICAL SYSTEM THAT IS IN CONFLICT WITH PROPOSED WORK AND NOTIFY THE OWNER 48 HOURS IN ADVANCE FOR WATER SHUT OFF. REMOVE/RELOCATE EXISTING IRRIGATION AND ELECTRICAL SYSTEM, AS NEEDED TO COMPLETE THE NEW WORK. CONTRACTOR SHALL FURNISH & INSTALL ALL NECESSARY MATERIAL TO ENSURE THE REMAINING OF THE LANDSCAPE AREA IS IRRIGATED. CONTRACTOR SHALL REMOVE AND REPLACE EXISTING DAMAGED LANDSCAPE, IRRIGATION AND ELECTRICAL SYSTEM LIKE FOR LIKE.

- 9 existing electrical equipment, refer to electical plans for demoltion and re-routing for construction of New Sign.
- FOR THESE PORTIONS OF THE EXISTING WALL TO BE REMOVED, CONTRACTOR SHALL DOCUMENT ALL NECESSARY INFORMATION REGARDING EXISTING WALL SO THAT SMALL PORTIONS OF IT CAN BE REBUILT PER THE CONSTRUCTION PLAN UTILIZING THE SAME DIMENSIONS FOR WALL AND FOOTINGS AS WELL AS ALL REINFORCING. DOCUMENT THE EXISTING CONDITION, BY VISUAL INSPECTION OR X-RAY AS DEEMED APPROPRIATE BY CONTRACTOR AND PROVIDE A SHOP DRAWING BACK TO ENGINEER FOR REVEIW AND APPROVAL PRIOR TO WRAPPING UP THE DEMOLITION WORK IN THIS AREA. SHOP DRAWING SHALL DOCUMENT EXISTING WALL AND DEMONSTRATE HEIGHT AND WIDTH OF WALL, DEPTH TO FOOTING, SIZE OF FOOTING, AND REINFORCMENT OF WALL AND FOOTING.

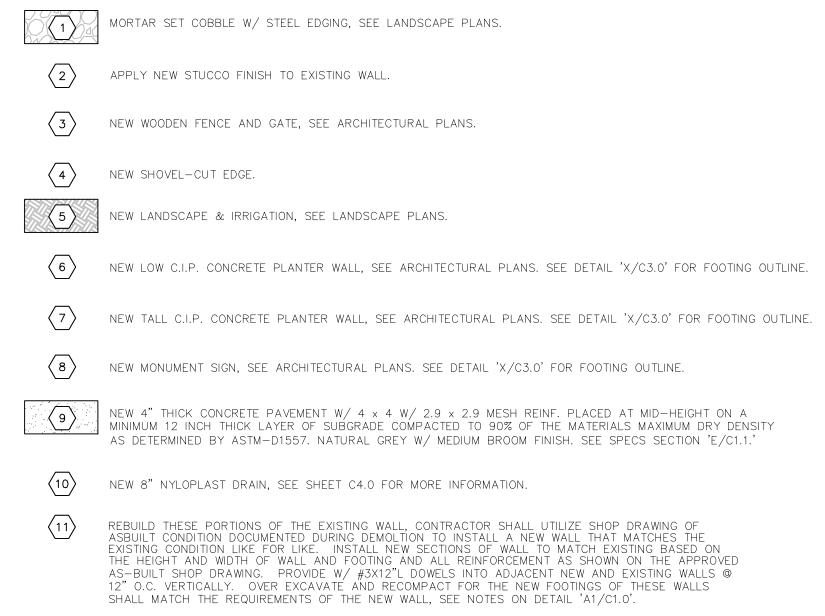


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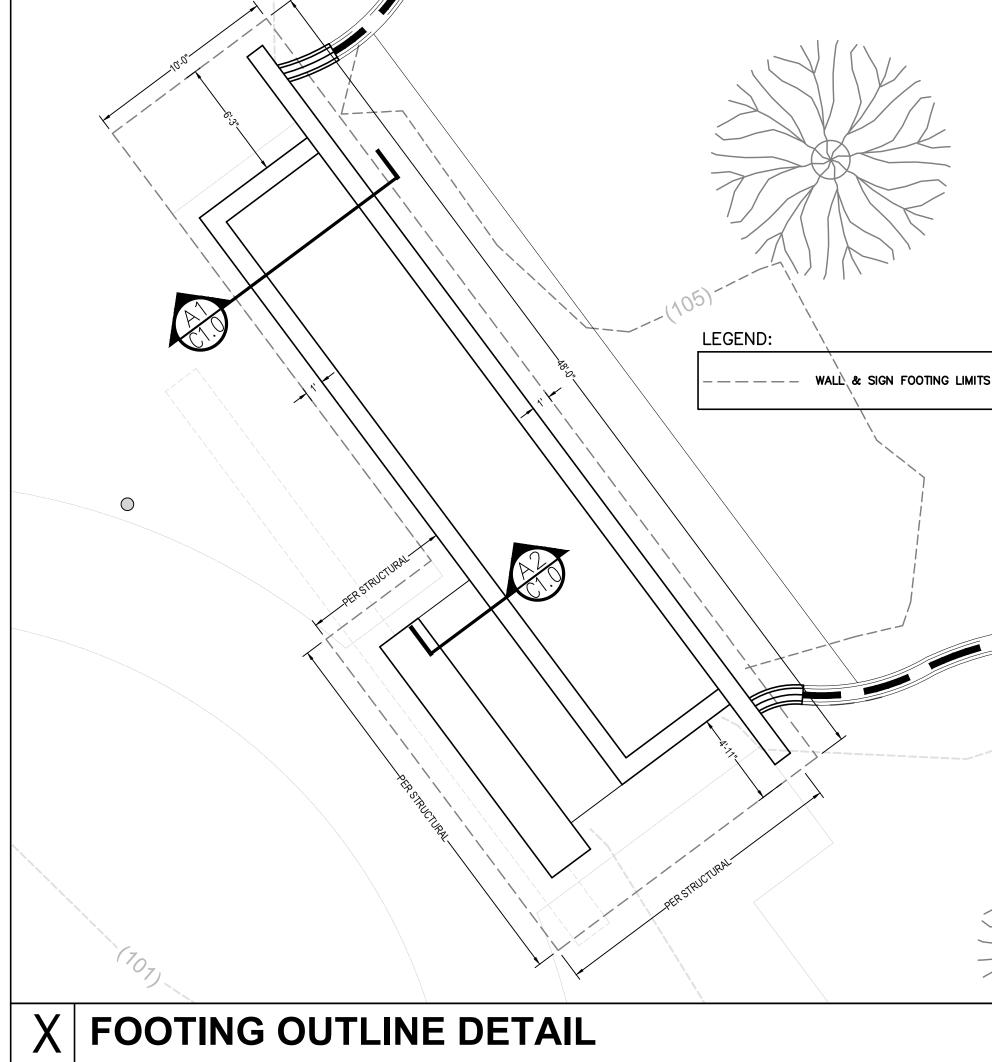


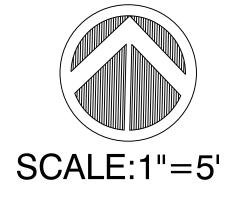
CAMPUS PARK DRIVE

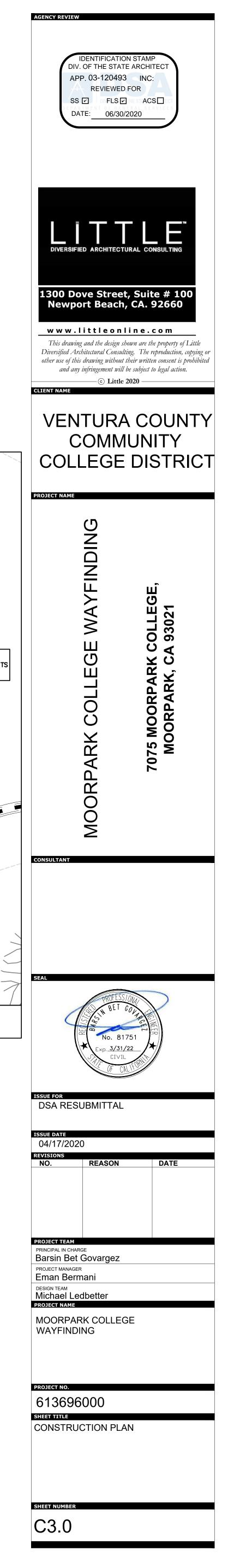
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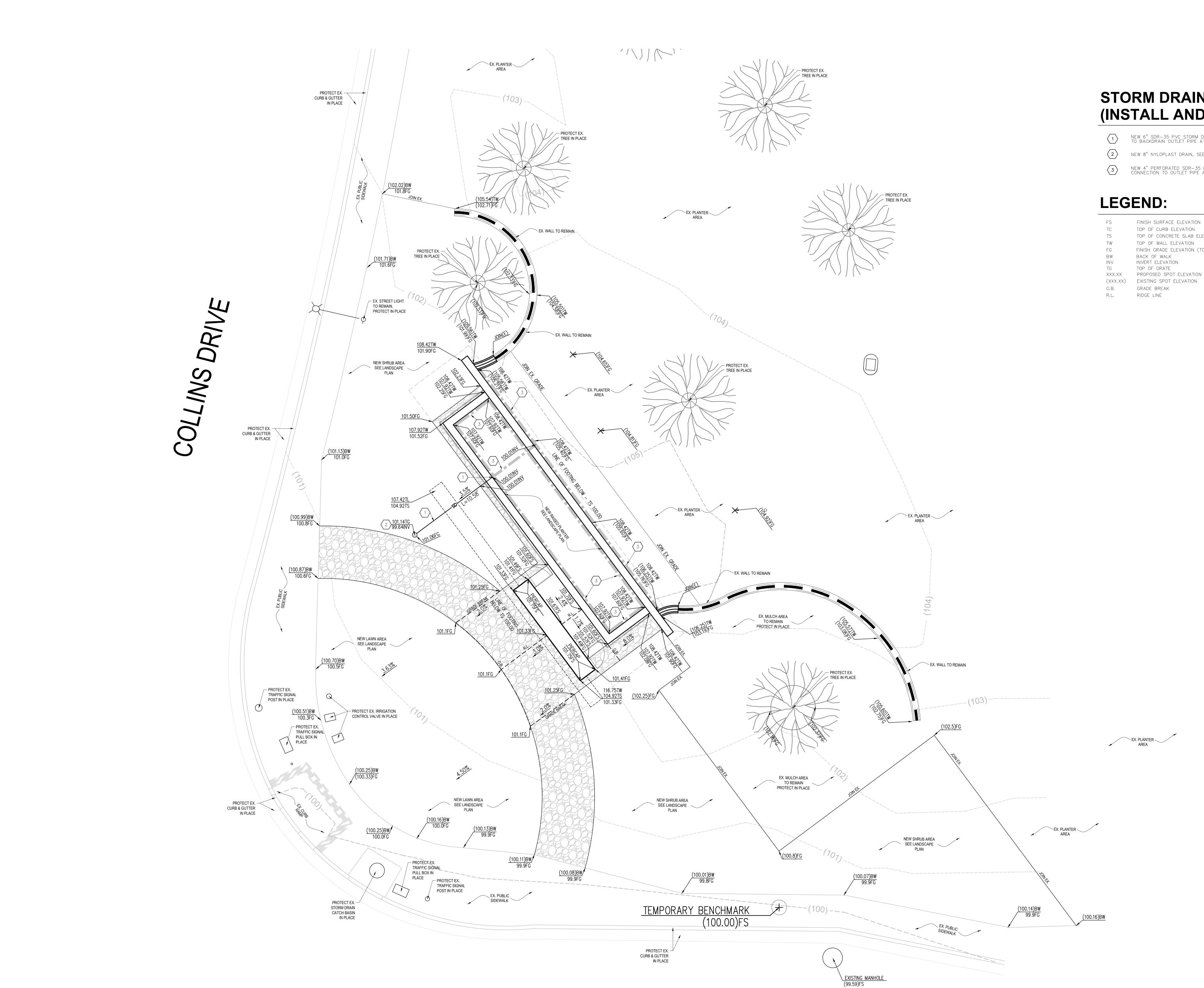










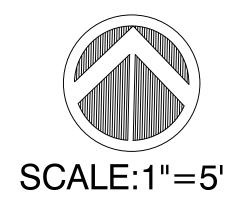


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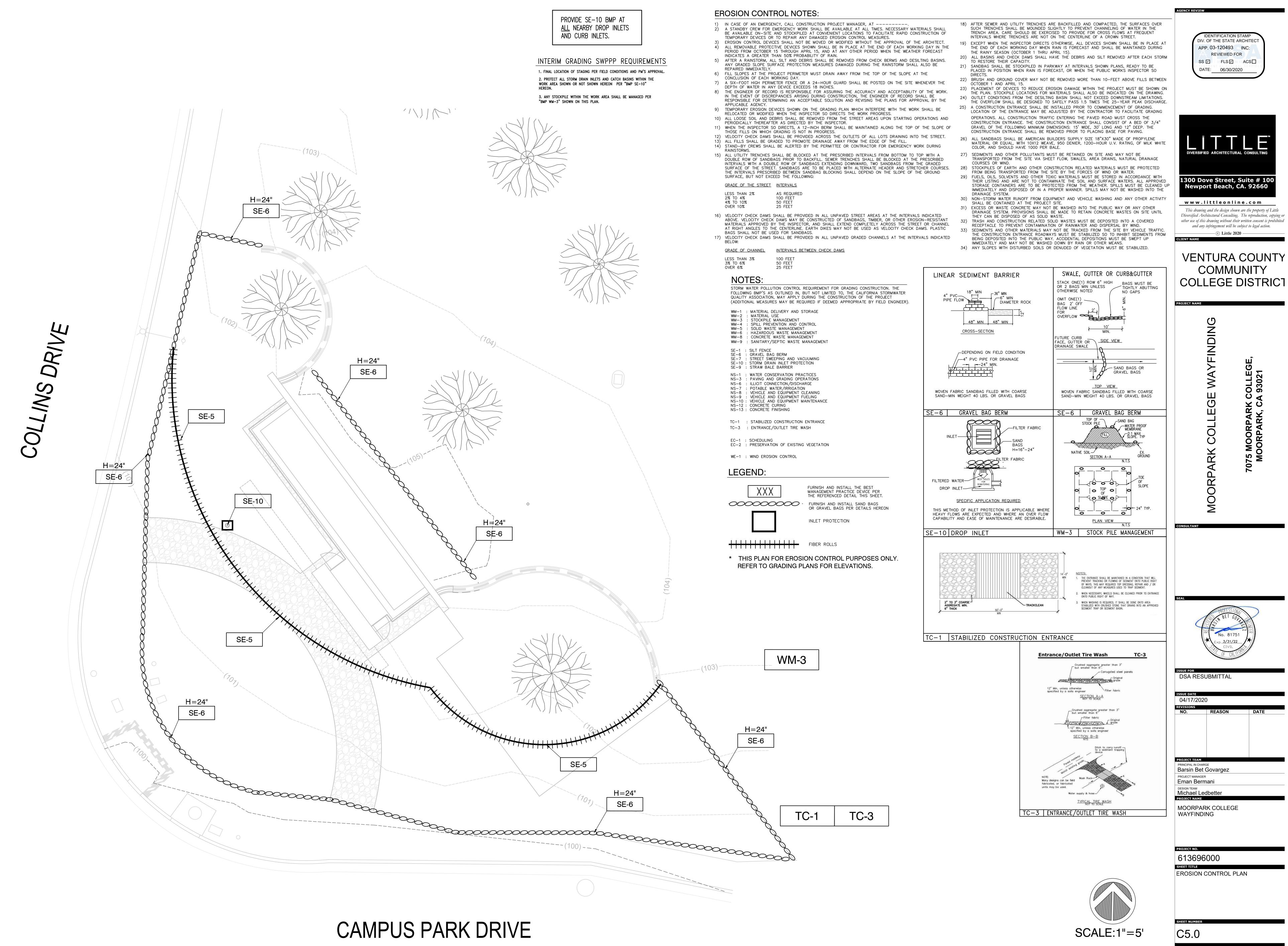
STORM DRAIN KEYNOTES (INSTALL AND FURNISH):

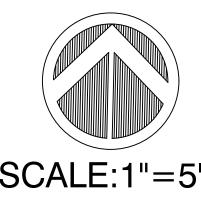
- NEW 6" SDR-35 PVC STORM DRAIN PIPE, SEE DETAIL 'A/C1.0' FOR CONNECTION TO BACKDRAIN OUTLET PIPE AT PLANTER WALL.
- $\left< 2 \right>$ New 8" nyloplast drain, see detail 'B/c1.0.'
- $\left< 3 \right>$ New 4" perforated SDR-35 pvc storm drain pipe, see detail 'A/C1.0' for connection to outlet pipe as shown.

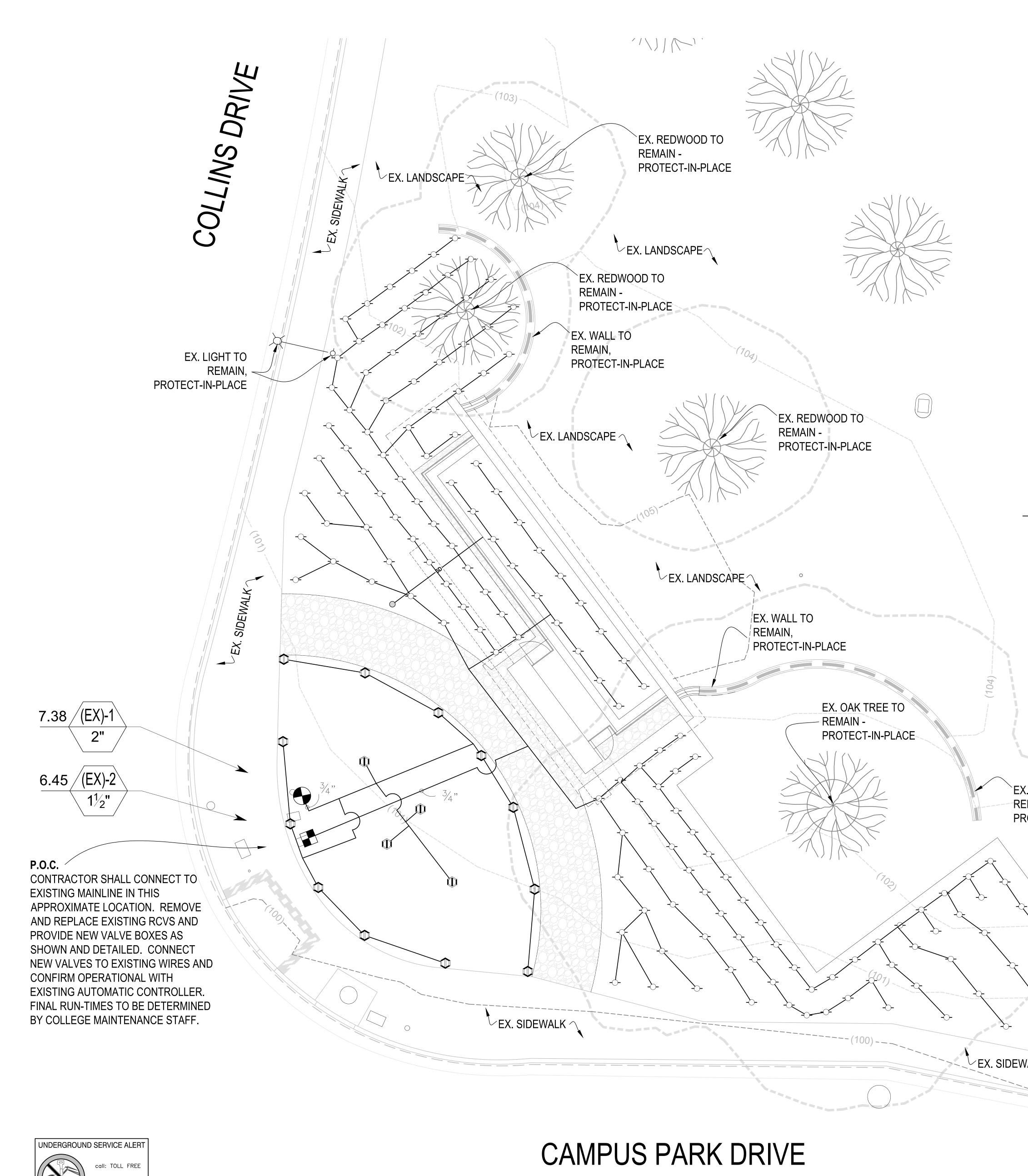
- FINISH SURFACE ELEVATION TOP OF CURB ELEVATION
- TOP OF CONCRETE SLAB ELEVATION TOP OF WALL ELEVATION
- FINISH GRADE ELEVATION (TOP OF DECORATIVE ROCK GROUT WHERE OCCURS) BACK OF WALK
- EXISTING SPOT ELEVATION



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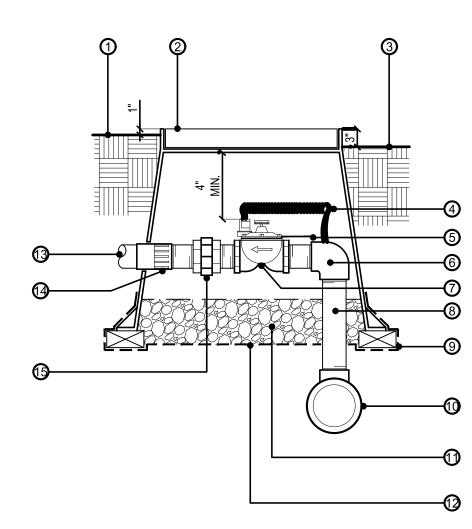
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IRRIGATION SCHEDULE

SYMBOL	MANUFACTURER/MOI	DEL/DESCRIPTION	<u>PSI</u>
ADJ. 360	Turf Rotator, 6.0" po floguard, pressure r Rotator nozzle on P	205-06-PRS40-CV-F op-up with check valve, egulated to 40 psi, MP RS40 body. ADJ=Orange 210), 360=Lime Green and	40
SYMBOL	MANUFACTURER/MOI	DEL/DESCRIPTION	
	with I" HYIOO filter s 40psi. Flow Range:	it. I-1/2" ICV Globe Valve bystem. Pressure Regulation: 20 GPM to 60 GPM. 120 screen. I-1/2" inlet x dual 1"	
-O- KO -O- 0.6 1.0 2.0	Bowsmith SL200 Series Single Outlet Emitter Single Outlet emitter, 0.6gph through 2.0gph flow, 1/2" FPT with barbed elbow outlet. Green=0.6gph, Blue=1.0gph, Red=2.0gph, Yellow=3.0gph.		
SYMBOL	MANUFACTURER/MOI	DEL/DESCRIPTION	
	Control Valves, Glob	Plastic Electric Remote be Configuration, with NPT t, for Commercial/Municipal	
	Valve Callout Valve Number Valve Flow Valve Size		
		TION NOTES:	
EX. LANDSCAPE	INTO 3. THE	THESE PLANS AND SHALL BE CARRIED OUT BY THE CONTRACTOR. CONTRACTOR IS EXPECTED TO SECURE COPIES OF THE CURRENT HITECTURAL AND ENGINEERING PLANS AND ANIMALIZE THEMSELVES	
	WITH	I ALL ASPECTS OF THE PROJECT AS IT RELATES TO THEIR SCOPE. THE CONTRACTOR'S RESPONSIBILITY TO SECURE ANY AND ALL	
EX. WALL TO REMAIN, PROTECT-IN-PLACE	5. IT IS ALL I COM APPF GUA AUTH FOUI 6. THE FOR INST ARE/	AITS REQUIRED TO PERFORM THEIR SCOPE OF WORK. THE CONTRACTOR'S RESPONSIBILITY TO VERIFY THE LOCATIONS OF EXISTING UTILITIES WITHIN THE LIMIT OF WORK PRIOR TO MENCING ANY WORK. LOCATIONS SHOWN ON THE PLANS ARE ROXIMATE AND THE LANDSCAPE ARCHITECT MAKES NO RANTEES ABOUT THEIR ACTUAL LOCATIONS. NOTIFY THE OWNER'S HORIZED REPRESENTATIVE IN THE EVENT DISCREPANCIES ARE ND BETWEEN THE PLANS AND CONDITIONS IN THE FIELD. IRRIGATION DESIGN SHOWN HERE-IN IS DIAGRAMMATIC AND SHOWN GRAPHIC CLARITY ONLY. ALL LATERALS, VALVES, ETC. SHALL BE ALLED WITHIN THE LIMIT OF WORK AND LOCATED IN LANDSCAPE AS WHERE EVER POSSIBLE. CONTRACTOR WILL BE EXPECTED TO	
-	PLAN 7. CON (HAR LEGE MINII BUNI	E ADJUSTMENTS IN THE FIELD TO AVOID CONFLICTS WITH PROPOSED ITING AND ARCHITECTURAL IMPROVEMENTS. TRACTOR SHALL INSTALL ALL PIPE UNDER PAVED AREAS DSCAPE, PARKING LOTS, ETC.) INSIDE SLEEVING AS SHOWN ON THE END AND SPECIFICATIONS. INSTALL PER DETAILS PROVIDED. AT A MUM, SLEEVES ARE TO BE 2X THE DIAMETER OF THE PIPE OR WIRE DLE CARRIED. SLEEVES SHALL EXTEND 6" MIN. PAST THE EDGE OF	
EX. LAN	IDSCAPE IDSCAPE	ED AREAS ABOVE. GATION HEADS SHALL BE INSTALLED WITH THE NOZZLE, SCREEN, ARCS SHOWN ON THE LEGEND. CONTRACTOR IS EXPECTED TO FORM MINOR ADJUSTMENTS IN THE FIELD TO LIMIT THE AMOUNT OF R-SPRAY ONTO ANY HARDSCAPE ELEMENT. WHERE OCCURS, AND O ADDITIONAL COST TO THE OWNER, CONTRACTOR IS HEREBY CTED TO REPLACE NOZZLES, SCREENS, ETC. WITH MORE ROPRIATE RADIUS EQUIPMENT TO BETTER FIT ACTUAL FIELD DITIONS ENCOUNTERED.	
	PERI OF A PRO OPEI	OT SHUT DOWN EXISTING SYSTEM LONGER THAN 48 HOURS IN ONE OD. NOTIFY THE SCHOOL DISTRICT AT LEAST 48 HOURS IN ADVANCE NY SUCH SHUTDOWNS. CONTRACTOR SHALL BE RESPONSIBLE TO /IDE TEMPORARY IRRIGATION TO MAINTAIN SYSTEM ACTIVE IN RATION TO EXISTING LANDSCAPE AREAS AFFECTED OUTSIDE THE JECT WORK AREA BY ANY SUCH SHUTDOWNS.	
DEWALK	CON	TAIN A SET OF 'AS-BUILT' DRAWINGS ON SITE DURING STRUCTION AND DELIVER TO ARCHITECT AND DISTRICT PROJECT AGER AT CLOSE OF PROJECT.	



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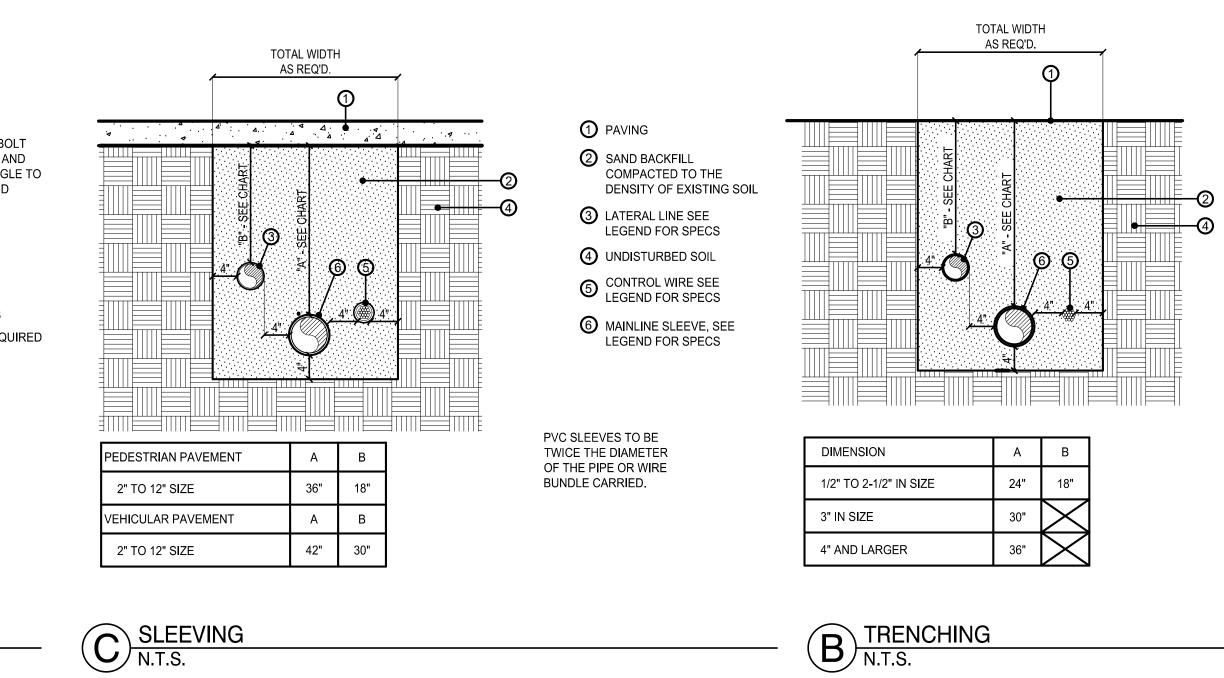


NOTES: a. ALL THREADED CONNECTIONS TO HAVE TEFLON TAPE OR PASTE. D REMOTE CONTROL VALVE N.T.S.

- 1 FINISH GRADE IN TURF AREAS
- PLASTIC RECTANGULAR VALVE BOX WITH BOLT DOWN COVER, USE STAINLESS BOLT, NUT, AND WASHER. BOX TO BE PLACED AT RIGHT ANGLE TO HARDSCAPE EDGE. HEAT BRAND "RCV" AND
- CONTROL STATION # ONTO LID.
- ③ FINISH GRADE IN SHRUB AREAS 4 24" WIRE LOOP
- 5 VALVE ID TAG
- 6 SCH. 80 PVC THREADED ELL
- O CONTROL VALVE, SEE LEGEND FOR SPECS
- 8 SCH 80 PVC NIPPLES (TYP). LENGTH AS REQUIRED
- (4) BRICK SUPPORTS
- IRRIGATION MAINLINE
- 3/4" ROCK GRAVEL 2 CUBIC FEET
- 12 LANDSCAPE FABRIC
- 13 LATERAL
- 🔞 SCH. 80 PVC FEMALE ADAP. 15 SCH. 80 UNION

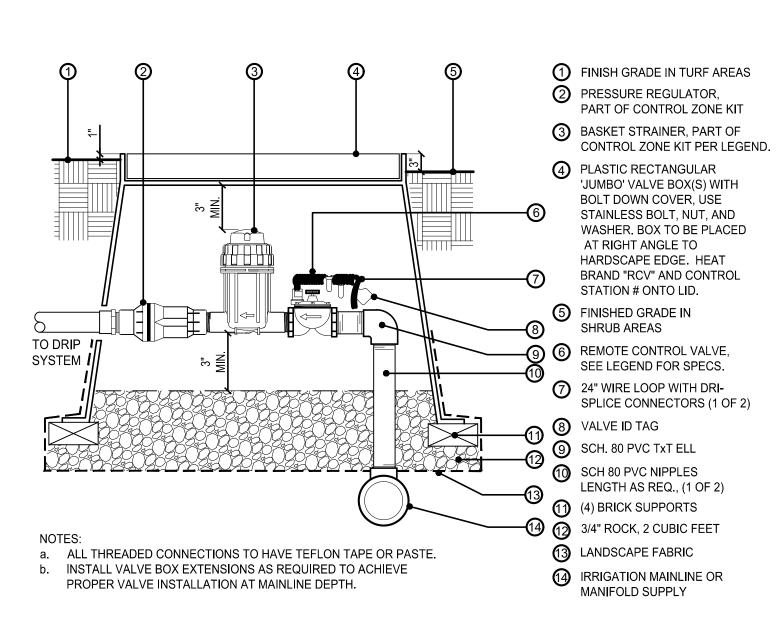


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1 FINISH GRADE

- CLEAN COMPACTED BACKFILL
- ${igidesigned}$ lateral line SEE plans and legend
- 4 UNDISTURBED SOIL
- 5 CONTROL WIRES, SEE SPECS.
- 6 MAINLINE SEE PLANS AND LEGEND



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SECTION 02810 - UNDERGROUND IRRIGATION SYSTEM PART 1 - GENERAL

1.1. RELATED DOCUMENTS:

1.1.1. DRAWINGS AND GENERAL PROVISIONS OF CONTRACT APPLY TO WORK OF THIS SECTION.

1.2. DESCRIPTION OF WORK:

1.2.1. THE WORK CONSISTS OF FURNISHING LABOR, TOOLS, MACHINERY, MATERIALS, AND PROCESSES REQUIRED TO COMPLETE THE IRRIGATION SYSTEM DESCRIBED HEREIN AND SHOWN ON THE DRAWINGS.

1.2.2. THE INTENT OF THE DRAWINGS AND SPECIFICATIONS IS TO INDICATE AND SPECIFY A COMPLETE IRRIGATION SYSTEM INSTALLED READY FOR USE WITHOUT FURTHER COST IN LABOR OR MATERIALS TO THE OWNER.

1.3. QUALITY ASSURANCE:

1.3.1. SUBCONTRACT WORK TO A SINGLE FIRM SPECIALIZING IN IRRIGATION WORK. CONTRACTOR SHALL POSSESS ALL LICENSES AND PERMITS REQUIRED PERFORMING THE WORK OF THIS CONTRACT INCLUDING A C-27 LANDSCAPING LICENSE.

1.4. SUBMITTALS:

1.4.1. THE CONTRACTOR SHALL FURNISH THE ARTICLES, EQUIPMENT, MATERIALS OR PROCESSES SPECIFIED BY NAME IN THE DRAWINGS AND SPECIFICATIONS. NO SUBSTITUTION WILL BE ALLOWED WITHOUT PRIOR WRITTEN APPROVAL BY THE LANDSCAPE ARCHITECT, OR THE OWNER'S AUTHORIZED REPRESENTATIVE.

1.4.2. THE CONTRACTOR SHALL SUBMIT TO THE LANDSCAPE ARCHITECT CATALOG DATA AND FULL DESCRIPTIVE LITERATURE FOR APPROVAL OF ITEMS DIFFERENT THAN THOSE SPECIFIED.

1.4.3. EQUIPMENT OR MATERIALS INSTALLED OR FURNISHED WITHOUT THE PRIOR APPROVAL OF THE LANDSCAPE ARCHITECT MAY BE REJECTED AND THE CONTRACTOR REQUIRED TO REMOVE SUCH MATERIALS FROM THE SITE AT HIS OWN EXPENSE.

1.4.4. APPROVAL OF ANY ITEM, ALTERNATE OR SUBSTITUTE INDICATES ONLY THAT THE PRODUCT(S) APPARENTLY MEET THE REQUIREMENTS OF THE DRAWINGS AND SPECIFICATIONS ON THE BASIS OF THE INFORMATION OR SAMPLES SUBMITTED.

1.4.5. MANUFACTURER'S WARRANTIES SHALL NOT RELIEVE THE CONTRACTOR OF HIS LIABILITY UNDER THE GUARANTEE. SUCH WARRANTY SHALL ONLY SUPPLEMENT THE GUARANTEE.

1.5. GUARANTEE:

1.5.1. FURNISH GUARANTEE IN ACCORDANCE WITH THE GENERAL CONDITIONS, FOR A PERIOD OF ONE (1) YEAR FROM THE DATE OF FINAL ACCEPTANCE - AT THE CONCLUSION OF THE MAINTENANCE PERIOD - A COMPLETE WATER IRRIGATION SYSTEM, INCLUDING NON-SETTLING OF THE BACKFILL IN TRENCHES WHICH, IF OCCURS, SHALL BE CORRECTED, INCLUDING REPAIRS AND/OR REPLACEMENT OF ANY MATERIAL DAMAGED THEREBY OR THEREFROM.

1.5.2. MANUFACTURER'S WARRANTIES SHALL NOT RELIEVE THE CONTRACTOR OF HIS LIABILITY UNDER THE GUARANTEE. SUCH WARRANTY SHALL ONLY SUPPLEMENT THE GUARANTEE.

PART 2 - PRODUCTS

2.1. MATERIALS:

2.1.1. PRESSURE PIPE: COMPLY WITH FOLLOWING:

2.1.1.1. PVC PLASTIC PRESSURE LINES: FOR PIPING UPSTREAM OF REMOTE CONTROL VALVES AND QUICK COUPLERS. ALL TWO (2) INCHES AND LARGER SHALL BE CLASS 315 POLYVINYL CHLORIDE (PVC) SIMPSON OR APPROVED EQUAL. ALL ONE AND ONE-HALF (1-1/2) INCHES AND SMALLER SHALL BE TYPE I, GRADE 2, DESIGNATED AS PVC 1220, SCHEDULE 40.

2.1.1.2. NON-PRESSURE PIPE: (DOWNSTREAM FROM REMOTE CONTROL VALVES): COMPLY WITH FOLLOWING:

2.1.2. PLASTIC NON-PRESSURE LINES: FOR PIPING DOWNSTREAM OF REMOTE CONTROL VALVES, TYPE 1, GRADE 2 (IMPACT MODIFIED), AS DESIGNATED AS PVC 1220, CLASS 200, (SDR21), CONFORMING TO COMMERCIAL STANDARDS CS256-63.

2.1.3. IDENTIFICATION: FURNISH PLASTIC PIPE CONTINUOUSLY AND PERMANENTLY MARKED WITH FOLLOWING INFORMATION: MANUFACTURER'S NAME OR TRADE MARK, SIZE, CLASS AND TYPE OF PIPE, WORKING PRESSURE AT 73.4 DEGREES F., AND NATIONAL SANITATION FOUNDATION (NSF) RATING.

PIPE FITTINGS AND CONNECTIONS: COMPLY WITH FOLLOWING:

2.2. FITTINGS AND CONNECTIONS:

2.2.1. POLYVINYL CHLORIDE PIPE FITTINGS AND CONNECTIONS: TYPE II, GRADE 1, SCHEDULE 40, HIGH IMPACT MOLDED FITTINGS, MANUFACTURED FROM VIRGIN COMPOUNDS AS SPECIFIED FOR PIPING TAPERED SOCKET OR MOLDED THREAD TYPE, SUITABLE FOR EITHER SOLVENT WELD OR SCREWED CONNECTIONS. MACHINE THREADED FITTINGS AND PLASTIC SADDLE AND FLANGE FITTINGS ARE NOT ACCEPTABLE. FURNISH FITTINGS PERMANENTLY MARKED WITH FOLLOWING INFORMATION: NOMINAL PIPE SIZE, TYPE AND SCHEDULE OF MATERIAL AND NATIONAL SANITATION FOUNDATION (NSF) SEAL OF APPROVAL. PVC FITTING SHALL CONFORM TO ASTM D2464 AND D2466 AND BE SUITABLE FOR NON-POTABLE WATER USE.

2.2.2. FLEXIBLE RISERS SHALL BE OF LINE SIZE IPS, PVC PLASTIC THREADED ADAPTERS SECURELY HELD TO APPROXIMATELY 4.6" LONG SYNTHETIC RUBBER OR FLEX-VINYL HOSE SHANKS, 85 POUND MINIMUM. KING BROS., EXCALIBRE OR EQUAL.

2.2.3. SOLVENT CEMENTS SHALL COMPLY WITH ASTM D2564. SOCKET JOINTS SHALL BE MADE PER RECOMMENDED PROCEDURES FOR JOINING PVC PLASTIC PIPE AND FITTINGS WITH PVC SOLVENT CEMENT BY THE PIPE AND FITTING MANUFACTURER AND PROCEDURES OUTLINED IN THE APPENDIX OF ASTM D2564.

2.2.4. THREAD LUBRICANT SHALL BE TEFLON RIBBON-TYPE, OR APPROVED EQUAL, SUITABLE FOR THREADED INSTALLATIONS AS PER MANUFACTURER'S RECOMMENDATIONS.

2.3. VALVES: MANUFACTURER'S STANDARD, OF TYPE AND SIZE INDICATED, AND AS FOLLOWS:

2.3.1. REMOTE CONTROL VALVES SHALL BE OF THE TYPE AND SIZE DESIGNATED ON THE

2.3.2. QUICK COUPLER VALVES (EXISTING)

2.3. SPRINKLER HEADS:

DRAWINGS.

2.3.1. SPRINKLER HEADS SHALL BE OF THE TYPES AND SIZES WITH DIAMETER (OR RADIUS) OF THROW, PRESSURE, NOZZLE DISCHARGE AND/OR OTHER DESIGNATIONS INDICATED ON THE DRAWINGS. ALL SPRINKLER HEADS OF THE SAME TYPE AND SIZE SHALL BE OF THE SAME MANUFACTURER.

2.4. VALVE BOXES

2.4.1. FOR REMOTE CONTROL VALVES: 10" X 19" NOM. PLASTIC VALVE BOX WITH TOP UNLESS NOTED OTHERWISE ON DRAWINGS.

UNDERGROUND SERVICE ALER



PART 3 - EXECUTION

3.1. SYSTEM DESIGN:

CONTRACT.

3.1.2. THE CONTRACTOR SHALL LOCATE AND MARK ALL EXISTING UTILITIES SUCH AS POWER, TELEPHONE, DOMESTIC WATER, WATER, AND TILE DRAINS. THE CONTRACTOR SHALL TAKE EXTREME CARE WHEN EXCAVATING OR WORKING IN THESE AREAS AND COORDINATION AND COOPERATION BETWEEN THE OWNER'S REPRESENTATIVE AND THE CONTRACTOR IS REQUIRED AS THE WORK PROGRESS TO THE AREA. CONTRACTOR SHALL GIVE 24 HOURS NOTICE TO REPRESENTATIVE AS WORK PROGRESSES TO UNDERGROUND UTILITY AREAS. CONTRACTOR SHALL BE RESPONSIBLE FOR DAMAGE TO ANY UTILITIES.

3.1.3. SHOULD UTILITIES NOT LOCATED OR MARKED BE FOUND DURING EXCAVATION, THE CONTRACTOR SHALL PROMPTLY NOTIFY THE OWNER AND SHALL DISCONTINUE WITH WORK IN THE AREA, EXCEPT NECESSARY EMERGENCY WORK, TO REPAIR OR PREVENT DAMAGE UNTIL INSTRUCTIONS ARE GIVEN TO THE CONTRACTOR BY THE OWNER'S REPRESENTATIVE.

3.1.4. FAILURE TO NOTIFY THE OWNER OF DISCOVERY OF SUCH UTILITIES OR DAMAGE THERETO WILL RESULT IN THE CONTRACTOR BEING LIABLE FOR ANY AND ALL DAMAGE CAUSED TO THE UTILITIES AS A RESULT OF HIS ACTIONS.

3.1.5. THE CONTRACTOR SHALL, BEFORE STARTING WORK ON THE IRRIGATION SYSTEM, CAREFULLY NOTE ALL FINISH GRADES IN ORDER TO SATISFY HIMSELF THAT HE MAY PROCEED WITH THE WORK AND TO RESTORE FINISH GRADES TO ORIGINAL CONTOURS BEFORE COMPLETION.

3.1.6. THE INSTALLATION OF ALL IRRIGATION MATERIALS, INCLUDING PIPE, SHALL BE COORDINATED WITH THE LANDSCAPE DRAWINGS TO AVOID INTERFERING WITH THE TREES, SHRUBS, OR OTHER PLANTING.

3.1.7. LAY OUT SPRINKLER HEADS AND MAKE ANY MINOR ADJUSTMENTS REQUIRED DUE TO DIFFERENCE BETWEEN SITE AND DRAWINGS. ANY SUCH DEVIATIONS IN LAYOUT SHALL BE WITHIN THE INTENT OF THE ORIGINAL DRAWINGS, AND WITHOUT ADDITIONAL COST TO THE OWNER. WHEN DIRECTED BY THE OWNER, THE LAYOUT SHALL BE APPROVED BEFORE INSTALLATION.

3.1.8. DO NOT WILLFULLY INSTALL THE SPRINKLER SYSTEM AS INDICATED ON THE DRAWING WHEN IT IS OBVIOUS IN THE FIELD THAT PREVIOUSLY UNKNOWN OBSTRUCTIONS OR GRADE DIFFERENCES EXIST, THAT MIGHT NOT HAVE BEEN CONSIDERED IN THE ENGINEERING. SUCH OBSTRUCTIONS OR DIFFERENCES SHOULD BE BROUGHT TO THE ATTENTION OF THE LANDSCAPE ARCHITECT.

3.1.9. WATER SUPPLY: THE CONTRACTOR SHALL CONNECT TO THE EXISTING IRRIGATION SYSTEM AS INDICATED ON THE DRAWINGS.

3.1.10. WORKMANSHIP AND PROCEDURE: THE ROUTING OF THE PRESSURE SUPPLY LINES AS INDICATED ON THE DRAWINGS IS DIAGRAMMATIC. LOCATE ALL PRESSURE SUPPLY LINES IN PLANTING AREAS. CROSS PERPENDICULAR UNDER PAVEMENT IN A SLEEVE AS DESCRIBED IN THESE SPECIFICATIONS.

3.2. INSTALLATION:

3.2.1. GENERAL: UNLESS OTHERWISE INDICATED, COMPLY WITH REQUIREMENTS OF UNIFORM PLUMBING CODE.

3.2.1.1. EXCAVATION OF TRENCHES: EXCAVATE TRENCHES, PREPARE SUB-GRADE AND BACKFILL TO LINE AND GRADE WITH SUFFICIENT ROOM FOR PIPES FITTINGS, TESTING AND INSPECTING OPERATIONS. DO NOT BACKFILL UNTIL THE PIPE SYSTEM HAS BEEN SUBJECTED TO A HYDROSTATIC TEST AS SPECIFIED.

3.2.1.2. DEPTH OF TRENCH:

3.2.1.2.2. POLYVINYL CHLORIDE NON-PRESSURE LINE....12" MIN.

3.2.1.3. SUBSOIL SHALL BE FREE OF ALL ROCKS OVER ONE (1) INCH DIAMETER, DEBRIS, AND LITTER PRIOR TO USE AS BACKFILL.

3.2.1.4. REPAIR ANY LEAKS AND REPLACE ALL DEFECTIVE PIPE OR FITTINGS UNTIL LINES MEET TEST REQUIREMENTS. DO NOT COVER ANY LINES UNTIL THEY HAVE BEEN CHECKED AND APPROVED FOR TIGHTNESS, QUALITY OF WORKMANSHIP AND MATERIALS.

3.2.1.5. BACKFILL TRENCHES, AFTER APPROVAL OF PIPING, WITH SUITABLE AND APPROVED MATERIAL, TAMPING SOIL AROUND PIPE AND THOROUGHLY COMPACTING ALL TRENCH FILLS UNTIL 90% COMPACTION HAS BEEN ACHIEVED.

3.2.1.6. BACKFILL MATERIAL SHALL BE AN APPROVED SOIL, FREE FROM ROCKS AND CLODS. PROVIDE BACKFILL UNDER, AROUND AND ABOVE TOP OF PIPE FOR PVC PLASTIC PIPE AND BRASS PIPING.

3.2.2. INSTALLATION OF POLYVINYL CHLORIDE PIPE:

3.2.2.1. BECAUSE OF THE NATURE OF PLASTIC PIPE AND FITTINGS, EXERCISE CAUTION IN HANDLING, LOADING AND STORING, TO AVOID DAMAGE.

3.2.2.2. THE PIPE AND FITTINGS SHALL BE STORED UNDER COVER UNTIL USING, AND SHALL BE TRANSPORTED IN A VEHICLE WITH A BED LONG ENOUGH TO ALLOW THE LENGTH OF PIPE TO LAY FLAT SO AS NOT BE SUBJECTED TO UNDUE BENDING OR CONCENTRATED EXTERNAL LOAD AT ANY POINT.

3.1.1. ALL SCALED DIMENSIONS ARE APPROXIMATE. THE CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS ON THE SITE PRIOR TO PROCEEDING WITH WORK UNDER THIS

3.2.2.3. ANY PIPE THAT HAS BEEN DENTED OR DAMAGED SHALL BE DISCARDED UNLESS SUCH DENT OR DAMAGED SECTION IS CUT OUT AND PIPE REJOINED WITH A COUPLING.

3.2.2.4. TRENCH DEPTH SHALL BE AS SPECIFIED ABOVE FROM THE FINISH GRADE TO THE TOP OF THE PIPE. THE BOTTOM OF THE TRENCH SHALL BE FREE OF ROCKS, CLODS, AND OTHER SHARP-EDGED OBJECTS.

3.2.2.5. PIPE ENDS AND FITTINGS SHALL BE WIPED WITH "MEK" PRIMER, OR APPROVED EQUAL BEFORE WELDING SOLVENT IS APPLIED. WELDED JOINTS SHALL BE GIVEN A MINIMUM OF 15 MINUTES TO SET BEFORE MOVING OR HANDLING. ALL FIELD CUTS SHALL BE BEVELED TO REMOVE BURRS AND EXCESS MATERIAL BEFORE FITTING AND GLUING TOGETHER.

3.2.2.6. PIPE SHALL BE SNAKED FROM SIDE-TO-SIDE OF TRENCH BOTTOM TO ALLOW FOR EXPANSION AND CONTRACTION.

3.2.2.7. CENTER LOAD PIPE WITH SMALL AMOUNT OF BACKFILL TO PREVENT ARCHING AND SLIPPING UNDER PRESSURE. LEAVE JOINTS EXPOSED FOR SITE OBSERVATION DURING TESTING.

3.2.2.8. NO WATER SHALL BE PERMITTED IN THE PIPE UNTIL SITE OBSERVATION HAS BEEN COMPLETED AND A PERIOD OF AT LEAST 24 HOURS HAS ELAPSED FOR SOLVENT WELD SETTING AND CURING.

3.2.2.9. PLASTIC TO METAL JOINTS SHALL BE MADE WITH PLASTIC MALE ADAPTERS, METAL NIPPLE HAND TIGHTENED, PLUS ONE TURN WITH A STRAP WRENCH.

3.2.2.10. PLASTIC TO PLASTIC JOINTS: SOLVENT-WELD, USING SOLVENT RECOMMENDED BY PIPE MANUFACTURER ONLY.

3.2.2.11. SOLVENT-WELD JOINTS: ASSEMBLE PER MANUFACTURER'S RECOMMENDATIONS. 3.2.3. NOT USED

3.2.4. REMOTE CONTROL WIRING:

3.2.4.1. DIRECT BURIAL CONTROL WIRE SIZES: AS SHOWN AND SPECIFIED HEREIN BEFORE.

3.2.4.2. PROVIDE ONE CONTROL WIRE AND ONE COMMON GROUND WIRE TO SERVICE EACH VALVE IN SYSTEM. PROVIDE 4-FOOT MINIMUM EXPANSION LOOP AT EACH VALVE TO PERMIT REMOVAL AND MAINTENANCE OF VALVES.

3.2.4.3. INSTALL CONTROL WIRES AT LEAST 12" BELOW FINISH GRADE AND MINIMUM OF 4" FROM ANY PIPE OR FITTINGS EXCEPT AT TERMINAL POINTS.

3.2.4.4. INSTALL CONTROL WIRES AND IRRIGATION PIPING IN COMMON TRENCHES WHEREVER POSSIBLE.

3.2.4.5. IN CASE OF DAMAGE TO ANY COMMON OR CONTROL WIRE, CONTRACTOR IS TO RUN AN EXTRA COMMON AND CONTROL WIRE ON EACH LEG OF MAINLINE TO THE FARTHEST RCV BACK TO THE CONTROLLER.

3.2.4.6. CONTROL WIRE SPLICES: ALLOW ONLY ON RUNS OF MORE THAN 300-FEET. SPLICES AS FOLLOWS:

3.2.4.6.1. STRIP OFF MINIMUM OF 2-1/2" OF INSULATION FROM EACH WIRE.

3.2.4.6.2. TWIST ON SCOTCHLOK ELECTRICAL SPRING CONNECTOR, MINIMUM FOUR COMPLETE TURNS.

3.2.4.6.3. SEAL CONNECTOR IN EPOXY RESIN.

3.2.4.6.4. TAPE COMPLETED SPLICE WITH SCOTCH 33 ELECTRICAL TAPE.

3.2.4.7. NUMBERING AND TAGGING: IDENTIFY DIRECT BURIAL CONTROL WIRES FROM AUTOMATIC VALVES TO TERMINAL STRIPS OF CONTROLLER AT TERMINAL STRIP BY TAGGING WIRE WITH NUMBER OF CONNECTED VALVES.

3.2.6. REMOTE CONTROL VALVES:

3.2.6.1. INSTALL REMOTE CONTROL VALVES IN LOCATIONS APPROXIMATELY AS SHOWN ON THE DRAWINGS, WITH A COVER OF 8 INCHES MINIMUM OVER TOP OF FLOW CONTROL STEM. INSTALL A UNION TYPE CONNECTION. FIT WITH PLASTIC VALVE BOX AND COVER.

3.2.6.2. EACH REMOTE CONTROL VALVE SHALL BE TAGGED WITH "CHRISTY TAG" INDICATING CONTROLLER NUMBER AND SEQUENCE NUMBER. 3.2.7. VALVE BOX:

3.2.7.1. INSTALL VALVE BOXES AS SHOWN ON DETAIL. INSTALL NO MORE THAN ONE VALVE PER BOX.

3.2.7.2. VALVE BOX LIDS SHALL BE AND BRANDED AS SPECIFIED ON DETAILS.

3.2.8. SPRINKLER HEADS:

3.2.8.1. ALL SPRINKLER HEADS SHALL BE INSTALLED AS PER DETAILS SHOWN.

3.2.8.2. NOZZLE SIZE OF ALL HEADS SHALL BE ADJUSTED TO SUIT ANY PARTICULAR CONDITIONS OF THE AREA. THIS SHALL BE DONE AFTER THE SYSTEM HAS BEEN THOROUGHLY TESTED, IMMEDIATELY AFTER WRITTEN NOTIFICATION BY THE LANDSCAPE ARCHITECT TO DO SO.

3.2.9. QUICK COUPLER ASSEMBLY: (EXISTING, P.I.P.)

3.2.10. FLUSHING OF SYSTEMS:

3.2.10.1. AFTER PIPING AND RISERS ARE IN PLACE, BUT PRIOR TO THE INSTALLATION OF THE SPRINKLER HEADS, A FULL HEAD OF WATER SHALL BE USED TO FLUSH OUT THE SYSTEM. AFTER SYSTEM IS THOROUGHLY FLUSHED, CAP ALL RISERS.

3.2.10.2. CONNECTION TO MAIN: CONNECT TO EXISTING IRRIGATION SYSTEM SERVING OVERALL CAMPUSE IN APPROXIMATE LOCATION AS INDICATED ON THE PLANS.

3.2.11. TESTING:

3.2.11.1. GENERAL: NOTIFY ARCHITECT/ENGINEER IN WRITING WHEN TESTING WILL BE CONDUCTED. CONDUCT TESTS IN PRESENCE OF ARCHITECT/ENGINEER.

3.2.11.2. PRESSURE TEST

3.2.11.2.1.ALL PRESSURE LINES SHALL BE TESTED UNDER HYDROSTATIC PRESSURE OF 125 LBS. PER SQUARE INCH AND ALL NON-PRESSURE LINES SHALL BE TESTED UNDER THE EXISTING STATIC PRESSURE AND BOTH ARE PROVEN WATERTIGHT. (CONTRACTOR TO SUPPLY ALL EQUIPMENT NEEDED FOR TESTING.)

3.2.11.2.2.PRESSURE SHALL BE SUSTAINED IN THE LINES FOR NOT LESS THAN FOUR HOURS IF LEAKS DEVELOP, THE JOINTS SHALL BE REPLACED AND THE TEST REPEATED UNTIL THE ENTIRE SYSTEM IS PROVEN WATERTIGHT.

3.2.11.2.3.TESTS SHALL BE OBSERVED AND APPROVED BY THE LANDSCAPE ARCHITECT AND/OR OWNER PRIOR TO BACKFILL. BACKFILLING TRENCHES PRIOR TO INSPECTION WILL NOT BE ALLOWED AND ALL PREMATURELY FILLED TRENCHES SHALL BE SUBJECT TO REOPENING AS DIRECTED BY THE LANDSCAPE ARCHITECT.

3.2.11.3. COVERAGE TESTING: PERFORM OPERATIONAL TESTING AFTER HYDROSTATIC TESTING IS COMPLETED, BACKFILL IS IN PLACE, AND SPRINKLER HEADS ADJUSTED TO FINAL POSITION.

3.2.11.4. AFTER COMPLETION OF LANDSCAPE WORK, CAREFULLY ADJUST HEADS SO THEY WILL BE FLUSH WITH LAWN AREAS OR NOT MORE THAN 1/2" ABOVE FINISH GRADE IN GROUNDCOVER AREA.

3.3. SITE OBSERVATION VISITS BY THE ARCHITECT:

3.3.1. IN ALL CASES WHERE SITE OBSERVATION VISITS OF THE IRRIGATION SYSTEM WORK IS REQUIRED AND/OR WHERE PORTIONS OF THE WORK ARE SPECIFIED TO BE PERFORMED UNDER THE DIRECTION AND/OR SITE OBSERVATION OF THE ARCHITECT OR HIS REPRESENTATIVE, THE CONTRACTOR SHALL NOTIFY THE ARCHITECT AT LEAST THREE (3) WORKING DAYS IN ADVANCE OF THE TIME SUCH SITE OBSERVATION AND/OR DIRECTION IS REQUIRED.

3.3.2. SITE OBSERVATION WILL BE REQUIRED FOR THE FOLLOWING PARTS OF THE WORK:

3.3.2.1. FINAL SITE OBSERVATION VISIT BY THE ARCHITECT AND PERFORMANCE TEST SHALL BE AT THE SAME TIME AS THE FINAL SITE OBSERVATION OF THE SPECIFIED LANDSCAPE MAINTENANCE PERIOD WORK.

3.4. RECORD DRAWINGS:

3.4.1. BEFORE FINAL ACCEPTANCE OF WORK, THE CONTRACTOR SHALL PROVIDE A RECORD SET OF DRAWINGS SHOWING THE SPRINKLER SYSTEM WORK. INFORMATION SHALL BE ON BROWN LINE SEPIA TRANSPARENCIES FOR REPRODUCTION PURPOSES.

3.4.2. ANY CHANGES IN LOCATION OF ITEMS OR TYPE OF INSTALLATIONS FROM THAT SHOWN ON DRAWINGS SHALL BE SO INDICATED ON THE RECORD DRAWINGS.

3.4.3. VALVES SHALL BE NUMBERED AND CORRESPONDING NUMBERS SHALL BE SHOWN ON THE RECORD DRAWINGS.

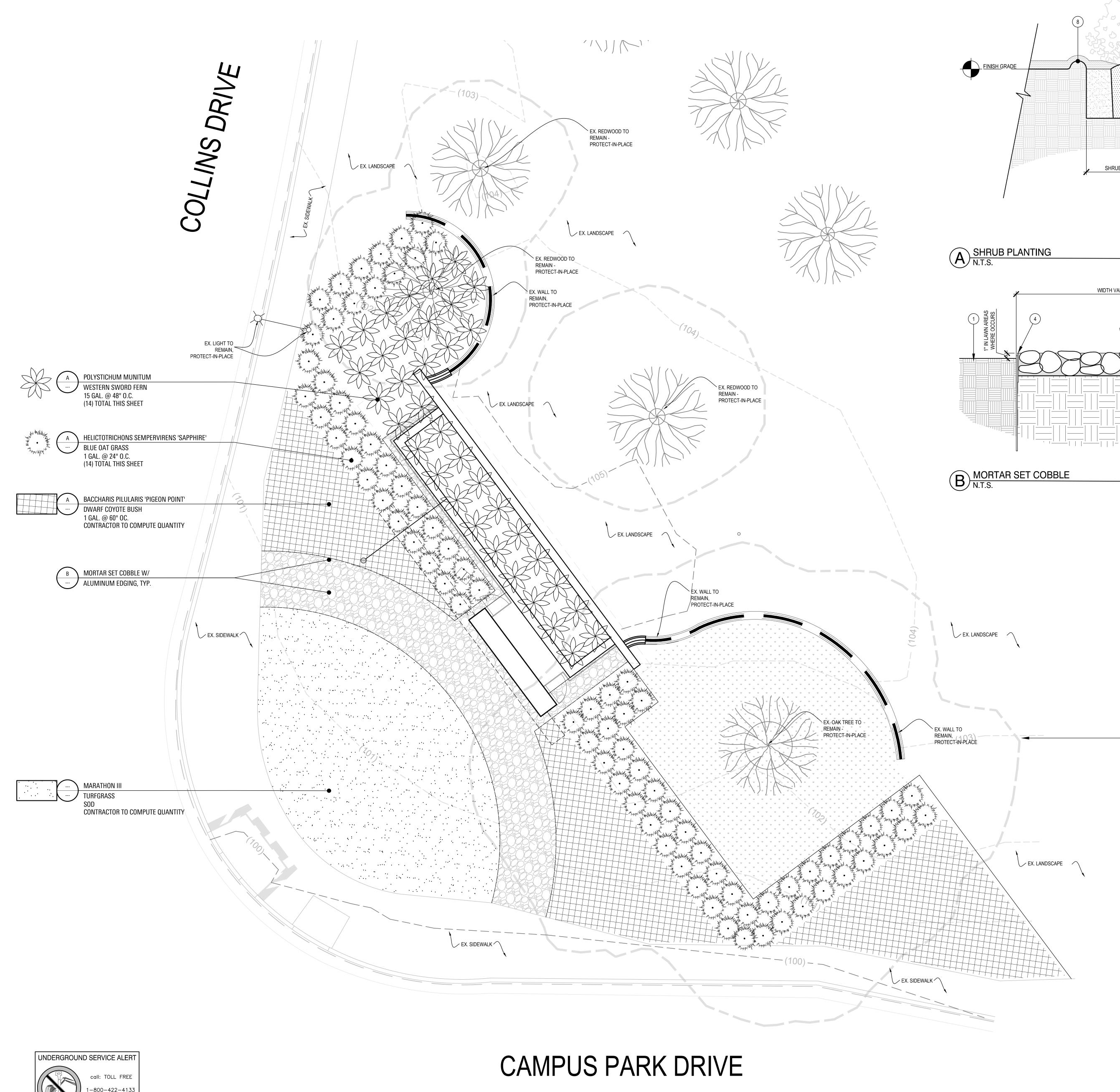
3.4.4. ALL REMOTE CONTROL VALVES, SHUT-OFF VALVES, QUICK COUPLER VALVES SHALL BE LOCATED BY MEASURED DIMENSIONS DIMENSIONS SHALL BE GIVEN TO PERMANENT OBJECTS AND SHALL BE TO THE NEAREST ONE-HALF FOOT.

3.4.5. ON THE INSIDE SURFACE OF THE COVER OF EACH AUTOMATIC CONTROLLER, PREPARE AND MOUNT A COLOR-CODED CHART SHOWING THE VALVES, MAINLINE, AND SPRINKLER HEADS SERVICED BY THAT PARTICULAR CONTROLLER. ALL VALVES SHALL BE NUMBERED TO MATCH THE OPERATION SCHEDULE AND THE DRAWINGS. ONLY THOSE AREAS CONTROLLED BY THAT CONTROLLER SHOULD BE SHOWN. THIS CHART SHALL BE A PLOT PLAN, ENTIRE OR PARTIAL, SHOWING BUILDING, WALKS, ROADS AND WALLS. A PHOTOSTATIC PRINT OF THIS PLAN, REDUCED AS NECESSARY AND LEGIBLE IN ALL DETAILS, SHALL BE MADE TO A SIZE THAT WILL FIT INTO THE CONTROLLER COVER. THIS PRINT SHALL BE APPROVED BY THE ARCHITECT AND SHALL BE HERMETICALLY SEALED BY PLASTIC. THIS PLAN SHALL THEN BE SECURED TO THE BACK OF THE ENCLOSURE DOOR.

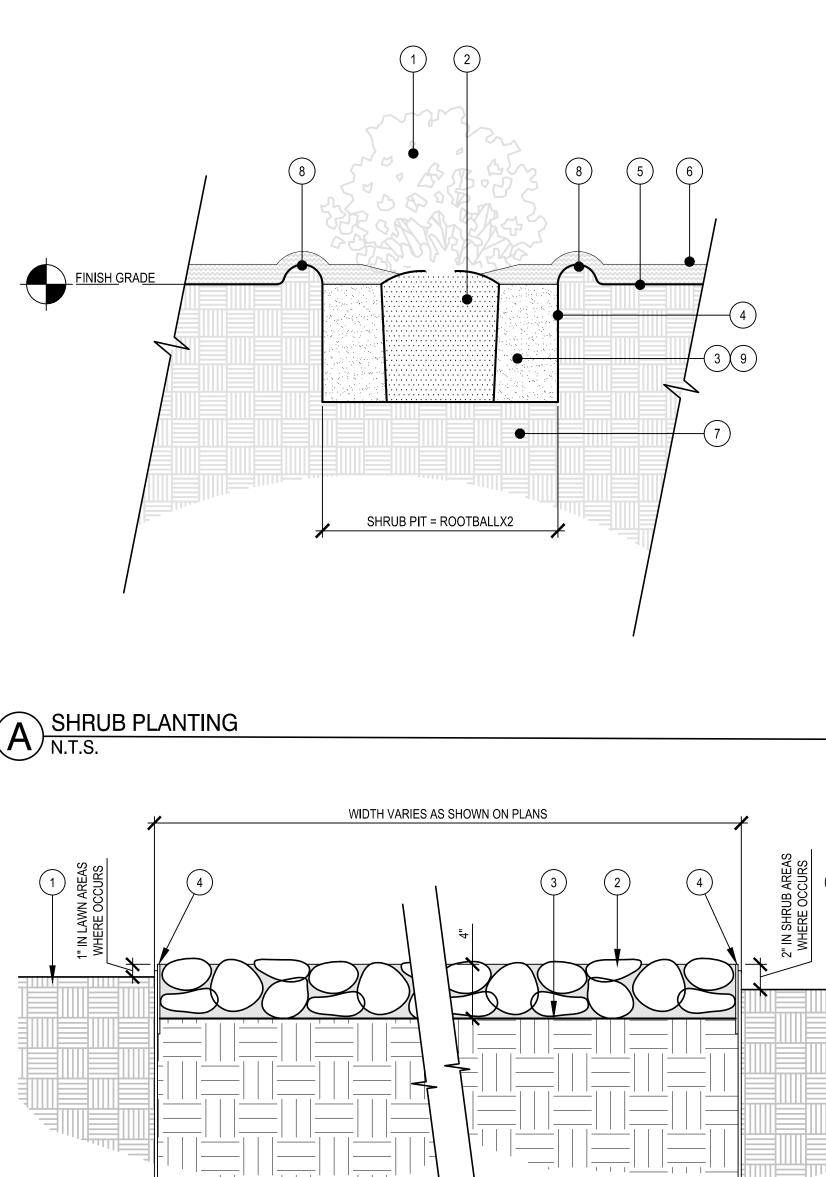
3.4.6. IMMEDIATELY UPON THE INSTALLATION OF ANY BURIED PIPE OR EQUIPMENT, THE CONTRACTOR SHALL INDICATE ON THE DRAWINGS THE LOCATIONS OF SAID EQUIPMENT. DIMENSIONS SHALL BE GIVEN FROM PERMANENT OBJECTS SUCH AS BUILDINGS, SIDEWALKS, CURBS AND DRIVEWAYS.

END OF SECTION 02810

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- (1) SHRUB PER PLAN CENTER IN PIT, SET LEVEL TYPICAL.
- (2) ROOTBALL SET 2" ABOVE FINISH GRADE
- (3) AMENDED BACKFILL
- (4) SCARIFY BOTTOM AND SIDES OF PLANTING PIT, TYPICAL.
- (5) FINISH GRADE
- 6 MULCH
- (7) NATIVE UNDISTURBED SOIL
- 8 WATERING BASIN
- 9 PLANT TABLETS (3) PER SHRUB

- (1) ADJACENT FINISH GRADE PER PLAN. (2) 4"-6" MEXICAN BLACK BEACH COBBLE HAND SET WITH DARK GREY MORTAR.
- (3) COMPACTED SUBGRADE ROLLED FLAT
- (4) 3/16" THICK ALUMINUM LANDSCAPE EDGING SHALL BE 'SURE-LOC' OR APPROVED EQUAL 5.5" DEEP W/ 18" STAKES. INSTALL PER MANUFACTURER'S RECOMMENDATIONS.

PROVIDE 4'X4' MOCK-UP IN THE FIELD FOR REVIEW AND APPROVAL PRIOR TO COMMENCING REST OF COBBLE INSTALLATION. ANY MORTAR SET COBBLE INSTALLED WITHOUT PRIOR APPROVAL WILL BE SUBJECT TO REMOVAL AND REPLACEMENT AT CONTRACTOR'S EXPENSE.

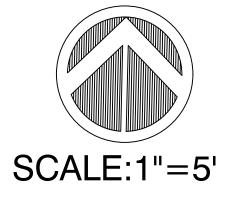
PLANTING NOTES:

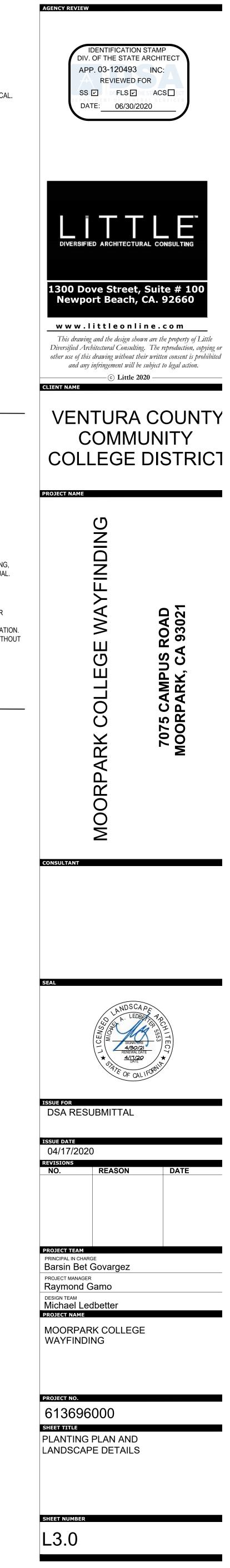
- 1. IT IS THE CONTRACTOR'S RESPONSIBILITY TO FURNISH PLANT MATERIAL THAT IS FREE OF PESTS AND DISEASE. ANY PLANT MATERIAL FOUND AT THE SITE EXHIBITING SIGNS OF PESTS AND/OR DISEASE WILL BE REMOVED AND REPLACED AT NO COST TO THE OWNER.
- 2. ALL SHRUB AREAS SHALL RECEIVE 3" DARK, HARDWOOD BARK MUCH. SHALL BE CAMPUS STANDARD TO MATCH EXISTING SHRUB AREAS. SUBMIT SAMPLE FOR REVIEW AND APPROVAL PRIOR TO PURCHASE. NO SHREDDED MATERIAL WILL BE ACCEPTED.
- 3. IT IS THE CONTRACTOR'S RESPONSIBILITY TO COORDINATE WITH OTHER TRADES AS REQUIRED TO ACCOMPLISH PLANTING OPERATIONS PER THE CONSTRUCTION SCHEDULE.
- 4. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENSURE POSITIVE DRAINAGE AWAY FROM ALL BUILDINGS AND STRUCTURES, 2% MIN.
- 5. CONTRACTOR SHALL NOT WILLFULLY INSTALL ANY PLANT MATERIAL WHEN IT IS OBVIOUS THAT UNKNOWN CONDITIONS OR GRADE DIFFERENCES EXIST THAT WOULD MAKE THE PROPOSED DESIGN UNACHIEVABLE. NOTIFY THE GENERAL CONTRACTOR IMMEDIATELY IF ANY SUCH CONDITIONS ARE DISCOVERED. FAILURE TO NOTIFY THE APPROPRIATE PARTIES COULD RESULT IN THE REJECTION AND REMOVAL OF FINISHED WORK AT NO COST TO THE OWNER.

TREE PROTECTION NOTES & SPECIAL REQUIREMENTS FOR WORK WITHIN DRIP LINE OF EXISTING OAK TREE

HATCHED AREA BENEATH EXISTING OAK TO REMAIN UNDISTURBED THROUGHOUT CONSTRUCTION.

- 1. PRIOR TO COMMENCING ANY WORK WITHIN THE DRIPLINE OF THE EXISTING OAK, CONTRACTOR WILL BE REQUIRED TO SECURE THE SERVICES OF A CERTIFIED ARBORIST TO REVIEW THE EXISTING CONDITIONS AROUND THE OAK TREE AND PROVIDE RECOMMENDATIONS REGARDING CONSTRUCTION OPERATIONS
- WITHIN THE DRIPLINE OF THE EXISTING OAK. 2. SUBMIT NAME AND QUALIFICATIONS OF CERTIFIED ARBORISTS TO
- BE USED AS PART OF BID PACKAGE. 3. IF DEEMED NECESSARY, ARBORIST SHALL OBSERVE WORK BEING PERFORMED WITHIN THE DRIPLINE OF THE EXISTING OAK AND PROVIDE RECOMMENDATIONS FOR EXISTING ROOTS OVER 2" IN
- DIAMETER TO BE DISTURBED BY CONSTRUCTION OPERATIONS. 4. PRIOR TO ANY GRADING OR CONSTRUCTION, THE CONTRACTOR SHALL SET UP A TREE PROTECTION ZONE, TREE PROTECTION ZONE
- SHALL FOLLOW EXISTING OAK'S DRIPLINE, TYPICAL. 5. TREE PROTECTION FENCING SHALL BE 6-FT HIGH CHAINLINK.
- 6. ALL EQUIPMENT TRACKS SHALL REMAIN OUTSIDE OF THE DRIPLINE AND AVOID CONTACT WITH ANY PART OF THE PROTECTED TREE.
- 7. NO STOCKPILING, TRENCHING FOR UTILITIES OR DRAINAGE, GRADING, OR SOIL COMPACTING SHALL BE ALLOWED WITHIN THE TREE PROTECTION ZONE.
- 8. NO SOLVENTS, FUELS OR CONSTRUCTION DEBRIS SHALL BE ALLOWED WITHIN THE TREE PROTECTION ZONE.
- 9. THE CONTRACTOR SHALL BE REQUIRED TO INSTALL A 6" LAYER OF WOOD CHIPS TO PREVENT COMPACTION IF ENTERING THE TREE PROTECTION ZONE.
- 10. THE CONTRACTOR SHALL MAINTAIN ALL PROTECTION FENCING IN AN UPRIGHT, STURDY MANNER AT THE PRESCRIBED LOCATIONS THROUGHOUT THE ENTIRE CONSTRUCTION PHASE.
- 11.ALL TRENCHING AND/OR POST DIGGING WITHIN THE TREE PROTECTION ZONE SHALL BE DONE WITH MAN-POWERED TOOLS. NO HEAVY EQUIPMENT SHALL BE ALLOWED WITHIN THE TREE PROTECTION ZONE.





SECTION 02900 - LANDSCAPE WORK PART 1 - GENERAL

1.1. RELATED DOCUMENTS

1.1.1. DRAWINGS AND GENERAL PROVISIONS OF CONTRACT, INCLUDING GENERAL AND SUPPLEMENTARY CONDITIONS AND DIVISION 1 SPECIFICATION SECTIONS, APPLY TO THIS SECTION.

1.2. SUMMARY

1.2.1. THIS SECTION INCLUDES PROVISIONS FOR THE FOLLOWING ITEMS:

- 1.2.1.1. NOT USED.
- 1.2.1.2. SHRUBS. 1.2.1.3. PLANTS.
- 1.2.1.4. GROUNDCOVER.
- 1.2.1.5. SODDING LAWN AREAS. 1.2.1.6. NOT USED
- 1.2.1.7. SOIL AMENDMENTS
- 1.2.1.8. FINISH GRADING. 1.2.1.9. MAINTENANCE PERIOD.

1.2.2. RELATED SECTIONS: THE FOLLOWING SECTIONS CONTAIN REQUIREMENTS THAT RELATE TO THIS SECTION.

1.2.3. UNDERGROUND SPRINKLER SYSTEM IS SPECIFIED IN DIVISION 2 SECTION, "UNDERGROUND IRRIGATION SYSTEM."

1.3. QUALITY ASSURANCE

1.3.1. SUBCONTRACT LANDSCAPE WORK TO A SINGLE FIRM SPECIALIZING IN LANDSCAPE WORK. THE CONTRACTOR SHALL POSSESS ALL LICENSES AND PERMITS REQUIRED TO PERFORM THE WORK INCLUDING A C-27 LANDSCAPING LICENSE.

1.3.2. SOURCE QUALITY CONTROL:

1.3.2.1. GENERAL: SHIP LANDSCAPE MATERIALS WITH CERTIFICATES OF INSPECTION REQUIRED BY GOVERNING AUTHORITIES. COMPLY WITH REGULATIONS APPLICABLE TO LANDSCAPE MATERIALS.

1.3.2.2. DO NOT MAKE SUBSTITUTIONS. IF SPECIFIED LANDSCAPE MATERIAL IS NOT OBTAINABLE, SUBMIT PROOF OF NON-AVAILABILITY TO LANDSCAPE ARCHITECT, TOGETHER WITH PROPOSAL FOR USE OF EQUIVALENT MATERIAL.

1.3.2.3. ANALYSIS AND STANDARDS: PACKAGE STANDARD PRODUCTS WITH MANUFACTURER'S CERTIFIED ANALYSIS. FOR OTHER MATERIALS, PROVIDE ANALYSIS BY RECOGNIZED LABORATORY MADE IN ACCORDANCE WITH METHODS ESTABLISHED BY THE ASSOCIATION OF OFFICIAL AGRICULTURE CHEMISTS. WHEREVER APPLICABLE.

1.3.2.4. TREES, SHRUBS, AND PLANTS: PROVIDE TREES, SHRUBS, AND PLANTS OF QUANTITY, SIZE, GENUS, SPECIES, AND VARIETY SHOWN AND SCHEDULED FOR LANDSCAPE WORK AND COMPLYING WITH RECOMMENDATIONS AND REQUIREMENTS OF ANSI Z60.1 "AMERICAN STANDARD FOR NURSERY STOCK" PROVIDE HEALTHY, VIGOROUS STOCK, GROWN IN RECOGNIZED NURSERY IN ACCORDANCE WITH GOOD HORTICULTURAL PRACTICE AND FREE OF DISEASE. INSECTS, EGGS, LARVAE, AND DEFECTS SUCH AS KNOTS, SUN-SCALD, INJURIES, ABRASIONS, OR DISFIGUREMENT.

1.3.2.5. LABEL EACH TREE AND SHRUB WITH SECURELY ATTACHED WATERPROOF TAG BEARING LEGIBLE DESIGNATION OF BOTANICAL AND COMMON NAME.

1.3.2.6. WHERE FORMAL ARRANGEMENTS OR CONSECUTIVE ORDER OF TREES OR SHRUBS ARE SHOWN. SELECT STOCK FOR UNIFORM HEIGHT AND SPREAD. AND LABEL WITH NUMBER TO ASSURE SYMMETRY IN PLANTING.

1.3.2.7. SELECTION: THE LANDSCAPE ARCHITECT MAY CHECK TREES AND SHRUBS EITHER AT PLACE OF GROWTH OR AT SITE BEFORE PLANTING, FOR COMPLIANCE WITH REQUIREMENTS FOR GENUS, SPECIES, VARIETY, SIZE, AND QUALITY. THE CONTRACTOR SHALL SUBMIT PHOTOGRAPHS TO LANDSCAPE ARCHITECT OF TYPICAL TREE (15 GAL, AND LARGER CONTAINER SIZES) FOR LANDSCAPE WORK. LANDSCAPE ARCHITECT RETAINS RIGHT TO FURTHER CHECK TREES AND SHRUBS FOR SIZE AND CONDITION OF BALLS AND ROOT SYSTEMS, INSECTS, INJURIES AND LATENT DEFECTS, AND TO REJECT UNSATISFACTORY OR DEFECTIVE MATERIAL AT ANY TIME DURING PROGRESS OF WORK. REMOVE REJECTED TREES OR SHRUBS IMMEDIATELY FROM PROJECT SITE.

1.4. SUBMITTALS

1.4.1. GENERAL: SUBMIT THE FOLLOWING IN ACCORDANCE WITH CONDITIONS OF CONTRACT.

1.4.2. PLANT AND MATERIAL CERTIFICATIONS:

1.4.2.1. CERTIFICATES OF INSPECTION AS REQUIRED BY GOVERNMENTAL AUTHORITIES.

1.4.2.2. MANUFACTURER'S OR VENDOR'S CERTIFIED ANALYSIS FOR SOIL AMENDMENTS AND FERTILIZER MATERIALS.

1.4.2.3. LABEL DATA SUBSTANTIATING THAT PLANTS, TREES, SHRUBS AND PLANTING MATERIALS COMPLY WITH SPECIFIED REQUIREMENTS.

1.4.2.4. PHOTOS OF ALL PROPOSED PLANT MATERIAL FROM NURSERY SHOWING TYPICAL SIZE AND HEALTH OF PLANTS TO BE PROVIDED AND INSTALLED. 1.5. DELIVERY, STORAGE AND HANDLING

1.5.1. PACKAGED MATERIALS: DELIVER PACKAGED MATERIALS IN CONTAINERS SHOWING WEIGHT, ANALYSIS, AND NAME OF MANUFACTURER. PROTECT MATERIALS FROM DETERIORATION DURING DELIVERY, AND WHILE STORED AT SITE.

1.5.3. TREES AND SHRUBS: DO NOT PRUNE PRIOR TO DELIVERY UNLESS OTHERWISE APPROVED BY LANDSCAPE ARCHITECT. DO NOT BEND OR BIND-TIE TREES OR SHRUBS IN SUCH MANNER AS TO DAMAGE BARK, BREAK BRANCHES, OR DESTROY NATURAL SHAPE. PROVIDE PROTECTIVE COVERING DURING DELIVERY.

1.5.4. DO NOT REMOVE CONTAINER-GROWN STOCK FROM CONTAINERS UNTIL PLANTING TIME.

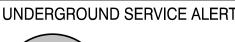
1.6. JOB CONDITIONS

1.6.1. UTILITIES: DETERMINE LOCATION OF UNDERGROUND UTILITIES AND PERFORM WORK IN A MANNER, WHICH WILL AVOID POSSIBLE DAMAGE. HAND EXCAVATES, AS REQUIRED. MAINTAIN GRADE STAKES SET BY OTHERS UNTIL PARTIES CONCERNED MUTUALLY AGREE UPON REMOVAL.

1.6.2. EXCAVATION: WHEN CONDITIONS DETRIMENTAL TO PLANT GROWTH ARE ENCOUNTERED, SUCH AS RUBBLE FILL, ADVERSE DRAINAGE CONDITIONS, OR OBSTRUCTIONS, NOTIFY LANDSCAPE ARCHITECT BEFORE PLANTING.

1.7. SEQUENCING AND SCHEDULING

1.7.1. PLANTING TIME: PROCEED WITH, AND COMPLETE LANDSCAPE WORK AS RAPIDLY AS PORTIONS OF SITE BECOME AVAILABLE, AND SHALL BE PERFORMED DURING THOSE PERIODS WHEN WEATHER AND SOIL CONDITIONS ARE SUITABLE IN ACCORDANCE WITH LOCALLY ACCEPTED HORTICULTURAL PRACTICE.



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1.7.2. COORDINATION WITH GROUNDCOVERS: PLANT TREES AND SHRUBS AFTER FINAL GRADES ARE ESTABLISHED AND PRIOR TO PLANTING OF GROUNDCOVER, UNLESS OTHERWISE ACCEPTABLE BY LANDSCAPE ARCHITECT. IF PLANTING OF TREES AND SHRUBS OCCUR AFTER GROUNDCOVER, PROTECT GROUNDCOVER AREAS AND PROMPTLY REPAIR DAMAGE TO GROUNDCOVER RESULTING FROM PLANTING OPERATIONS.

SOD: TPI (SPEC), CERTIFIED TURFGRASS SOD QUALITY; CULTIVATED GRASS SOD; Α. TYPE INDICATED IN PLANT SCHEDULE ON DRAWINGS; WITH STRONG FIBROUS ROOT SYSTEM, FREE OF STONES, BURNED OR BARE SPOTS; CONTAINING NO MORE THAN 5 WEEDS PER 1000 SQ FT. MINIMUM AGE OF 18 MONTHS. WITH ROOT DEVELOPMENT THAT WILL SUPPORT ITS OWN WEIGHT WITHOUT TEARING, WHEN SUSPENDED VERTICALLY BY HOLDING THE UPPER TWO CORNERS. TOPSOIL: EXCAVATED FROM SITE AND FREE OF WEEDS. WATER: CLEAN, FRESH AND FREE OF SUBSTANCES OR MATTER THAT COULD INHIBIT VIGOROUS GROWTH OF GRASS.

2.5.1. PROVIDE PLANTS ESTABLISHED AND WELL ROOTED IN FLAT REMOVABLE CONTAINERS, OR INTEGRAL PEAT POTS AND WITH NOT LESS THAN MINIMUM NUMBER AND LENGTH OF RUNNERS REQUIRED BY ANSI Z60.1 FOR THE POT SIZE SHOWN OR LISTED.

1.8.1. WARRANTY GROUNDCOVERS THROUGH SPECIFIED MAINTENANCE PERIOD, AND UNTIL FINAL ACCEPTANCE.

1.8.2. WARRANTY SHRUBS FOR A PERIOD OF 90 DAYS AFTER DATE OF FINAL ACCEPTANCE.

1.8.3. WARRANTY TREES, FOR A PERIOD OF ONE YEAR AFTER DATE OF SUBSTANTIAL COMPLETION. AGAINST DEFECTS INCLUDING DEATH AND FOR PERIOD OF 90 DAYS AFTER DATE OF UNSATISFACTORY GROWTH, EXCEPT FOR DEFECTS RESULTING FROM NEGLECT BY OWNER, ABUSE OR DAMAGE BY OTHERS, OR UNUSUAL PHENOMENA OR INCIDENTS WHICH ARE BEYOND LANDSCAPE INSTALLER'S CONTROL.

1.8.4. REMOVE AND REPLACE TREES, SHRUBS, OR OTHER PLANTS FOUND TO BE DEAD OR IN UNHEALTHY CONDITION DURING WARRANTY PERIOD. MAKE REPLACEMENTS WITHIN 14 CALENDAR DAYS. REPLACE TREES AND SHRUBS, WHICH ARE IN DOUBTFUL CONDITION AT END OF WARRANTY PERIOD; UNLESS, IN OPINION OF LANDSCAPE ARCHITECT, IT IS ADVISABLE TO EXTEND WARRANTY PERIOD FOR A FULL GROWING SEASON.

1.8.5. ANOTHER WARRANTY SITE OBSERVATION VISIT WILL BE CONDUCTED AT END OF EXTENDED WARRANTY PERIOD, IF ANY, TO DETERMINE ACCEPTANCE OR REJECTION. REPLACEMENT SHALL BE THE PLANTS USED FOR SAME KIND AND SIZE AS SPECIFIED FOR LANDSCAPE WORK. REPLACEMENTS SHALL BE FURNISHED, AND PLANTED AS ORIGINALLY SPECIFIED.

2.1.1. BARK MULCH: A 3-INCH MINUS BLEND, DARK COLORED PRODUCT RECOMMENDED FOR MULCHING IN SHRUB BEDS. PRODUCT AS NOTED ON THE DRAWINGS.

2.1.2. COMMERCIAL FERTILIZER: COMPLETE FERTILIZER OF NEUTRAL CHARACTER, WITH SOME ELEMENTS DERIVED FROM ORGANIC SOURCES AND CONTAINING THE FOLLOWING PERCENTAGES OF AVAILABLE PLANT NUTRIENTS:

2.1.2.1. PRE-PLANT FERTILIZER: PROVIDE FERTILIZER WITH NOT MORE THAN 1 PERCENT TOTAL NITROGEN; AND NOT LESS THAN 10 PERCENT AVAILABLE PHOSPHORIC ACID AND 10 PERCENT SOLUBLE POTASH.

2.1.2.2. POST-PLANT FERTILIZER: PROVIDE FERTILIZER WITH PERCENTAGE OF NITROGEN REQUIRED TO PROVIDE NOT LESS THAN 18 POUNDS OF ACTUAL NITROGEN, 6 PERCENT PHOSPHORIC ACID AND 8 PERCENT POTASSIUM. PROVIDE NITROGEN IN A FORM THAT WILL BE AVAILABLE DURING INITIAL PERIOD OF GROWTH; AT LEAST 50 PERCENT OF NITROGEN TO BE ORGANIC FORM.

2.1.3. IRON SULPHATE, IRON SHALL BE EXPRESSED AS METALLIC-DERIVED FROM SULFATE-DEEP GREEN (FESO4OH2O) A MINIMUM ANALYSIS OF 200% AND 98.3% RETAINED ON A 10 MESH SCREEN.

2.2.3. GYPSUM, AGRICULTURAL GRADE GYPSUM SHALL BE A (CASO4OH2O) CALCIUM SULFATE 94.3%. 90% SHALL PASS A 50-MESH SCREEN.

2.2.4. SOIL SULPHUR, SHALL BE ELEMENTAL SULPHUR (99.5%) COMMERCIALLY MANUFACTURED SO THAT A PURE SULPHUR PRODUCT IS USED.

2.2.5. ORGANIC SOIL CONDITIONER, SHALL BE A PRODUCT THAT AIDS THE STRUCTURE OF THE SOIL CONSISTING OF RAPIDLY DECAYING SLOWLY DECAYING, AND NON-DECAYING MATERIAL. NITROGEN (ORGANIC OR AMMONIC) 0.5%, PH LESS THAN 6.8 SALINITY (ECE X 103 AT 25° C) = 2.5, ASH CONTENT NOT TO EXCEED 10%. IRON (FE) EXPRESSED AS METALLIC 0.08% ORGANIC MATTER 35%. PROPERTIES: SCREEN ANALYSIS:

MESH + 36.6% 12 MESH + 30.7% REMAINDER 0.9%

2.2.5.2. THE COMMERCIAL GRADE PRODUCT USED SHALL BE PLANT CHOICE SOIL AMENDMENT, ORGANIC RECYCLING WEST SOIL AMENDMENT. OR BUTLERS MILL LOAMEX OR APPROVED EQUAL BY LANDSCAPE ARCHITECT.

2.2.6. PLANTING BACKFILL, SHALL BE A THOROUGHLY BLENDED MIXTURE OF EXCAVATED SOIL FROM THE PITS AND SOIL AMENDMENTS AT THE FOLLOWING MIXTURE SOIL CONDITIONER:

2.2.6.1. SOIL CONDITIONER 50% 2.2.6.2. ON SITE SOIL 50%

2.3.1. QUALITY: PROVIDE TREES, SHRUBS, AND OTHER PLANTS OF SIZE, GENUS, SPECIES, AND VARIETY SHOWN AND SCHEDULED FOR LANDSCAPE WORK AND COMPLYING WITH RECOMMENDATIONS AND REQUIREMENTS OF ANSI Z60.1 "AMERICAN STANDARD FOR NURSERY STOCK."

2.3.2. CONTAINER STOCK (1 GAL., 5 GAL., 15 GAL., BOXES) SHALL HAVE GROWN IN CONTAINER FOR AT LEAST SIX MONTHS, BUT NOT OVER TWO YEARS. NO CONTAINER PLANTS THAT HAVE CRACKED OR BROKEN BALLS OF EARTH, WHEN TAKEN FROM THE CONTAINER, SHALL BE PLANTED, EXPECT UPON SPECIAL APPROVAL. NO TREES WITH DAMAGED ROOTS OR BROKEN BALLS SHALL BE PLANTED.

2.3.3. TREES: NOT APPLICABLE

2.3.4. SHRUBS: PROVIDE SHRUBS OF THE HEIGHT AND WIDTH SHOWN OR LISTED REQUIRED BY ANSI Z60.1 FOR TYPE AND HEIGHT OF SHRUB REQUIRED. 2.4 SODDED AREAS

2.5. GROUNDCOVER

1.8. SPECIAL PROJECT WARRANTY

PART 2 - PRODUCTS

2.1. SOIL AMENDMENTS

2.1.2.3. PLANT TABLETS, AGRIFORM (20-10-5-) BLUE CHIP TABLETS 21 GRAM.

2.2.5.1. MESH + 0.2% 8 MESH + 25.7% 32 MESH + 5%

2.3. PLANT MATERIALS

PART 3 - EXECUTION

3.1. PREPARATION - GENERAL

3.1.1. LAY OUT PLANTING AREAS SHALL MEAN ALL AREAS TO BE PLANTED WITH TREES, SHRUBS, GROUNDCOVERS AND AREAS FOR MULTIPLE PLANTINGS. STAKE LOCATIONS AND OUTLINE AREAS AND SECURE LANDSCAPE ARCHITECT'S ACCEPTANCE BEFORE START OF PLANTING WORK. MAKE MINOR ADJUSTMENTS AS MAY BE REQUIRED.

3.1.2. ALL ROCK AND OTHER GROWTH OR DEBRIS ACCUMULATED DURING THE DURATION OF THE PROJECT SHALL BE REMOVED FROM THE SITE. UPON COMPLETION OF ALL GRADING OPERATIONS, SOIL SAMPLES (3 LOCATIONS MIN.) SHALL BE TAKEN BY THE CONTRACTOR AND ANALYZED BY A SOIL LABORATORY. THE RESULTS OF THESE TESTS ARE TO BE REVIEWED BY THE LANDSCAPE ARCHITECT FOR ANY REQUIRED MODIFICATIONS TO SPECIFIED SOIL PREPARATION.

3.1.3. GRADING AND SOIL PREPARATION WORK SHALL BE PERFORMED ONLY DURING THE PERIOD WHEN BENEFICIAL AND OPTIMUM RESULTS MAY BE OBTAINED. IF THE MOISTURE CONTENT OF THE SOIL SHOULD REACH SUCH A LEVEL THAT WORKING IT WOULD DESTROY SOIL STRUCTURE, SPREADING AND GRADING OPERATIONS SHALL BE SUSPENDED UNTIL THE MOISTURE CONTENT IS INCREASED OR REDUCED TO ACCEPTABLE LEVELS AND THE DESIRED RESULTS ARE LIKELY TO BE OBTAINED.

3.1.4. ALL SCALED DIMENSIONS ARE APPROXIMATE. BEFORE PROCEEDING WITH ANY WORK, CAREFULLY CHECK AND VERIFY ALL DIMENSIONS AND IMMEDIATELY INFORM THE LANDSCAPE ARCHITECT OF ANY DISCREPANCY BETWEEN THE DRAWINGS AND/OR SPECIFICATIONS AND ACTUAL CONDITIONS.

3.1.5. QUANTITIES FOR PLANT MATERIALS ARE SHOWN FOR CONVENIENCE ONLY. AND NOT GUARANTEED. CHECK AND VERIFY COUNT AND SUPPLY SUFFICIENT NUMBER TO FULFILL INTENT OF DRAWINGS. CERTIFY ANY CLARIFICATIONS WITH THE LANDSCAPE ARCHITECT. ADEQUATELY STAKE, BARRICADE, AND PROTECT ALL IRRIGATION EQUIPMENT, MANHOLES, UTILITY LINES, AND OTHER EXISTING PROPERTY DURING ALL PHASES OF THE SOIL AMENDING PLANTING AND GRADING OPERATIONS.

3.1.6. UPON DELIVERY OF MATERIAL AND/OR COMPLETION OF ALL SOIL CONDITIONING AND GRADING BUT PRIOR TO INITIATING PLANTING OPERATIONS, THE LANDSCAPE ARCHITECT WITH THE HERETOFORE SPECIFIED SIGNED COPIES OF REQUIRED CERTIFICATES, TRIP SLIPS, AND INVOICES FOR SOIL PREPARATION MATERIALS, SHALL INVOICE SUCH MATERIAL, COMPARING THE TOTAL QUANTITIES OF EACH MATERIAL FURNISHED AGAINST THE TOTAL AREA TO EACH OPERATION. IF THE MINIMUM RATES OF APPLICATION HAVE NOT BEEN MET, THE LANDSCAPE ARCHITECT WILL REQUIRE THE DISTRIBUTION OF ADDITIONAL QUANTITIES OF THESE MATERIALS TO FULFILL THE MINIMUM APPLICATION REQUIREMENTS SPECIFIED AT NO COST TO OWNER.

3.2. FINISH GRADING

3.2.1. FINISH GRADING: FINISH GRADES SHALL BE AS INDICATED ON THE CIVIL ENGINEER'S DRAWINGS AND LANDSCAPE DRAWINGS.

3.2.2. FINISH GRADES SHALL BE MEASURED AS THE FINAL WATER COMPACTED AND SETTLED SURFACE GRADES: AND SHALL BE WITHIN PLUS OR MINUS 0.1 FOOT OF THE SPOT ELEVATIONS AND GRADE LINES INDICATED ON THE DRAWING.

3.2.3. FINISH GRADES SHALL BE MEASURED AT THE TOP SURFACE OF SURFACE MATERIALS.

3.2.4. MOLDING AND ROUNDING OF THE GRADES SHALL BE PROVIDED AT ALL CHANGES IN SLOPE.

3.2.5. ALL UNDULATIONS AND IRREGULARITIES IN THE PLANTING SURFACES RESULTING FROM TILLAGE, ROTOTILLING AND ALL OTHER OPERATIONS SHALL BE LEVELED AND FLOATED OUT BEFORE PLANTING OPERATIONS ARE INITIATED.

3.2.6. THE CONTRACTOR SHALL TAKE EVERY PRECAUTION TO PROTECT AND AVOID DAMAGE TO SPRINKLER HEADS, IRRIGATION LINES, AND OTHER UNDERGROUND UTILITIES DURING HIS GRADING AND CONDITIONING OPERATIONS.

3.2.7. FINAL FINISH GRADES SHALL INSURE POSITIVE DRAINAGE OF THE SITE WITH ALL SURFACE DRAINAGE AWAY FROM BUILDINGS, WALLS, AND TOWARD ROADWAYS. DRAINS AND CATCH BASINS.

3.2.8. FINAL GRADES SHALL BE ACCEPTABLE TO THE LANDSCAPE ARCHITECT BEFORE PLANTING OPERATIONS WILL BE ALLOWED TO BEGIN.

3.2.9. PLANTING SURFACES SHALL BE GRADED WITH NO LESS THAN 2 PERCENT SURFACE SLOPE FOR POSITIVE DRAINAGE UNLESS OTHERWISE NOTED ON PLANS.

3.2.10. COMPACT THE SOIL MIX TO A RELATIVE COMPACITON OF 85% OF MODIFIED MAXIMUM DRY DENSITY (ASTM D1557).

3.2.11. FINAL GRADES WITHIN RAISED PLANTER WALLS BEHIND NEW SIGN SHALL BE DETERMINED AFTER ADEQUATE COMPACTING OF BACKFILL WITHIN RAISED PLANTER HAS BEEN ACHIEVED. FURTHER SETTLEMENT WITHIN THE RAISED PLANTER WILL NOT BE ACCEPTED. WHERE APPRECIABLE SETTLEMENT HAS OCCURRED WITHIN THE RAISED PLANTER PRIOR TO FINAL ACCEPTANCE AT THE END OF THE WARRANTY PERIOD. THE CONTRACTOR WILL BE REQUIRED TO ADDRESS AREAS THAT HAVE SETTLED AND RETURN THEM TO AS DESIGNED FINISH GRADES WITHIN THE RAISED PLANTER.

3.3. PREPARATION FOR SHRUB AREAS

3.3.2.1.1. "SCHEDULE OF PLANTING SOIL MIXTURE:

3.3.2.1.1.1. SOIL CONDITIONER: 6 CU. YD. PER 1,000 SQ. FT. 3.3.2.1.1.2. IRON SULPHATE: 20 LBS. PER 1,000 SQ. FT. 3.3.2.1.1.3. GYPSUM: 120 LBS. PER 1.000 SQ. FT. 3.3.2.1.1.4. PRE-PLANT FERTILIZER: 30 LBS. PER 1,000 SQ. FT.

3.3.2.2. SCHEDULE OF PLANTING SOIL MIXTURE IS FOR BIDDING PURPOSES ONLY. SOIL TEST MAY REDUCE OR INCREASE TOTAL SOIL AMENDMENT YARDAGE. ADJUSTMENT (PLUS OR MINUS) MAY BE NECESSARY. CONTRACTOR SHALL OBTAIN AT LEAST ONE SOIL TESTS OF FINAL GRADE AT SITE FOR SOIL MIXTURE RECOMMENDATIONS AND SUBMIT RESULTS TO LANDSCAPE ARCHITECT FOR THEIR RECORD. CONTRACTOR SHALL SUBMIT, IN ADDITION TO HIS BASE BID; UNIT COST FOR EACH SCHEDULE OF PLANTING SOIL MIXTURE.

3.4. DEEP WATERING AND WEED ABATEMENT:

3.4.1. AFTER COMPLETE INSTALLATION AND TESTING OF THE IRRIGATION SYSTEM, AND PREPARATION OF PLANTING AREAS, ALL PLANTING AREAS SHALL BE DEEP WATERED AND COMPACTED AND SETTLED BY CONTINUOUS APPLICATION OF IRRIGATION WATER UNTIL THE SOIL IS MOIST TO A MINIMUM DEPTH OF 8".

3.4.2. CARE SHALL BE TAKEN THAT THE RATE OF APPLICATION OF WATER DOES NOT CAUSE EROSION OR SLUFFING OF SOILS.

3.4.3. ALL DEPRESSIONS, VOIDS, EROSION SCARS AND SETTLED TRENCHES GENERATED BY THE DEEP WATERING SHALL BE FILLED WITH CONDITIONED TOPSOIL AND BROUGHT TO FINISH GRADE.

3.5. EXCAVATION FOR TREES AND SHRUBS

3.5.1. CONTAINER GROWN STOCK IN CANS SHALL BE CUT ON TWO SIDES WITH AN APPROVED CAN CUTTER. STOCK GROWN IN BOXES SHALL HAVE BOTTOMS REMOVED. ALL USED CONTAINERS SHALL BE REMOVED TO THE STORAGE AREAS OR FROM THE SITE. EACH TREE AND SHRUB SHALL BE PLACED IN THE CENTER OF THE HOLE AND SHALL BE SET PLUMB, REMOVE SIDES OF BOXES WHERE REQUIRED, AND HELD RIGIDLY IN POSITION UNTIL THE PLANTING BACKFILL HAS BEEN TAMPED FROM AROUND EACH ROOT BALL.

3.5.1.1. FOR CONTAINER-GROWN STOCK, EXCAVATE AS SPECIFIED FOR SIZE OF CONTAINER WIDTH AND DEPTH.

3.5.1.2. DISPOSE OF EXCESS SUBSOIL REMOVED FROM PLANTING EXCAVATIONS.

3.5.1.3. FILL EXCAVATIONS FOR TREES AND SHRUBS WITH WATER AND ALLOW WATER TO PERCOLATE OUT PRIOR TO PLANTING.

3.6. PLANTING TREES AND SHRUBS

3.6.1. SET CONTAINER GROWN STOCK AS SPECIFIED, CUT CANS ON 2 SIDES WITH AN APPROVED CAN CUTTER; REMOVE BOTTOMS OF WOODEN BOXES AFTER PARTIAL BACKFILLING SO AS NOT TO DAMAGE ROOTBALLS. ALL USED CONTAINERS SHALL BE REMOVED TO THE STORAGE AREAS OR FROM THE SITE. EACH TREE AND SHRUB SHALL BE PLACED IN THE CENTER OF THE HOLE AND SHALL BE SET PLUMB AND HELD RIGIDLY IN POSITION UNTIL THE PLANTING BACKFILL HAS BEEN TAMPED FROM AROUND EACH ROOT BALL.

3.6.2. ALL PLANTS SHALL BE SET AT SUCH A LEVEL THAT AFTER SETTLING, THEY BEAR THE SAME RELATIONSHIP TO THE SURROUNDING FINISH GRADE AS THEY BORE TO THE SOIL LINE GRADE IN THE CONTAINER.

3.6.3. PLANTING TABLETS SHALL BE PLACED IN EACH TREE-PLANTING HOLE AT THE FOLLOWING RATE

- 3.6.3.1. 1-21 GRAM TABLET PER 1-GALLON CONTAINER.
- 3.6.3.2. 3-21 GRAM TABLETS PER 5-GALLON CONTAINER.
- 3.6.3.3. 4-21 GRAM TABLETS PER 15-GALLON CONTAINER. 3.6.3.4. 1-21 GRAM TABLET PER EACH 4 INCH OF BOX SIZE.

3.6.4. NO PLANT WILL BE ACCEPTED IF THE ROOTBALL IS BROKEN OR CRACKED, EITHER BEFORE, DURING OR AFTER THE PROCESS OF INSTALLATION.

3.6.5. WATER BASIN SHALL BE FORMED AROUND EACH TREE AND SHRUB PER DETAIL. ALL PLANTS SHALL BE THOROUGHLY WATERED INTO THE FULL DEPTH OF EACH PLANT HOLE IMMEDIATELY AFTER PLANTING.

3.7 SODDING LAWN AREAS

3.71 PREPERATION

- A. APPLY FERTILIZER IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS.
- B. APPLY AFTER SMOOTH RAKING OF TOPSOIL AND PRIOR TO INSTALLATION OF SOD.
- C. APPLY FERTILIZER NO MORE THAN 48 HOURS BEFORE LAYING SOD.
- D. MIX THOROUGHLY INTO UPPER 2 INCHES OF TOPSOIL. E. LIGHTLY WATER TO AID THE DISSIPATION OF FERTILIZER.

3.72 LAYING SOD

- A. MOISTEN PREPARE SURFACE IMMEDIATELY PRIOR TO LAYING SOD. B. LAY SOD IMMEDIATELY AFTER DELIVERY TO SITE TO PREVENT
- DETERIORATION. C. LAY SOD SMOOTH AND TIGHT WITH NO OPEN JOINTS VISIBLE, AND NO
- OVERLAPPING; STAGGER END JOINTS 12 INCHES MINIMUM. DO NOT STRETCH OR OVERLAP SOD PIECES
- D. WHERE SOD IS PLACED ADJACENT TO HARD SURFACES, SUCH AS CURBS, PAVEMENTS, ETC., PLACE TOP ELEVATION OF SOD 1/2 INCH BELOW TOP OF HARD SURFACE.
- E. WATER SODDED AREAS IMMEDIATELY AFTER INSTALLATION. SATURATE SOD TO 4 INCHES OF SOIL.
- F. AFTER SOD AND SOIL HAVE DRIED, ROLL SODDED AREAS TO ENSURE GOOD BOND BETWEEN SOD AND SOIL AND TO REMOVE MINOR DEPRESSIONS AND IRREGULARITIES. ROLL SODDED AREAS WITH ROLLER NOT EXCEEDING 300 LBS.

3.8 NOT USED

3.9. PLANTING GROUND COVER

3.9.1. SPACE GROUND COVER PLANTS AS INDICATED OR SCHEDULED.

3.9.2. DIG HOLES LARGE ENOUGH TO ALLOW FOR SPREADING OF ROOTS AND BACKFILL WITH PLANTING SOIL. WORK SOIL AROUND ROOTS TO ELIMINATE AIR POCKETS AND LEAVE A SLIGHT SAUCER INDENTATION AROUND PLANTS TO HOLD WATER. WATER THOROUGHLY AFTER PLANTING, TAKING CARE NOT TO COVER CROWNS OF PLANTS WITH WET SOILS.

3.9.3. BARK MULCH. PROVIDE NOT LESS THAN FOLLOWING THICKNESS OF BARK MULCH IN PLANTING AREAS SPECIFIED.

3.9.4. PROVIDE 2 INCHES THICKNESS OF BARK MULCH.

3.10. POST FERTILIZATION

3.10.1. POST FERTILIZATION FOR ALL AREAS (18-6-8) SHALL OCCUR 45 DAYS AFTER PLANTING AT A RATE OF 8 LBS. PER 1,000 SQ. FT.

3.11. MAINTENANCE PERIOD

3.11.1. THE MAINTENANCE PERIOD BEGINS ON THE FIRST DAY AFTER ALL LANDSCAPE AND IRRIGATION WORK AND ALL OTHER INDICATED OR SPECIFIED WORK ON THIS PROJECT IS COMPLETE, CHECKED, ACCEPTED AND WRITTEN APPROVAL FROM THE LANDSCAPE ARCHITECT IS GIVEN TO BEGIN THE MAINTENANCE PERIOD.

3.11.2. THE CONTRACTOR SHALL CONTINUOUSLY MAINTAIN ALL INVOLVED AREAS OF THE CONTRACT DURING THE PROGRESS OF THE WORK AND DURING THE MAINTENANCE PERIOD UNTIL THE FINAL ACCEPTANCE OF THE WORK.

3.11.3. REGULAR PLANTING MAINTENANCE OPERATIONS SHALL BEGIN IMMEDIATELY AFTER EACH PLANT OR LAWN IS PLANTED. PLANTS AND LAWNS SHALL BE KEPT IN A HEALTHY, GROWING CONDITION AND IN A VISUALLY PLEASING APPEARANCE BY WATERING, PRUNING, MOWING, ROLLING, TRIMMING, EDGING, FERTILIZING, RE-STAKING, PEST AND DISEASE CONTROLLING, SPRAYING, WEEDING, CLEANING-UP AND ANY OTHER NECESSARY OPERATION.

3.11.4. THE MAINTENANCE PERIOD SHALL CONTINUE UNTIL FINAL ACCEPTANCE. BUT IN NO CASE, LESS THAN FOLLOWING PERIOD:

3.11.4.1. 60 DAYS AFTER SUBSTANTIAL COMPLETION OF PLANTING.

3.11.5. THE CONTRACT COMPLETION DATE OF THE CONTRACT MAINTENANCE PERIOD WILL BE EXTENDED, WHEN IN THE OPINION OF THE LANDSCAPE ARCHITECT, IMPROPER MAINTENANCE AND/OR POSSIBLE POOR OR UNHEALTHY CONDITION OF PLANTED MATERIAL OR UN-ESTABLISHED NON-COVERING LAWNS ARE EVIDENT AT THE TERMINATION OF THE SCHEDULED MAINTENANCE PERIOD. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ADDITIONAL MAINTENANCE OF THE WORK AT NO CHANGE IN CONTRACT PRICE UNTIL ALL OF THE WORK IS COMPLETED AND ACCEPTABLE.

3.11.6. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING ADEQUATE PROTECTION OF THE AREAS. DAMAGED AREAS SHALL BE REPAIRED IMMEDIATELY AT THE CONTRACTOR'S EXPENSE.

3.11.7. MAINTAIN TREES, SHRUBS, AND OTHER PLANTS BY PRUNING, CULTIVATING, AND WEEDING AS REQUIRED FOR HEALTHY GROWTH. RESTORE PLANTING WATER BASINS. TIGHTEN AND REPAIR STAKE AND GUY SUPPORTS AND RESET TREES AND SHRUBS TO PROPER GRADES OR VERTICAL POSITION AS REQUIRED. SPRAY AS REQUIRED TO KEEP TREES AND SHRUBS FREE OF INSECTS AND DISEASE.

3.12. CLEANUP AND PROTECTION

3.12.1. DURING LANDSCAPE WORK, KEEP PAVEMENTS CLEAN AND WORK AREA IN AN ORDERLY CONDITION.

3.12.2. PROTECT LANDSCAPE WORK AND MATERIALS FROM DAMAGE DUE TO LANDSCAPE OPERATIONS, OPERATIONS BY OTHER CONTRACTORS AND TRADES, AND TRESPASSERS. MAINTAIN PROTECTION DURING INSTALLATION AND MAINTENANCE PERIODS. TREAT, REPAIR, OR REPLACE DAMAGED LANDSCAPE WORK AS DIRECTED.

3.13. SITE OBSERVATION VISITS:

3.17.1. SITE OBSERVATION VISITS HEREIN SPECIFIED SHALL BE MADE BY THE LANDSCAPE ARCHITECT. THE CONTRACTOR SHALL REQUEST SITE OBSERVATION (48 HOURS MIN.) IN ADVANCE OF THE TIME OBSERVATION IS REQUIRED.

3.13.2. SITE OBSERVATION WILL BE REQUIRED FOR THE FOLLOWING PARTS OF THE WORK:

3.13.2.1. WHEN PLANTING, AND ALL OTHER INDICATED OR SPECIFIED WORK, EXCEPT THE MAINTENANCE PERIOD. HAS BEEN COMPLETED. ACCEPTANCE AND WRITTEN APPROVAL SHALL ESTABLISH BEGINNING OF THE MAINTENANCE PERIOD.

3.13.2.2. FINAL SITE OBSERVATION VISITS AT THE COMPLETION OF THE MAINTENANCE PERIOD. THIS SITE OBSERVATION VISIT SHALL ESTABLISH THE BEGINNING DATE FOR THE WARRANTY PERIOD OF PLANT MATERIAL

3.13.2.3. UPON COMPLETION OF THE WARRANTY PERIOD.

3.13.3. ACCEPTANCE: UPON COMPLETION OF THE FINAL SITE OBSERVATION VISIT AND THE WORK OF THIS SECTION, THE CONTRACTOR WILL BE NOTIFIED IN WRITING (1) WHETHER THE WORK IS ACCEPTABLE; (2) OF ANY REQUIREMENTS NECESSARY FOR COMPLETION AND ACCEPTANCE

3.13.4. THIS CONTRACTOR OR HIS AUTHORIZED REPRESENTATIVE SHALL BE ON THE SITE AT THE TIME OF EACH SITE OBSERVATION VISIT BY THE LANDSCAPE ARCHITECT.

END OF SECTION 02900

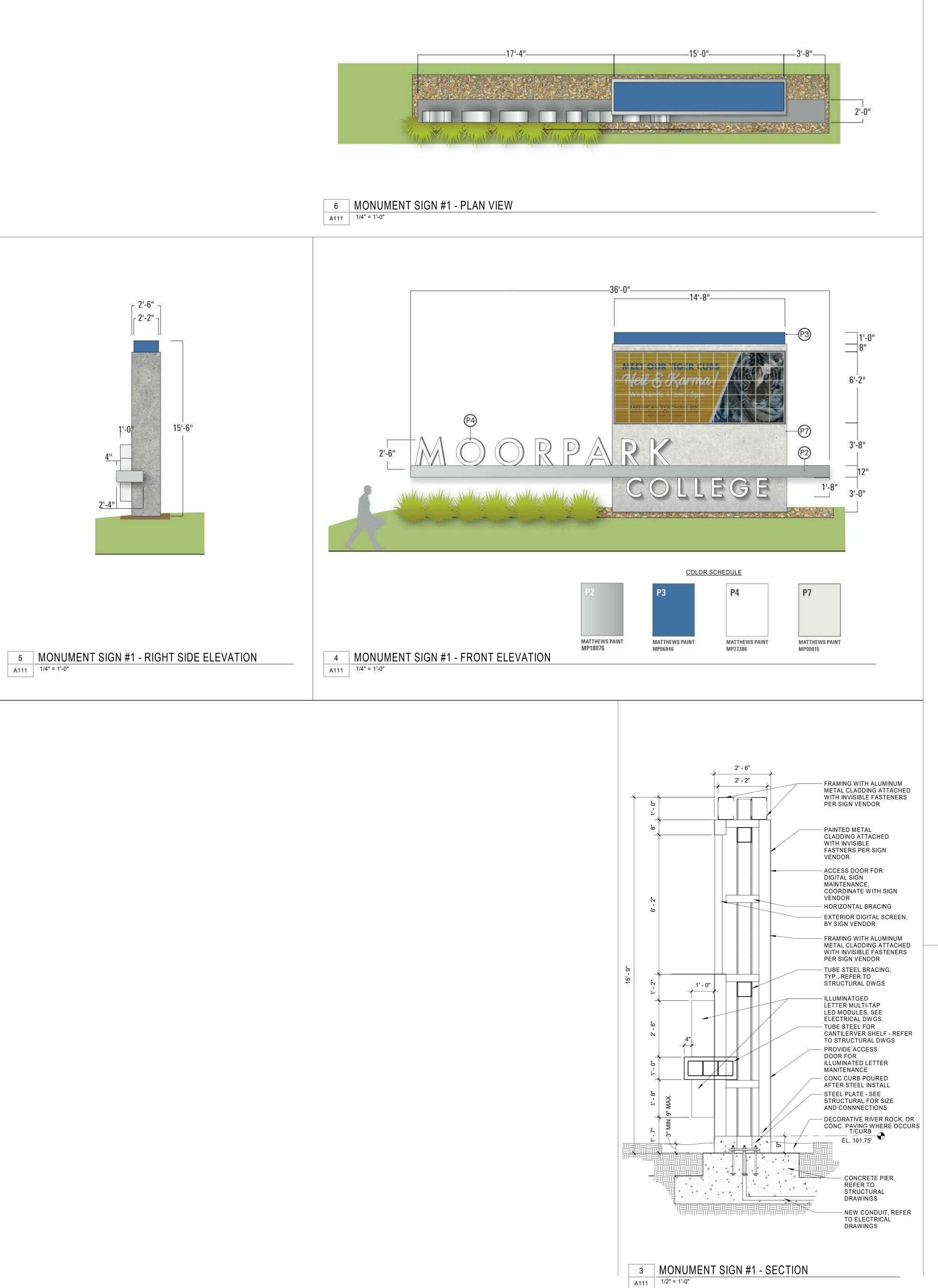
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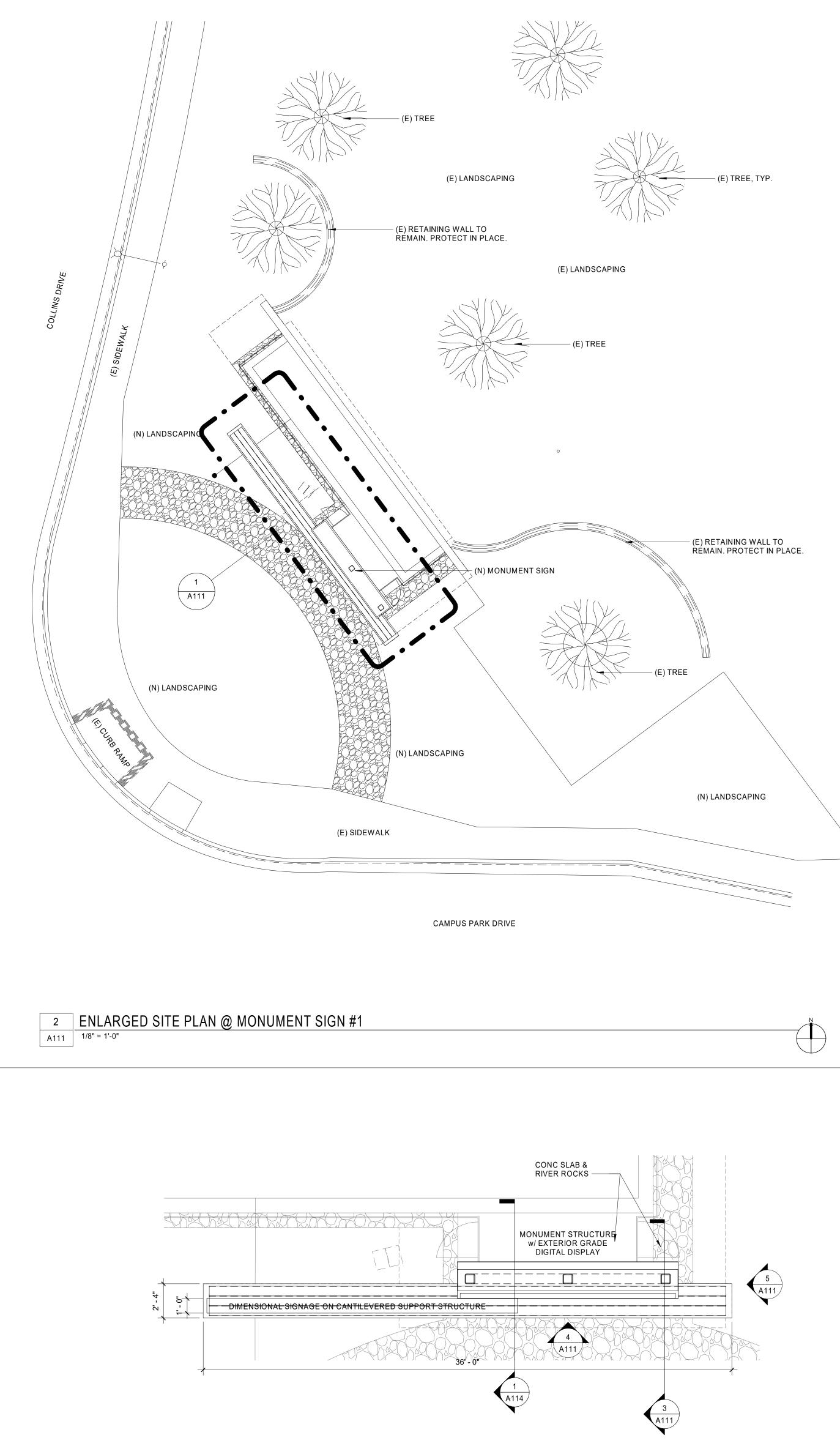




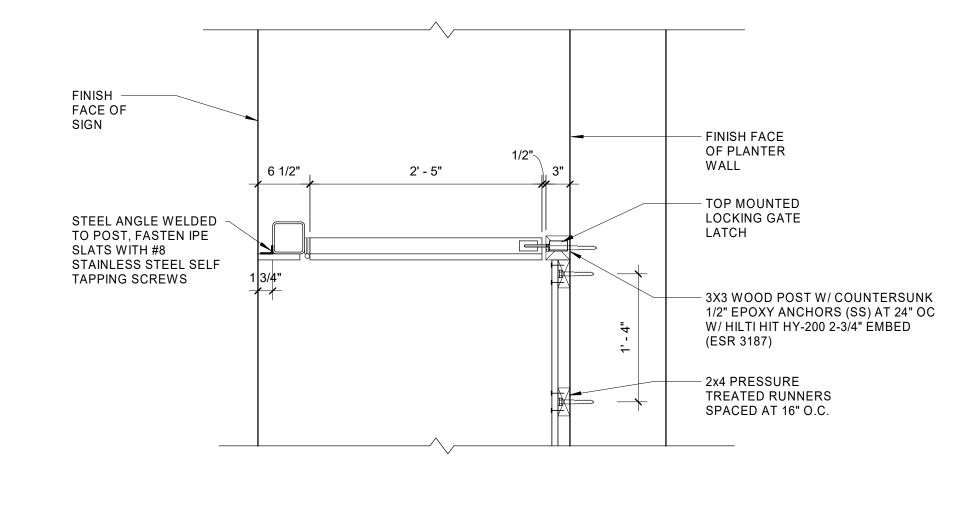
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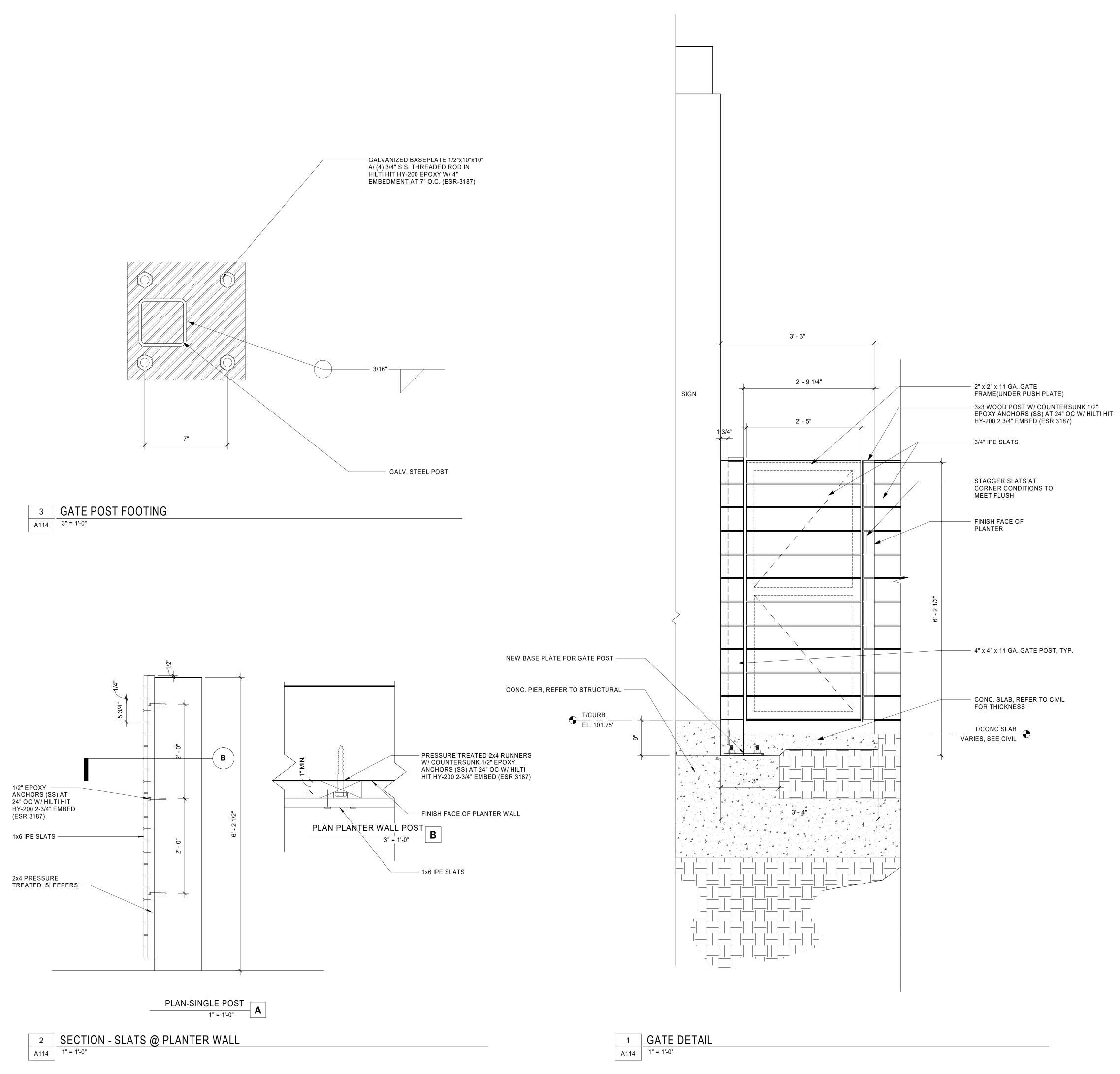


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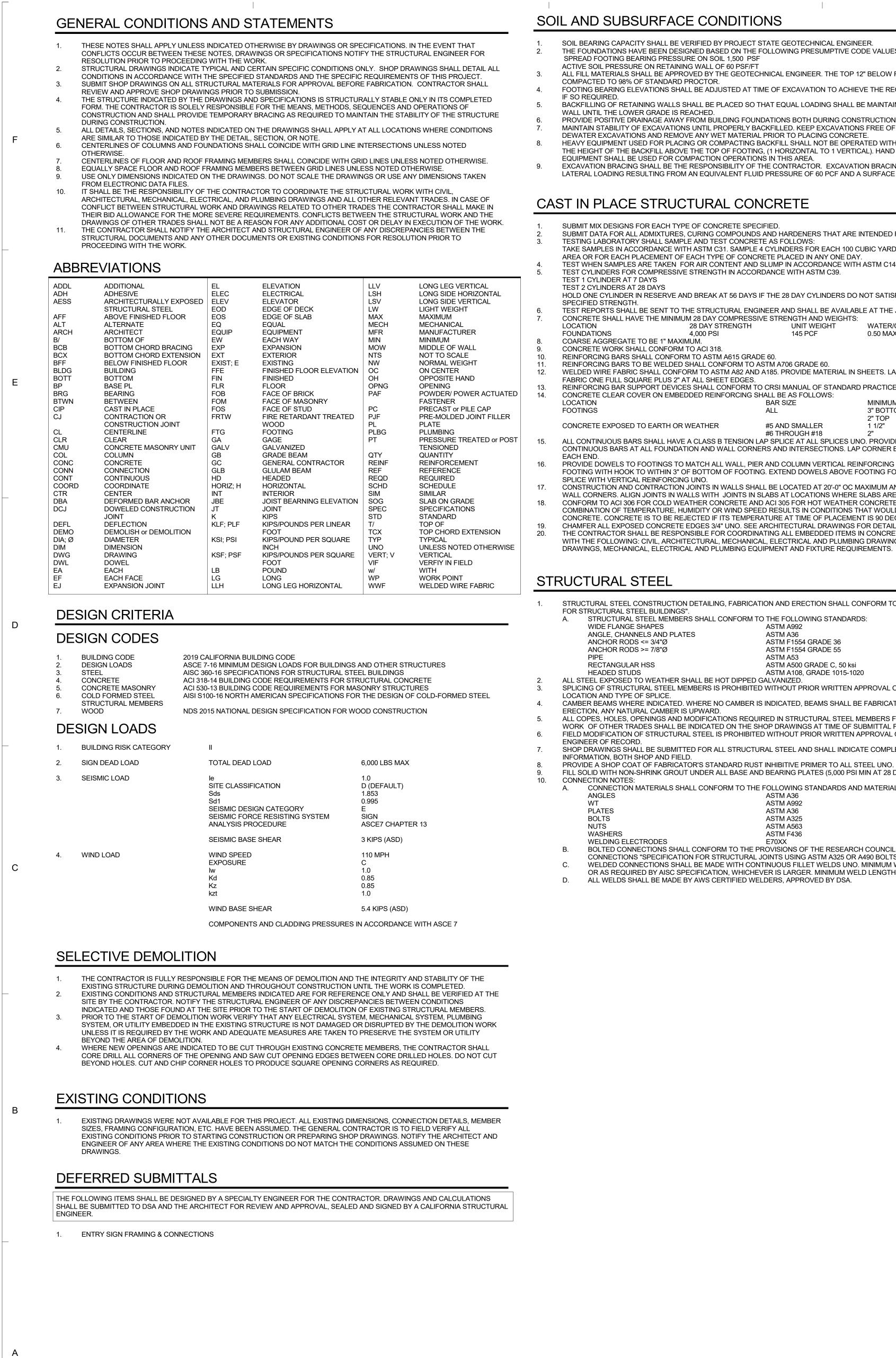


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 GATE - PLAN VIEW

 A114
 1" = 1'-0"



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SOIL AND SUBSURFACE CONDITIONS

SOIL BEARING CAPACITY SHALL BE VERIFIED BY PROJECT STATE GEOTECHNICAL ENGINEER. THE FOUNDATIONS HAVE BEEN DESIGNED BASED ON THE FOLLOWING PRESUMPTIVE CODE VALUES: SPREAD FOOTING BEARING PRESSURE ON SOIL 1,500 PSF

ACTIVE SOIL PRESSURE ON RETAINING WALL OF 60 PSF/FT ALL FILL MATERIALS SHALL BE APPROVED BY THE GEOTECHNICAL ENGINEER. THE TOP 12" BELOW FOOTINGS SHALL BE COMPACTED TO 98% OF STANDARD PROCTOR. FOOTING BEARING ELEVATIONS SHALL BE ADJUSTED AT TIME OF EXCAVATION TO ACHIEVE THE REQUIRED BEARING CAPACITY IF SO REQUIRED. BACKFILLING OF RETAINING WALLS SHALL BE PLACED SO THAT EQUAL LOADING SHALL BE MAINTAINED ON EACH SIDE OF

WALL UNTIL THE LOWER GRADE IS REACHED. PROVIDE POSITIVE DRAINAGE AWAY FROM BUILDING FOUNDATIONS BOTH DURING CONSTRUCTION AND PERMANENTLY. MAINTAIN STABILITY OF EXCAVATIONS UNTIL PROPERLY BACKFILLED. KEEP EXCAVATIONS FREE OF LOOSE MATERIAL. DEWATER EXCAVATIONS AND REMOVE ANY WET MATERIAL PRIOR TO PLACING CONCRETE. HEAVY EQUIPMENT USED FOR PLACING OR COMPACTING BACKFILL SHALL NOT BE OPERATED WITHIN A DISTANCE EQUAL TO THE HEIGHT OF THE BACKFILL ABOVE THE TOP OF FOOTING, (1 HORIZONTAL TO 1 VERTICAL). HAND OPERATED COMPACTION EQUIPMENT SHALL BE USED FOR COMPACTION OPERATIONS IN THIS AREA. EXCAVATION BRACING SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. EXCAVATION BRACING SHALL BE DESIGNED FOR LATERAL LOADING RESULTING FROM AN EQUIVALENT FLUID PRESSURE OF 60 PCF AND A SURFACE SURCHARGE OF 250 PSF.

CAST IN PLACE STRUCTURAL CONCRETE

SUBMIT MIX DESIGNS FOR EACH TYPE OF CONCRETE SPECIFIED. SUBMIT DATA FOR ALL ADMIXTURES. CURING COMPOUNDS AND HARDENERS THAT ARE INTENDED FOR USE. TESTING LABORATORY SHALL SAMPLE AND TEST CONCRETE AS FOLLOWS: TAKE SAMPLES IN ACCORDANCE WITH ASTM C31. SAMPLE 4 CYLINDERS FOR EACH 100 CUBIC YARDS, 5000 SF OF SURFACE AREA OR FOR EACH PLACEMENT OF EACH TYPE OF CONCRETE PLACED IN ANY ONE DAY.

TEST WHEN SAMPLES ARE TAKEN FOR AIR CONTENT AND SLUMP IN ACCORDANCE WITH ASTM C143. TEST CYLINDERS FOR COMPRESSIVE STRENGTH IN ACCORDANCE WITH ASTM C39. TEST 1 CYLINDER AT 7 DAYS

TEST 2 CYLINDERS AT 28 DAYS HOLD ONE CYLINDER IN RESERVE AND BREAK AT 56 DAYS IF THE 28 DAY CYLINDERS DO NOT SATISFY ACI CRITERIA FOR THE SPECIFIED STRENGTH.

TEST REPORTS SHALL BE SENT TO THE STRUCTURAL ENGINEER AND SHALL BE AVAILABLE AT THE JOBSITE CONCRETE SHALL HAVE THE MINIMUM 28 DAY COMPRESSIVE STRENGTH AND WEIGHTS: WATER/CEMENT RATIO LOCATION 28 DAY STRENGTH UNIT WEIGHT FOUNDATIONS 4.000 PSI 145 PCF 0.50 MAX

COARSE AGGREGATE TO BE 1" MAXIMUM. CONCRETE WORK SHALL CONFORM TO ACI 318. REINFORCING BARS SHALL CONFORM TO ASTM A615 GRADE 60.

REINFORCING BARS TO BE WELDED SHALL CONFORM TO ASTM A706 GRADE 60. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A82 AND A185. PROVIDE MATERIAL IN SHEETS. LAP ALL WELDED WIRE

FABRIC ONE FULL SQUARE PLUS 2" AT ALL SHEET EDGES. REINFORCING BAR SUPPORT DEVICES SHALL CONFORM TO CRSI MANUAL OF STANDARD PRACTICE. CONCRETE CLEAR COVER ON EMBEDDED REINFORCING SHALL BE AS FO

HALL BE AS FOLLOWS:	
BAR SIZE	MINIMUM CLEAR COVER
ALL	3" BOTTOM AND SIDES,
	2" TOP
#5 AND SMALLER	1 1/2"

#6 THROUGH #18 ALL CONTINUOUS BARS SHALL HAVE A CLASS B TENSION LAP SPLICE AT ALL SPLICES UNO. PROVIDE CORNER BARS FOR ALL CONTINUOUS BARS AT ALL FOUNDATION AND WALL CORNERS AND INTERSECTIONS. LAP CORNER BARS 48 BAR DIAMETERS EACH END. PROVIDE DOWELS TO FOOTINGS TO MATCH ALL WALL, PIER AND COLUMN VERTICAL REINFORCING UNO. EMBED DOWELS IN

FOOTING WITH HOOK TO WITHIN 3" OF BOTTOM OF FOOTING. EXTEND DOWELS ABOVE FOOTING FOR 48 BAR DIAMETER LAP SPLICE WITH VERTICAL REINFORCING UNO. CONSTRUCTION AND CONTRACTION JOINTS IN WALLS SHALL BE LOCATED AT 20'-0" OC MAXIMUM AND 10'-0" MAXIMUM FROM WALL CORNERS. ALIGN JOINTS IN WALLS WITH JOINTS IN SLABS AT LOCATIONS WHERE SLABS ARE CONNECTED TO WALLS. CONFORM TO ACI 306 FOR COLD WEATHER CONCRETE AND ACI 305 FOR HOT WEATHER CONCRETE WORK WHEN ANY COMBINATION OF TEMPERATURE, HUMIDITY OR WIND SPEED RESULTS IN CONDITIONS THAT WOULD IMPAIR THE QUALITY OF CONCRETE. CONCRETE IS TO BE REJECTED IF ITS TEMPERATURE AT TIME OF PLACEMENT IS 90 DEGREES F OR ABOVE. CHAMFER ALL EXPOSED CONCRETE EDGES 3/4" UNO. SEE ARCHITECTURAL DRAWINGS FOR DETAILS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING ALL EMBEDDED ITEMS IN CONCRETE WORK. COORDINATE WITH THE FOLLOWING: CIVIL, ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS, PRECAST SHOP

STRUCTURAL STEEL

STRUCTURAL STEEL CONSTRUCTION DETAILING, FABRICATION AND ERECTION SHALL CONFORM TO THE AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS".

A	. STRUCTURAL STEEL MEMBERS SHALL CONFORM	1 TO THE FOLLOWING STANDARDS:
	WIDE FLANGE SHAPES	ASTM A992
	ANGLE, CHANNELS AND PLATES	ASTM A36
	ANCHOR RODS <= 3/4"Ø	ASTM F1554 GRADE 36
	ANCHOR RODS >= 7/8"Ø	ASTM F1554 GRADE 55
	PIPE	ASTM A53
	RECTANGULAR HSS	ASTM A500 GRADE C, 50 ksi
	HEADED STUDS	ASTM A108, GRADE 1015-1020
A	LL STEEL EXPOSED TO WEATHER SHALL BE HOT DIPPE	D GALVANIZED.
S	PLICING OF STRUCTURAL STEEL MEMBERS IS PROHIBIT	TED WITHOUT PRIOR WRITTEN APPROVAL OF THE ENGINEER FOR THE
	OCATION AND TYPE OF SPLICE.	
С	AMBER BEAMS WHERE INDICATED. WHERE NO CAMBER	R IS INDICATED, BEAMS SHALL BE FABRICATED SO THAT AFTER
	RECTION, ANY NATURAL CAMBER IS UPWARD.	
	, ,	QUIRED IN STRUCTURAL STEEL MEMBERS FOR ERECTION OR THE
		SHOP DRAWINGS AT TIME OF SUBMITTAL FOR REVIEW.
F	IELD MODIFICATION OF STRUCTURAL STEEL IS PROHIBI	TED WITHOUT PRIOR WRITTEN APPROVAL OF THE STRUCTURAL
_	NGINEER OF RECORD.	
S	HOP DRAWINGS SHALL BE SUBMITTED FOR ALL STRUC	TURAL STEEL AND SHALL INDICATE COMPLETE CONNECTION

INFORMATION, BOTH SHOP AND FIELD. PROVIDE A SHOP COAT OF FABRICATOR'S STANDARD RUST INHIBITIVE PRIMER TO ALL STEEL UNO.

FILL SOLID WITH NON-SHRINK GROUT UNDER ALL BASE AND BEARING PLATES (5,000 PSI MIN AT 28 DAYS). CONNECTION NOTES: CONNECTION MATERIALS SHALL CONFORM TO THE FOLLOWING STANDARDS AND MATERIAL PROPERTIES:

ANGLES	ASTM A36
WT	ASTM A992
PLATES	ASTM A36
BOLTS	ASTM A325
NUTS	ASTM A563
WASHERS	ASTM F436
WELDING ELECTRODES	E70XX
BOLTED CONNECTIONS SHALL CONF	FORM TO THE PROVISIONS OF THE RESEARCH COUNCIL ON STRUCTURAL
CONNECTIONS "SPECIFICATION FOR	STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS."
WELDED CONNECTIONS SHALL BE M	IADE WITH CONTINUOUS FILLET WELDS UNO. MINIMUM WELD SIZE SHALL E

ASTM A325 OR A490 BOLTS." WELDED CONNECTIONS SHALL BE MADE WITH CONTINUOUS FILLET WELDS UNO. MINIMUM WELD SIZE SHALL BE 1/4" OR AS REQUIRED BY AISC SPECIFICATION, WHICHEVER IS LARGER. MINIMUM WELD LENGTH SHALL BE 2". ALL WELDS SHALL BE MADE BY AWS CERTIFIED WELDERS, APPROVED BY DSA.

LIGHT GAGE METAL FRAMING

1.	LIGHT GAGE METAL FRAMING INDICATED ON THE DRAWINGS INDICATES TYPICAL CONDITIONS AND MINIMUM
	REQUIREMENTS.
2.	SHOP DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT/ENGINEER OF RECORD FOR REVIEW. SHOP DRAWINGS
	SHALL INCLUDE LAYOUT OF ALL LIGHT GAGE METAL FRAMING INCLUDING ARRANGEMENT, DIMENSIONS, MATERIALS,
	STRESS VALUES, CONNECTORS, ANCHORAGE, AND RELATION TO ADJACENT WORK.
3.	LIGHT GAGE METAL FRAMING DESIGN AND CONSTRUCTION SHALL CONFORM TO THE AISI NORTH AMERICAN
4	SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS.
4.	MINIMUM GAGE OF MEMBERS PROVIDING LATERAL SUPPORT FOR MASONRY VENEER SHALL BE 18 GAGE (43 MILS).
5.	LIMIT LATERAL DEFLECTION OF STUDS PROVIDING LATERAL SUPPORT FOR MASONRY VENEER TO H/600. MINIMUM YIELD STRENGTH (Fy) FOR LIGHT GAGE METAL FRAMING MEMBERS SHALL BE 33,000 PSI FOR 18 GAGE (43
5.	MINIMOM TIELD STRENGTH (FY) FOR EIGHT GAGE METAL FRAMING MEMBERS SHALL BE 53,000 FSTFOR 18 GAGE (43 MILS) AND THINNER. MINIMUM YIELD STRENGTH (FY) FOR MEMBERS SHALL BE 50,000 PSI FOR 16 GAGE (54 MILS) AND
	THICKER.
6.	ALL LIGHT GAGE METAL STUDS, TRUSSES, TRACK, BRIDGING AND ACCESSORIES SHALL BE FORMED FROM STEEL
-	HAVING A G-60 GALVANIZED COATING CONFORMING TO ASTM A653 AND C955.
7.	A MINIMUM OF 10" LENGTH OF UN-PUNCHED STEEL IS REQUIRED AT ENDS OF STUDS AND AT ALL BEARING POINTS
	AND CONCENTRATED LOADS (NO PUNCHING HOLES OF ANY SIZE IS PERMITTED IN THESE 10 INCHES). NO CUTTING OF
	THE STUD FLANGES IS PERMITTED.
8.	SPLICES IN LOAD BEARING STUDS ARE NOT PERMITTED.
9.	LOAD BEARING STUDS SHALL HAVE FULL BEARING AGAINST THE INSIDE TRACK WEB TOP AND BOTTOM. STUD ENDS
10	SHALL BE CUT SQUARE.
10.	LATERAL BRIDGING SHALL BE USED TO PROVIDE LATERAL STABILITY OF LOAD BEARING STUDS. BRIDGING SHALL BE (2) 1 1/2" - 18 GA (43 MILS) FLAT STRAP (ONE EACH SIDE OF WALL). FASTEN BRIDGING TO EACH STUD FLANGE WITH (1)
	#10 SCREW. PROVIDE TRACK BLOCKING BETWEEN STUDS IN LINE WITH BRIDGING SPACED AT 10'-0" MAXIMUM ALONG
	LENGTH OF ALL BRIDGING LINES AND EACH SIDE OF WALL OPENINGS.
11.	BRIDGING IS TO BE SPACED AT 4' - 0" OC VERTICALLY.
12.	MINIMUM TRACK FASTENING AT FOUNDATION SHALL BE 0.177"Ø POWDER ACTUATED FASTENERS (PAF) SPACED AT 8"
	OC. WITH 1 1/2" MINIMUM PENETRATION INTO CONCRETE.
13.	CUTTING OF LOAD BEARING METAL STUDS, TRACK, BRIDGING OR BRACING IS NOT PERMITTED WITHOUT SPECIFIC
	APPROVAL FROM THE ENGINEER OF RECORD.
14.	ATTACH ALL EXTERIOR SHEATHING AND INTERIOR SHEATHING AT BEARING WALLS TO METAL STUDS WITH #6
	SCREWS SPACED AT 6" OC AT ALL PANEL EDGES AND PANEL INTERIOR. REFER TO ARCHITECTURAL DRAWINGS FOR
	NON-LOAD BEARING WALLS AND ALL WALL DIMENSIONS.

POST-INSTALLED ANCHORS

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			•	HILTI K	WIK BOL	T-TZ EXP
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				VACUL	JM (VC10	5 OR VC 3
2.	ALL A	NCHOF	R BASE PL	ATES, F	RESSUR	E TREATE
	STAI	VLESS S	STEEL.			
3.	ANCH	IOR CA	PACITY US	SED IN [DESIGN S	HALL BE
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	TO TH	HE COM	IMENCEM	ENT OF	INSTALL	ING ANCH
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EXISTING REINFORCING BARS IN THE CONCRETE STRUCTURE MAY CONFLICT WITH SPECIFIC ANCHOR LOCATIONS. THE POSITION OF THE REINFORCING BARS AT THE CONCRETE ANCHORS, BY HILTI FERROSCAN, GPR, X-RAY, OR OTHER MEANS.

S, POST-INSTALLED ANCHORS SHALL CONSIST OF THE FOLLOWING ANCHOR HILTI AT (800) 879-8000 FOR PRODUCT RELATED QUESTIONS. ACKED AND UNCRACKED CONCRETE USE: SET SYSTEM WITH HILTI HIT-Z ROD PER ICC ESR-3187.

VCHORS FOR CRACKED AND UNCRACKED CONCRETE USE: D KWIK HUS EZ-1 SCREW ANCHORS PER ICC ESR-3027 PANSION ANCHORS PER ICC ESR-1917

CKED AND UNCRACKED CONCRETE USE: SYSTEM WITH HILTI HOLLOW DRILL BIT (TE-CD OR TE-YD) AND VC150/300 C 300) SYSTEM WITH CONTINUOUSLY DEFORMED REBAR PER ICC ESR-3187 TED WOOD SILLS, OR EXTERIOR APPLICATIONS SHALL BE GALVANIZED OR

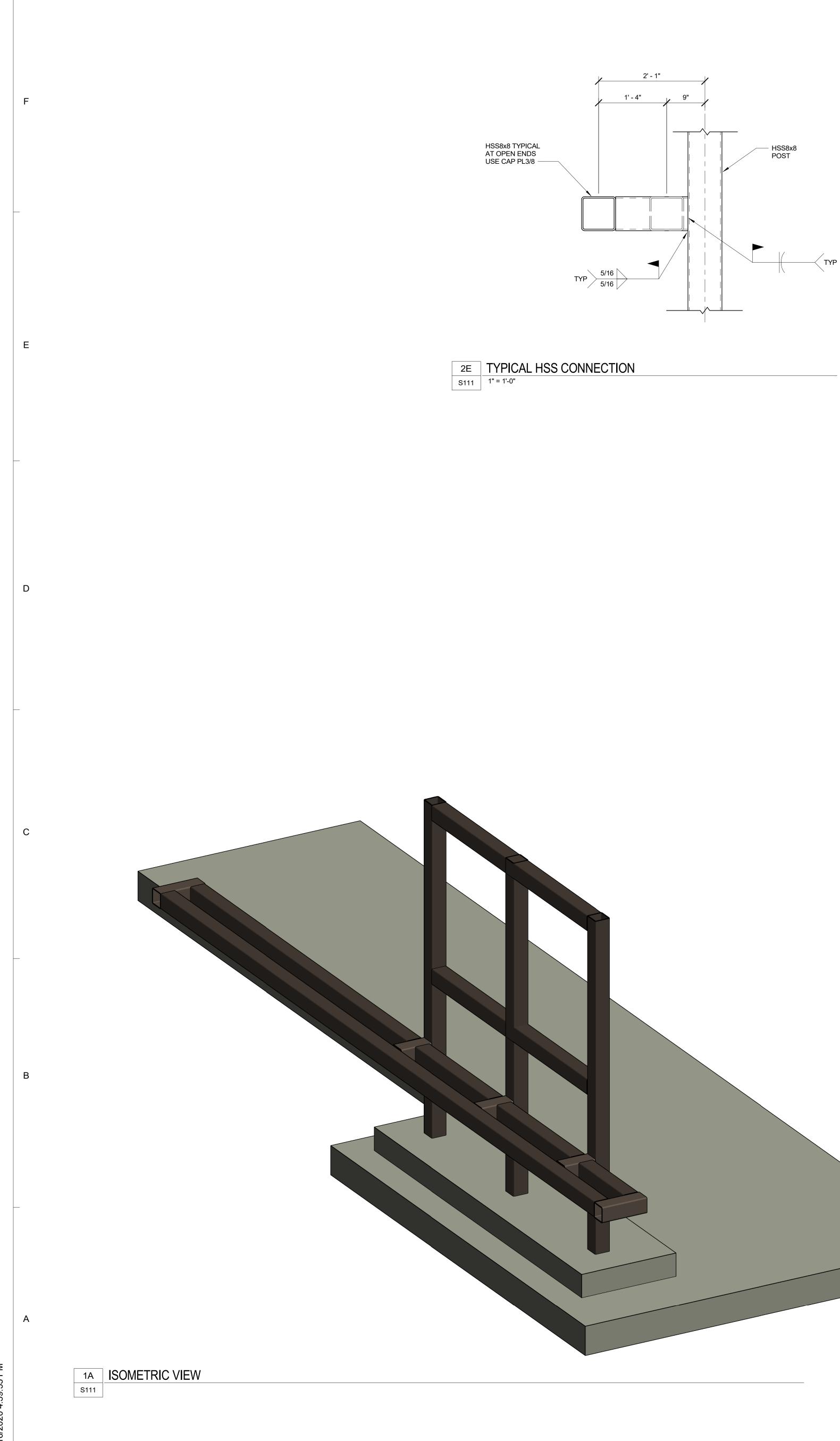
E BASED ON THE TECHNICAL DATA PUBLISHED BY HILTI OR SUCH OTHER ENGINEER OF RECORD. R INSTRUCTIONS, AS INCLUDED IN THE ANCHOR PACKAGING.

STALLED USING THE HILTI PROFI SYSTEM. IOR MANUFACTURER'S REPRESENTATIVE TO PROVIDE ONSITE INSTALLATION DUCTS SPECIFIED. THE STRUCTURAL ENGINEER OF RECORD MUST RECEIVE THE CONTRACTOR'S PERSONNEL WHO INSTALL ANCHORS ARE TRAINED PRIOR

HORS. ANCHOR CAPACITY IS DEPENDANT UPON SPACING BETWEEN ADJACENT ANCHORS AND PROXIMITY OF ANCHORS TO EDGE OF CONCRETE. INSTALL ANCHORS IN ACCORDANCE WITH SPACING AND EDGE CLEARANCES INDICATED ON DRAWINGS.

EXISTING REBAR AND STANDS MUST NOT BE CUT OR DAMAGED, UNLESS NOTED ON THE DRAWINGS THAT THE BARS CAN BE CUT. THE CONTRACTOR SHALL REVIEW THE EXISTING STRUCTURAL DRAWINGS AND SHALL UNDERTAKE TO LOCATE

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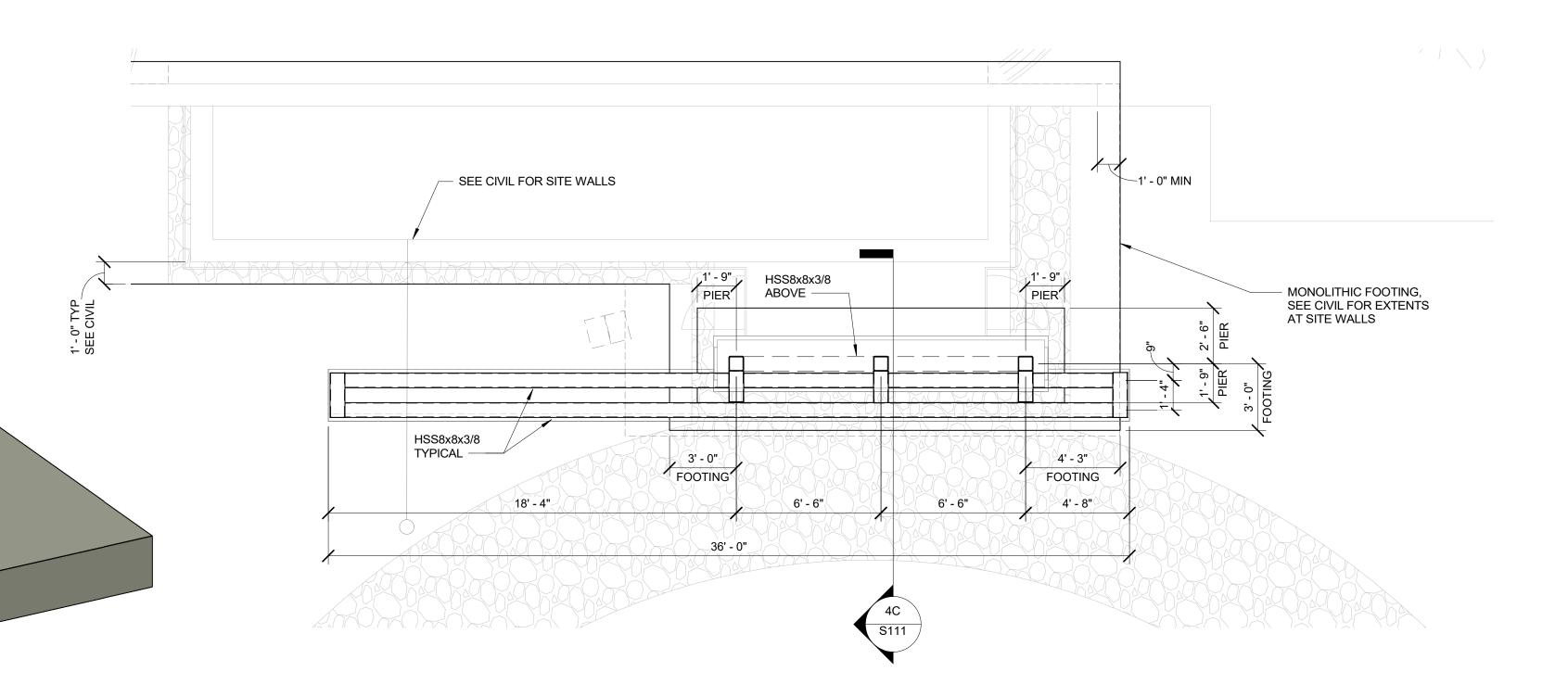
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SIGN LOW GRADE 0" DATUM EL. 101.25' _____ -3"

<u>ო</u> _ნ T/PIER EL. 101.00' ● T/FTG -1' - 3" – EL. 100.00'

(3) #4 HORIZ AT 3" OC ALL SIDES OF PIER W/ CORNER BARS ------#6 DOWELS AT 12" OC

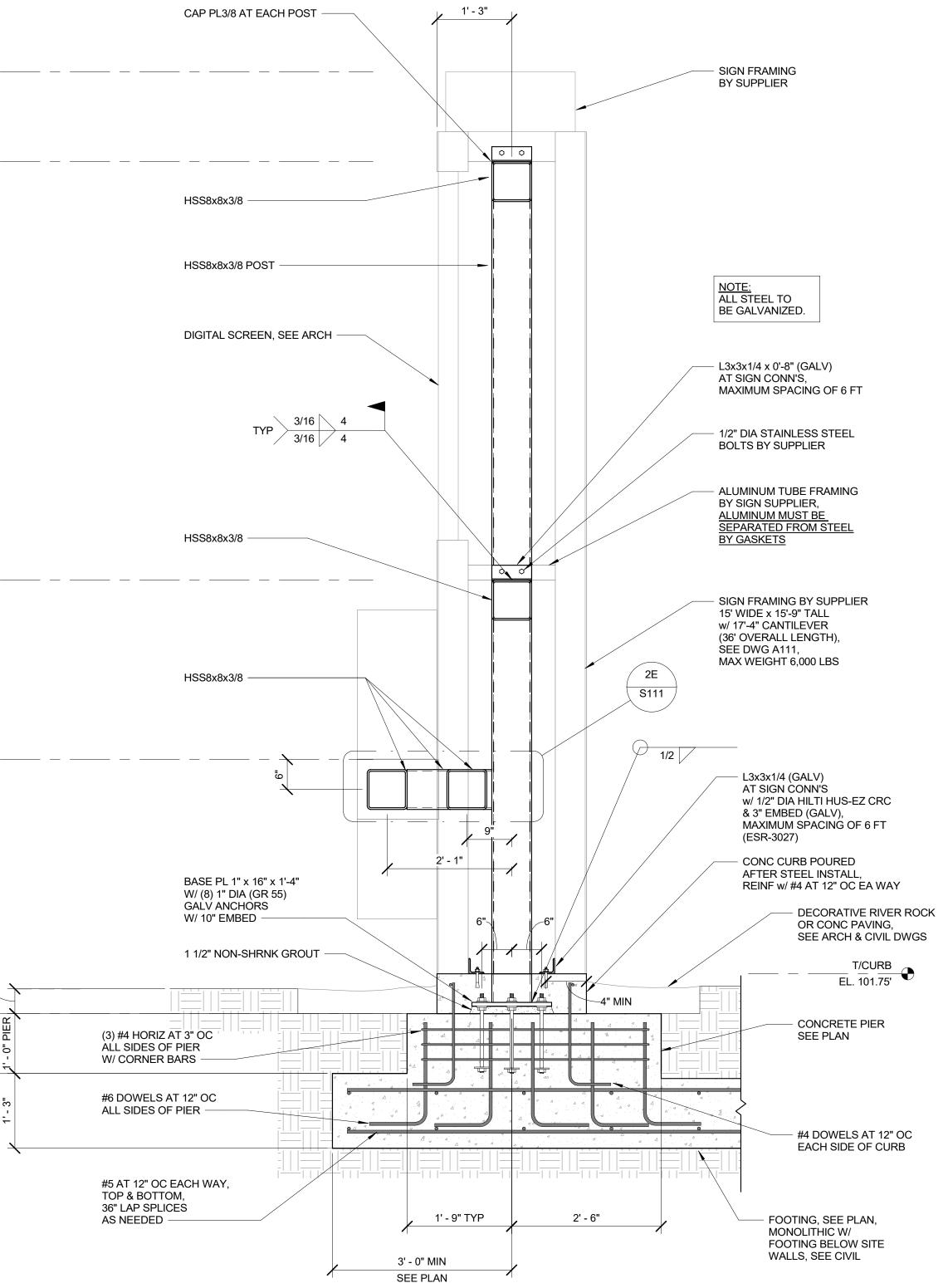
4C MAIN ENTRY SIGN SECTION S111 3/4" = 1'-0"



 4A
 FOUNDATION PLAN

 \$111
 1/4" = 1'-0"

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	E ARCHITECT INC: FOR ACS
1300 Dove Stree Newport Beach T: 949.69 w w w . l i t t l e o This drawing and the desig property of Little Diversifi	eet, Suite 100 n, CA. 92660 8.1400 n l i n e . c o m gn shown are the
Consulting. The reproduce use of this drawing withou is prohibited and any infra- to legal action. © Little	tion, copying or other t their written consent ingement will be subject
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	 ALL 5-20 AMP 128 YOLD, SURGE PHASE RECEPTACLES WITHIN NICHEH AND EXOD REPARATION AREAS TO BE GCT PER INC 2008. PROVIDE LOCAL DISCONNECTS FOR ALL MARXWIRED EQUIPMENT THAT IS NOT "WITHIN SIGHT OF THE SOURCE PANEL. MUTTRE RACEWARS CONTAINING MORE THAN 3 CURRENT CARRYING CONDUCTORS SHALL COMPLY WITH [2016 CCC, 310.15(0)(2)(A)). HI DINTIFICATION OF EMPTY CIRCUIT OF A AVEA. HONDE MORE DEFAIL ON PHACT TO ALLOW EXCIT TO THE DISTINGUISTIC PROVIDE INC. 3016 CCC 300.45 (100.000 CCC) SUFFICIENT OFFICI TO ALLOW EXCIT TO REDISTINGUISTIC PROVIDE RECEPTACLE DISTALLED ON AN INDUDEAL BRANCI CIRCUIT SHALL INC. A MAPERE RATING OF NOT LESS THAN THAT OF THE BRANCH CIRCUIT. INDUCET HE RECEPTACLE DISTING. (20.21(2)(1)) PROVIDE RECEPTACLE DISTALLED ON AN INDUDEAL BRANCI CIRCUIT SHALL INC. A MAPERE RATING OF NOT LESS THAN THAT OF THE BRANCH CIRCUIT. INDUCET HE RECEPTACLE DISTING. (20.21(2)(1)) PROVIDE RECEPTACLE DOTIES CONNECTS ARE NOT PROVIDED WITHIN SIGHT FROM THE EQUIPMENT IT SUPPLIES, THE SWITCH OR CIRCUIT BREAKER MUST INCLUDE PROVISION FOR TADIO AND ADDIG AND THESE PROVISION SUBST BHAMM WITH THE EQUIPMENT THE SWITCH OR CIRCUIT BREAKER MUST INCLUDE PROVISIONS FOR TADIO ADDIG AND THESE PROVISIONS SUBST BHAMM WITH THE EQUIPMENT THE SWITCH OR CIRCUIT BREAKER MUST INCLUDE PROVISIONS FOR TADIO THE RECEPTACLES IN 120- AND 250-YOLT CONTIGURATION AT WET/DAMP LOCATION ARE REQUIRED TO BE LISTED WEATHER RESISTANT TYPE. (CCC 466.8(A)). 	Interest DERRATING TABLE Nome Than Three Current-CARRYING CONDUCTORS IN A RACEWAY OR CARLE. WHERE THE NUMBER OF CURRENT-CARRYING CONDUCTORS IN A RACEWAY OR CARLE. WHERE THE NUMBER OF CURRENT-CARRYING CONDUCTORS IN A RACEWAY OR CARLE SHALL BE REDUCED AS SM NUMEER OF CURRENT-CARRYING PERCENT OF VALUES IN TABLES 1 THEOLOGI 6 90 21 THEOLOGI 10 20 21 THEOLOGI 10 10 21 THEOLOGI 10 10 21 THEOLOGI 10 10 21 THEOLOGI 10 10 21 THEOLOGI 10 11 22 AND 240 THEOLOGI 10 23 THEOLOGI 10 THE RADIK 10 24 CALE THEOLOGI 10

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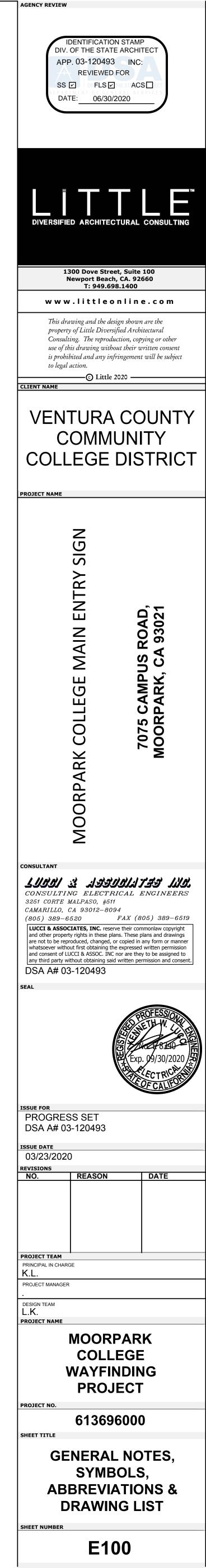
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DATE: 16 April 2020 DT DATE: 4/16/2020 5:22:20 PM

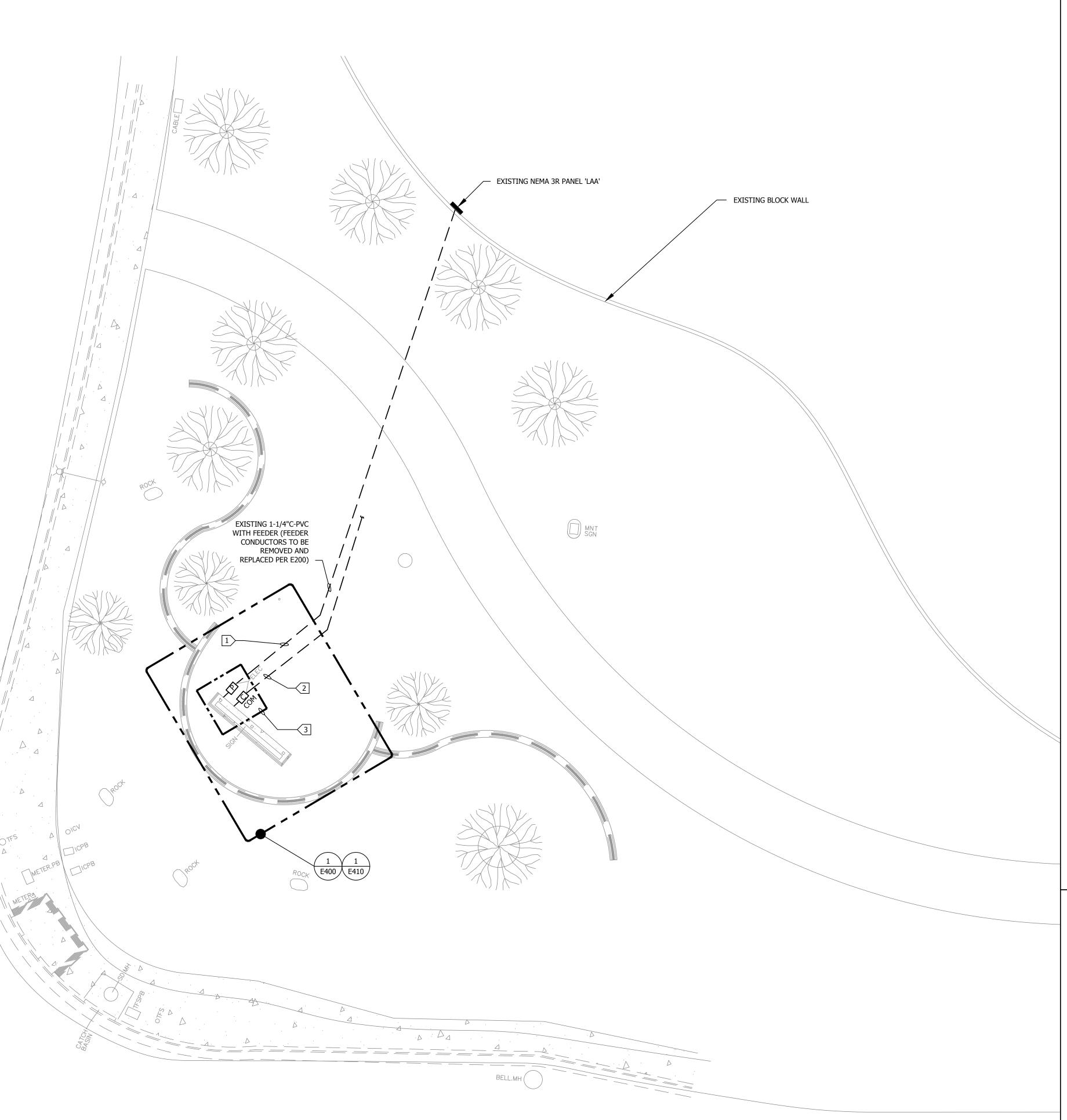
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S & SPECIFICATION. PROVIDE	E150 E200	EXISTING ELECTRICAL SITE DEMOLITION ELECTRICAL SINGLE LINE DIAGRAM & PAN			
	E400 E410	ELECTRICAL SITE PLAN - NEW WORK ELECTRICAL PLAN ENLARGED AREA - NEW			
	E600	ELECTRICAL PLAN ENLARGED AREA - NEW	WORK		
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	DI	EMO OR EXTEND OLD SIGN ELEC			
ALL) STANDARDS
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	CALIF (2018	ORNIA CODE OF REGULATIONS (CCR) TITLE INTERNATIONAL BUILDING CODE (IBC) W/C		(CAL	CALIFORNIA GREEN BUILDING STANDARDS CODE GREEN), PART II, TITLE 24 C.C.R.
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NTRACTOR TO PROVIDE DO NOT HAVE COMMON	TITLE	II - ACCESSIBILITY GUIDELINES FOR BUILDI STATE FIRE MARSHAL REGULATIONS AND AM			
G THE CIRCUITS)		ABBREVIAT	TONS	Ν	10UNTING HEIGHT
		AMPERES M	METER	_	VER OBSTRUCTION
	AFC AFF	AMP FRAME/AMP FUSE MC AVAILABLE FAULT CURRENT MDF ABOVE FINISHED FLOOR MIN	MINIMUM		
OUNTED @ 42" AFF	ARCH AS	AMP INTERRUPTING CURRENT MTD ARCHITECT MTB AMP SWITCH MTG AMERICAN SOCIETY OF MV	MAIN TELEPHONE BACKBOARD		
	AT	AMERICAN SOCIETY OF MV TESTING MATERIAL(S) MH AMP TRIP MFG AMERICAN WIRE GAGE NEC	METAL HALIDE MANUFACTURER	I	2'-6" CLR
	BKBD C	BACKBOARD (N) CONDUIT OR CEILING NIC CIRCUIT BREAKER NL	NEW NOT IN CONTRACT NIGHT LIGHT	-	TOP
	CONT CKT	CONTINUATIONNOCIRCUITNCCEILINGOH	NORMALLY OPEN NORMALLY CLOSED OVERHEAD		
ECTION U.O.N.	CTV	CONDUIT ONLY P CABLE TELEVISION PBO COPPER PNL	Power or Pole Provided by others Panel		
	DIS DS	COLD WATER PIPE (R) DISCONNECT RGS DISCONNECT SWITCH	CONDUIT		3'10" MAX 4'0" MAX
	ECD EM	DRAWING RM ELECTRICAL CONTRACTOR SN EMERGENCY LIGHT/FEEDER SPD			3'10" M
	EOR EPR	ELECTRICAL METAL TUBINGTCENGINEER OF RECORDTTBETHYLENE PROPYLENE RUBBERTTC	TELEPHONE TERMINAL CABINET		
	FS FT	FRONT TR SHALLOW FLOOR BOX TVS: FEET	SUPPRESSOR		
	GFI GND	GENERAL CONTRACTOR TYP GROUND FAULT INTERRUPTER UG GROUND UL HORSEPOWER UON	UNDERGROUND UNDERWRITERS LABORATORY	SW	TOP OF THERMOSTAT, ITCH, OUTLET, CONTROL
ORS	ID IDF	IDENTIFICATION UNS INTERMEDIATE DISTRIBUTION V FRAME VA		ዊ	THERMOSTAT, SWITCH, OUTLET, CONTROL
GREEN FOR GROUND.	JB	ISOLATED GROUND VD JUNCTION BOX W KILO WP	VOLTAGE DROP WATTS/WATTAGE OR WIRE WEATHERPROOF		
R NEUTRAL, GREEN FOR	LC LCL	KILO VOLT AMPS=1000VAW/LIGHTING CONTACTOR(X)LONG CONTINUOUS LOADφ	WITH EXISTING PHASE		
		LOW VOLTAGE	SITE/AR		ЧАР
R OF CURRENT-CARRYING D AS SHOWN IN THE					
TABLES AS ADJUSTED	-		No la glass		
ATURE IF NECESSARY	-		一次	-	3 Deal and a second second second
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DUCTOR SHALL BE REDUCED AS			Institute of Religion		ETT
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HALL APPLY. NOT EXCEEDING 24 INCHES		a.		-	CAR AR
LEAVING AN OUTDOOR TRENCH					
ATE METAL CONDUIT, OR RIGID BER OF CONDUCTORS DOES NOT				al.	MOORPARK
PERMITTED TO BE CALCULATED					
RRYING CONDUCTORS IN A			Fr Pall		
S SHALL BE MAINTAINED.					RAC (Entition Statelium)
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CAMPUS PARK DRIVE

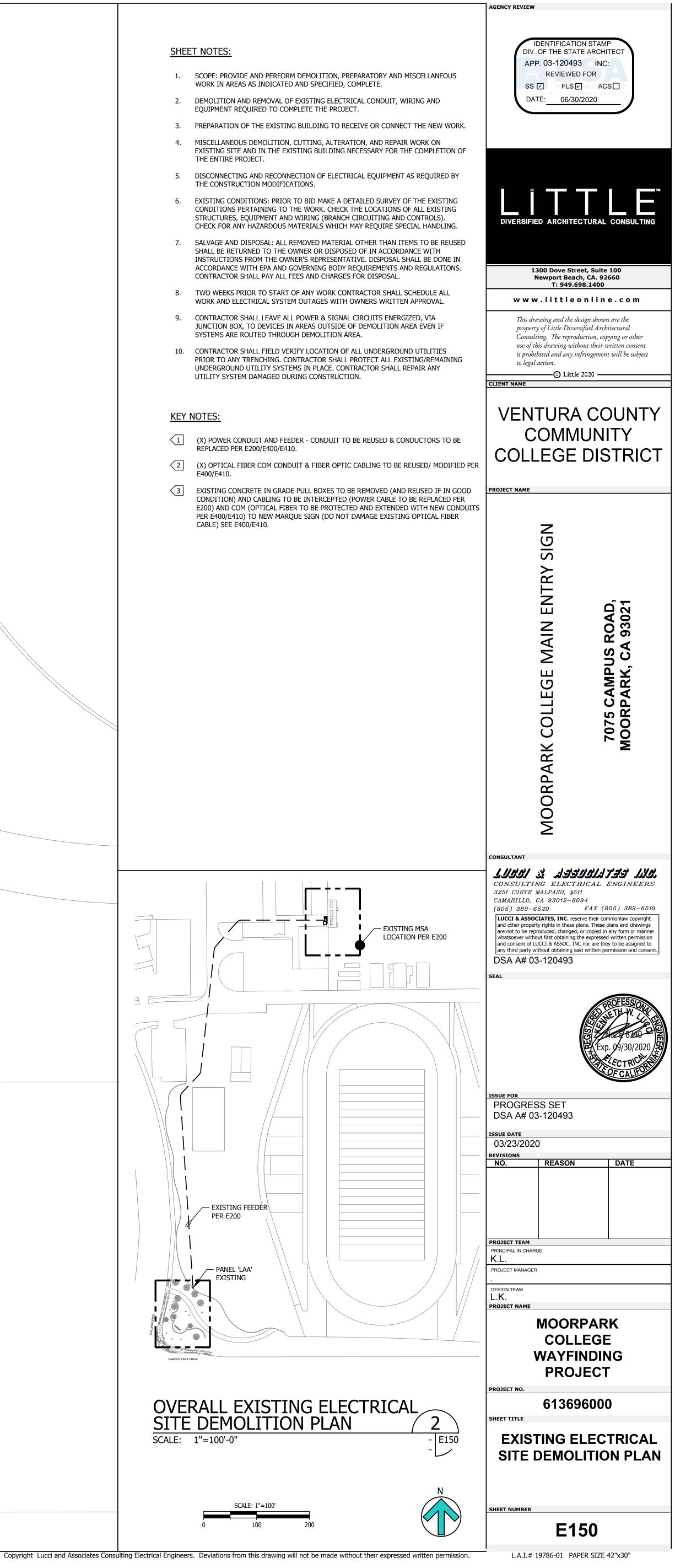
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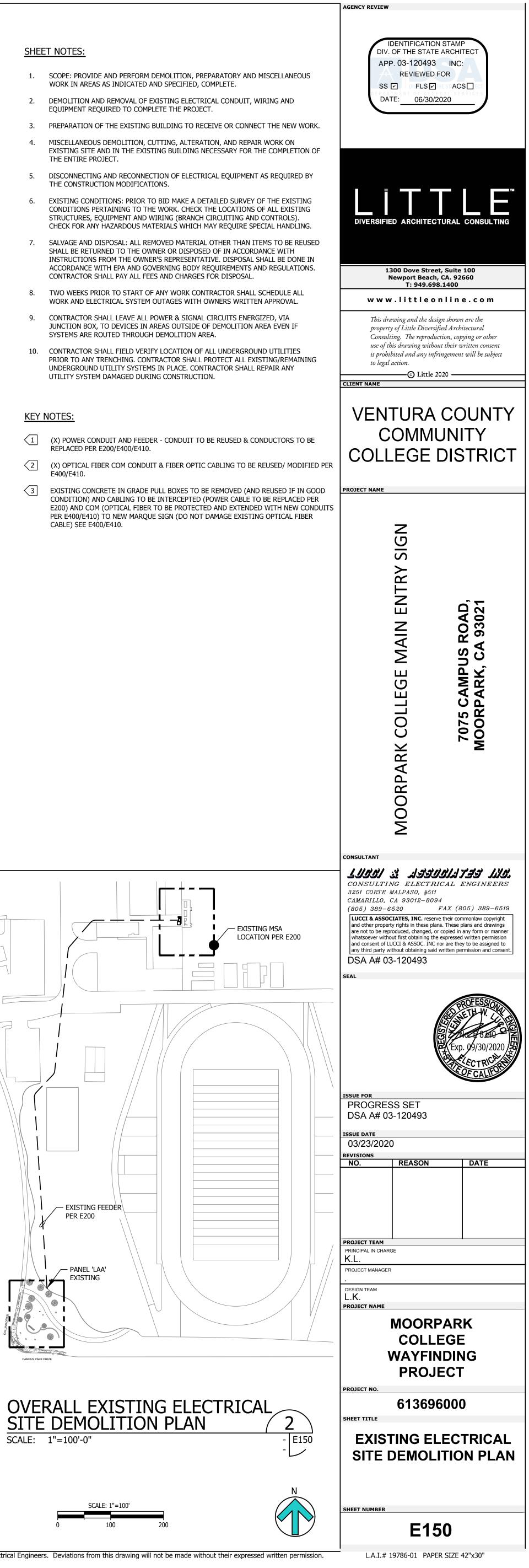
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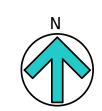
EXISTING ELECTRICAL SITE DEMOLITION PLAN (

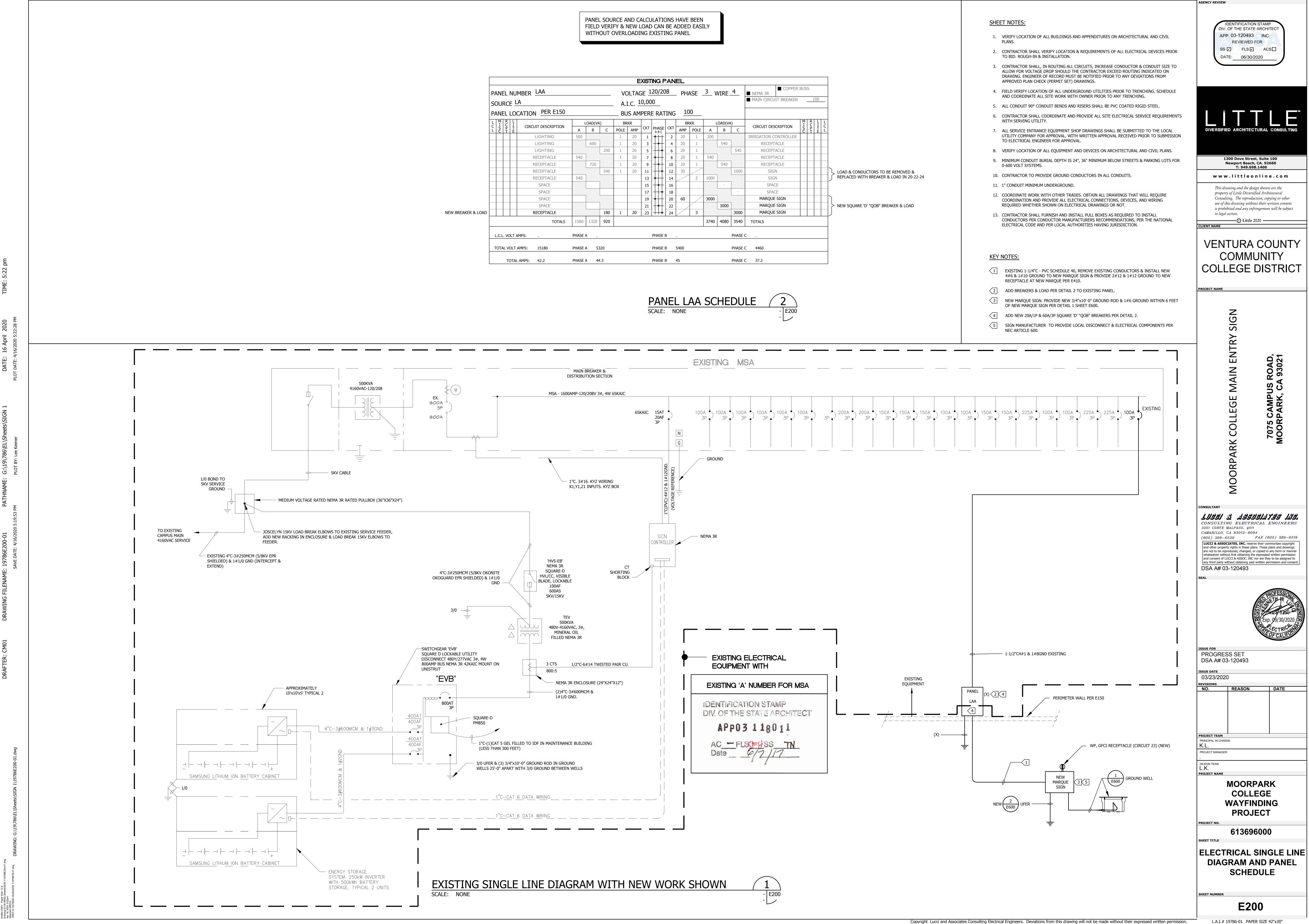
- THE CONSTRUCTION MODIFICATIONS.
- SHALL BE RETURNED TO THE OWNER OR DISPOSED OF IN ACCORDANCE WITH CONTRACTOR SHALL PAY ALL FEES AND CHARGES FOR DISPOSAL.
- SYSTEMS ARE ROUTED THROUGH DEMOLITION AREA.
- UNDERGROUND UTILITY SYSTEMS IN PLACE. CONTRACTOR SHALL REPAIR ANY UTILITY SYSTEM DAMAGED DURING CONSTRUCTION.







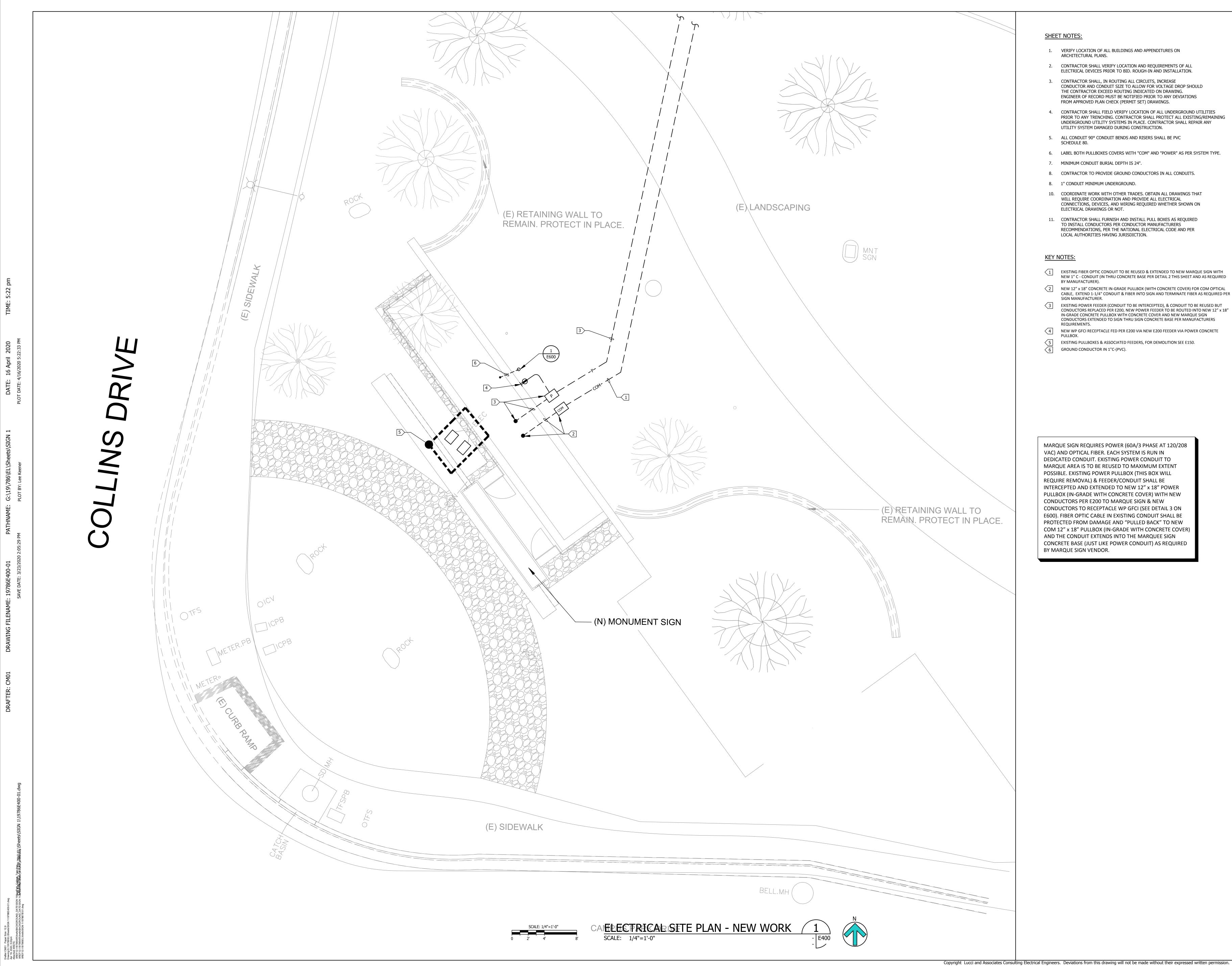




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									EXISTING PANEL										
	PANEL NUMBER LAA SOURCE LA PANEL LOCATION PER E150						VOLTAGE 120/208 PHASE				3 WIRE 4			NEMA 3R					
								A.]	I.C. <u>1</u>	0,00	00							MAIN CIRCUIT BE	
							BUS AMPERE RATING 100				_								
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	C L	Ŝ	P T	Ť	CIRCUIT DESCRIPTION	А	В	С	POLE	AMP	СКТ	PHASE A B C	СКТ	AMP	POLE	Α	В	С	CIRCUIT DESCRIP
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W BREAKER & LOAD					RECEPTACLE			180	1	20	23	+++	24		3			3000	MARQUE SIG
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			тс	DTAL	AMPS: 42.2	PHASE A	A 44	ł.3				PHASE	В	45				PHASE	C 37.2

L.A.I.# 19786-01 PAPER SIZE 42"x30"



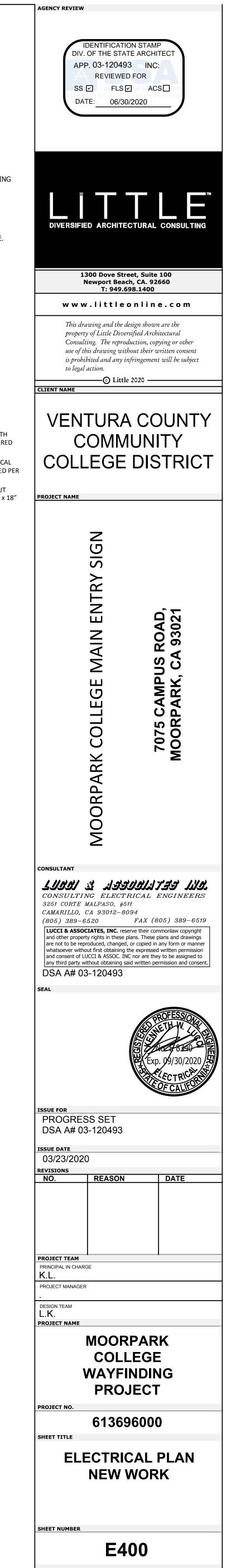
SHEET NOTES:

- 1. VERIFY LOCATION OF ALL BUILDINGS AND APPENDITURES ON ARCHITECTURAL PLANS.
- 2. CONTRACTOR SHALL VERIFY LOCATION AND REQUIREMENTS OF ALL ELECTRICAL DEVICES PRIOR TO BID. ROUGH-IN AND INSTALLATION.
- CONTRACTOR SHALL, IN ROUTING ALL CIRCUITS, INCREASE CONDUCTOR AND CONDUIT SIZE TO ALLOW FOR VOLTAGE DROP SHOULD THE CONTRACTOR EXCEED ROUTING INDICATED ON DRAWING. ENGINEER OF RECORD MUST BE NOTIFIED PRIOR TO ANY DEVIATIONS FROM APPROVED PLAN CHECK (PERMIT SET) DRAWINGS.
- 4. CONTRACTOR SHALL FIELD VERIFY LOCATION OF ALL UNDERGROUND UTILITIES PRIOR TO ANY TRENCHING. CONTRACTOR SHALL PROTECT ALL EXISTING/REMAINING UNDERGROUND UTILITY SYSTEMS IN PLACE. CONTRACTOR SHALL REPAIR ANY UTILITY SYSTEM DAMAGED DURING CONSTRUCTION.
- 5. ALL CONDUIT 90° CONDUIT BENDS AND RISERS SHALL BE PVC SCHEDULE 80.
- 6. LABEL BOTH PULLBOXES COVERS WITH "COM" AND "POWER" AS PER SYSTEM TYPE.
- 7. MINIMUM CONDUIT BURIAL DEPTH IS 24".
- 8. CONTRACTOR TO PROVIDE GROUND CONDUCTORS IN ALL CONDUITS.
- 8. 1" CONDUIT MINIMUM UNDERGROUND.
- 10. COORDINATE WORK WITH OTHER TRADES. OBTAIN ALL DRAWINGS THAT WILL REQUIRE COORDINATION AND PROVIDE ALL ELECTRICAL CONNECTIONS, DEVICES, AND WIRING REQUIRED WHETHER SHOWN ON ELECTRICAL DRAWINGS OR NOT.
- 11. CONTRACTOR SHALL FURNISH AND INSTALL PULL BOXES AS REQUIRED TO INSTALL CONDUCTORS PER CONDUCTOR MANUFACTURERS RECOMMENDATIONS, PER THE NATIONAL ELECTRICAL CODE AND PER LOCAL AUTHORITIES HAVING JURISDICTION.

KEY NOTES:

- EXISTING FIBER OPTIC CONDUIT TO BE REUSED & EXTENDED TO NEW MARQUE SIGN WITH $\langle 1 |$ NEW 1" C - CONDUIT (IN THRU CONCRETE BASE PER DETAIL 2 THIS SHEET AND AS REQUIRED BY MANUFACTURER).
- NEW 12" x 18" CONCRETE IN-GRADE PULLBOX (WITH CONCRETE COVER) FOR COM OPTICAL CABLE,EXTEND 1-1/4" CONDUIT & FIBER INTO SIGN AND TERMINATE FIBER AS REQUIRED PER < 2 SIGN MANUFACTURER.
- $\langle 3 \rangle$ EXISTING POWER FEEDER (CONDUIT TO BE INTERCEPTED), & CONDUIT TO BE REUSED BUT CONDUCTORS REPLACED PER E200, NEW POWER FEEDER TO BE ROUTED INTO NEW 12" x 18" IN-GRADE CONCRETE PULLBOX WITH CONCRETE COVER AND NEW MARQUE SIGN CONDUCTORS EXTENDED TO SIGN THRU SIGN CONCRETE BASE PER MANUFACTURERS REQUIREMENTS.
- NEW WP GFCI RECEPTACLE FED PER E200 VIA NEW E200 FEEDER VIA POWER CONCRETE **4** PULLBOX.
- EXISTING PULLBOXES & ASSOCIATED FEEDERS, FOR DEMOLITION SEE E150. 5 GROUND CONDUCTOR IN 1"C-(PVC).

MARQUE SIGN REQUIRES POWER (60A/3 PHASE AT 120/208 VAC) AND OPTICAL FIBER. EACH SYSTEM IS RUN IN DEDICATED CONDUIT. EXISTING POWER CONDUIT TO MARQUE AREA IS TO BE REUSED TO MAXIMUM EXTENT POSSIBLE. EXISTING POWER PULLBOX (THIS BOX WILL REQUIRE REMOVAL) & FEEDER/CONDUIT SHALL BE INTERCEPTED AND EXTENDED TO NEW 12" x 18" POWER PULLBOX (IN-GRADE WITH CONCRETE COVER) WITH NEW CONDUCTORS PER E200 TO MARQUE SIGN & NEW CONDUCTORS TO RECEPTACLE WP GFCI (SEE DETAIL 3 ON E600). FIBER OPTIC CABLE IN EXISTING CONDUIT SHALL BE PROTECTED FROM DAMAGE AND "PULLED BACK" TO NEW COM 12" x 18" PULLBOX (IN-GRADE WITH CONCRETE COVER) AND THE CONDUIT EXTENDS INTO THE MARQUEE SIGN CONCRETE BASE (JUST LIKE POWER CONDUIT) AS REQUIRED BY MARQUE SIGN VENDOR.

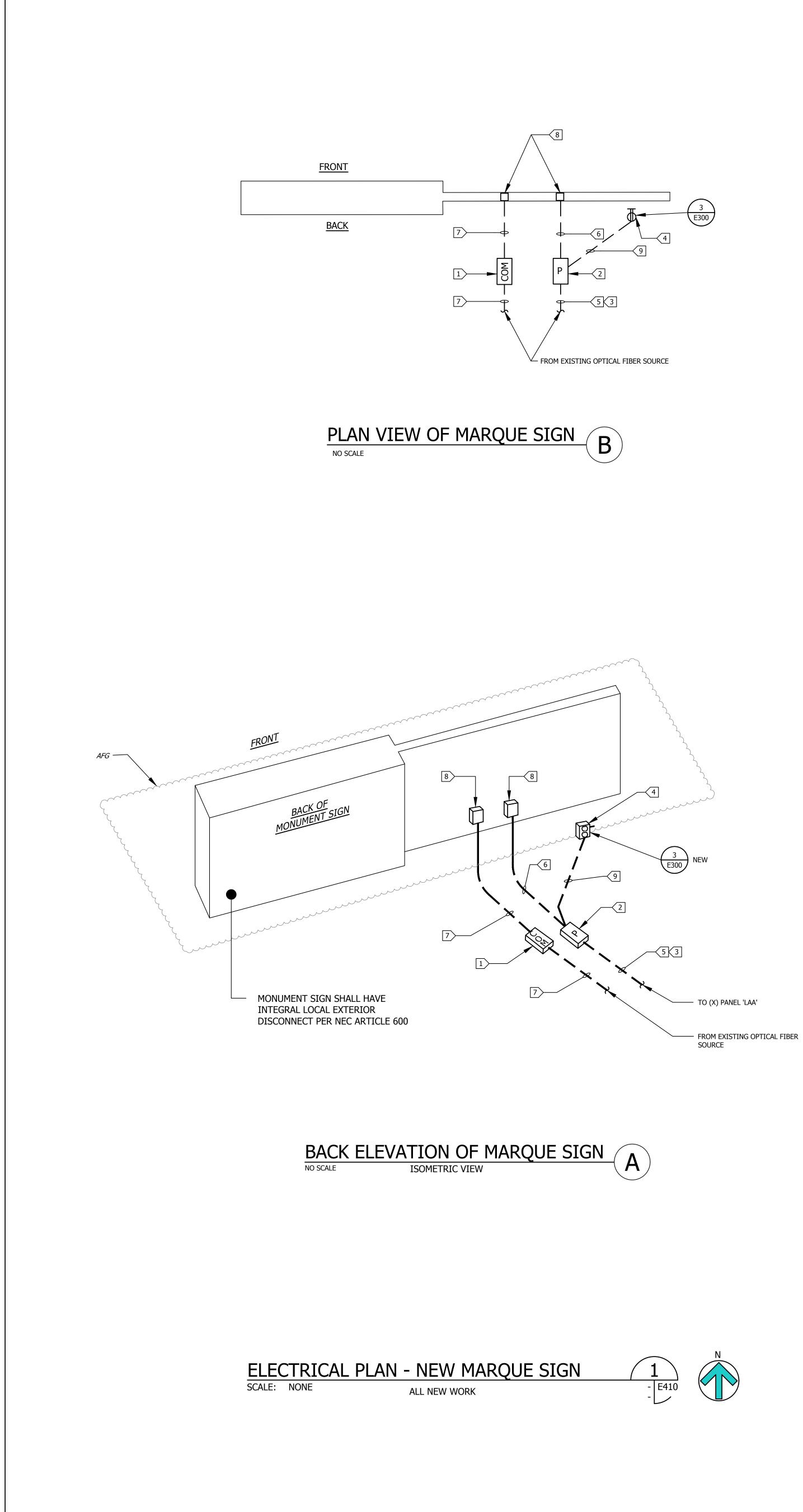


L.A.I.# 19786-01 PAPER SIZE 42"x30"

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SHEET NOTES:

- 1. VERIFY STUB UP LOCATIONS ON ARCHITECTURAL PLANS.
- 2. CONTRACTOR SHALL VERIFY LOCATION AND REQUIREMENTS OF ALL ELECTRICAL CONNECTIONS PRIOR TO BID PROPOSAL, ROUGH-IN, AND FINISH INSTALLATION.
- 3. CONTRACTOR SHALL, IN ROUTING ALL CIRCUITS, INCREASE CONDUCTOR & CONDUIT SIZE TO ALLOW FOR VOLTAGE DROP SHOULD THE CONTRACTOR EXCEED ROUTING INDICATED ON DRAWING. ENGINEER OF RECORD MUST BE NOTIFIED PRIOR TO ANY DEVIATIONS FROM APPROVED PLAN CHECK (PERMIT SET) DRAWINGS.
- 4. CONTRACTOR SHALL FURNISH AND INSTALL PULL BOXES AS REQUIRED TO INSTALL CONDUCTORS PER CONDUCTOR MANUFACTURERS RECOMMENDATIONS, PER THE NATIONAL ELECTRICAL CODE AND PER LOCAL AUTHORITIES HAVING JURISDICTION.
- 5. 1" CONDUIT MINIMUM UNLESS OTHERWISE NOTED. 6. COORDINATE WORK WITH OTHER TRADES. OBTAIN ALL DRAWINGS THAT WILL REQUIRE COORDINATION AND PROVIDE ALL ELECTRICAL CONNECTIONS, DEVICES, AND WIRING REQUIRED WHETHER SHOWN ON ELECTRICAL DRAWINGS OR NOT.
- PROVIDE CODE SIZED EQUIPMENT GROUNDING CONDUCTORS FOR MARQUE SIGN DOCUMENTS.

KEY NOTES:

- NEW 12"x18" CONCRETE IN-GRADE COMMUNICATION PULLBOX (OPTICAL FIBER CABLING TO BE REUSED FOR NEW MARQUE SIGN).
- 2 NEW 12"x18" POWER PULLBOX (CONDUCTORS TO BE REPLACED SEE E200).
- 3 EXISTING CONDUIT, PULL NEW CONDUCTOR PER E200 AND 5.
- 4 NEW RECEPTACLE GFCI.
- 5 REMOVE EXISTING CONDUCTORS AND REPLACE WITH 4#6 & 1#10GND TO MARQUE SIGN & 2#10 & 1#10 FOR NEW WP GFCI RECEPTACLE REUSE IN EXISTING 1-1/4" C FOR NEW FEEDER.
- 6 WITH NEW FEEDER 1-1/4"C-4#6 AND 1#10 GROUND.
 7
 EXISTING INTERCEPTED OPTICAL FIBER FOR MARQUE SIGN TO BE EXTENDED VIA NEW 1-1/4" C
- TO NEW SIGN.
- 8 NEW INTERIOR POINT OF CONNECTION PER SIGN MANUFACTURER.
- 9 NEW 1"C-2#10 AND 1#10 GROUND.



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